

CONNECT THE COASTSIDE

San Mateo County Midcoast Comprehensive Transportation Management Plan *Draft*

This Public Working Draft of Connect the Coastside and will be subject to extensive public review. The Public Working Draft of Connect the Coastside is available at the Planning and Building Website: <https://planning.smcgov.org/connect-coastside>



**COUNTY OF
SAN MATEO**

January 15, 2020

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1 CONNECT THE COASTSIDE EXECUTIVE SUMMARY

1.1 Project Summary

San Mateo County prepared **Connect the Coastsides**, a Comprehensive Transportation Management Plan (CTMP) to improve safety and mobility for residents, businesses and visitors. The plan focuses on the areas surrounding Highway 1 and State Route 92, including the unincorporated communities of Montara, Moss Beach, El Granada, Princeton and Miramar, as well as the City of Half Moon Bay (See **Figure 1**). Connect the Coastsides identifies and prioritizes a diverse range of road, highway and trail improvements, which address the present and future mobility needs of Coastsides communities. The plan seeks to:

- Improve existing traffic conditions and public safety
- Expand transportation choices for residents and visitors
- Encourage environmentally friendly options that reduce car trips, such as walking, biking and public transit
- Respect the character of Midcoast communities and protect coastal resources
- Maintain and improve access to coastal resources for both residents and visitors



Figure 1. San Mateo Connect the Coastsides Planning Area

The plan also helps Coastsides communities be better prepared to meet future transportation needs. As new development occurs, additional transportation improvements will be constructed to address traffic impacts. This plan includes new land use policies that will also help reduce traffic, protect natural resources and preserve coastal community character by limiting development.

The California Coastal Act and the County’s certified Local Coastal Program (LCP) directs the County to “protect, maintain, and where feasible, enhance and restore the overall quality of the coastal zone environment and its natural and artificial resources.” Connect the Coastside will inform the County’s implementation of the public works and land use components of its Local Coastal Program and outlines the partnerships that will be necessary to achieve these improvements. This plan will be the vehicle to apply for funding for priority projects.

Connect the Coastside was developed through an extended public process, building on the community-based Highway 1 Safety and Mobility Studies (Phases 1 and 2) conducted between 2009 and 2012. Working with a consultant team, County staff conducted several public workshops, consulted frequently with a Technical Advisory Committee and met regularly with staff and decision makers from the City of Half Moon Bay. The planning team also conducted a virtual survey and presented frequently to the Midcoast Community Council to develop Connect the Coastside. Thank you to all those who contributed to the plan’s development and helped provide feedback.

San Mateo County looks forward to working with community members, local organizations and partner agencies to see that the goals of Connect the Coastside are met.

1.2 Project Area

The project area for Connect the Coastside includes unincorporated San Mateo County along Highway 1 and the coastline between Devil’s Slide and the northern border of Half Moon Bay – the area covered by the Midcoast LCP. The study area, illustrated in **Figure 1**, includes the unincorporated part of the county along State Route 92 (SR-92) between Half Moon Bay and Interstate 280. The project also considered traffic delays and key intersections in the City of Half Moon Bay, along with future development within the City. Connect the Coastside will guide the County’s transportation plans for the unincorporated part of the study area.

1.3 Planning Context & Relationship to Other Plans

Mobility and multiple transportation options are a priority for people who live and work on San Mateo County’s Coastside. High levels of traffic from visitors and commuters has a negative impact on quality of life. Options for bicycling and walking are limited, as is transit service. This limits Midcoast residents’ transportation choices and makes traffic worse by encouraging driving.

Transportation improvements on the Midcoast are informed by the County’s General Plan and the County’s certified Local Coastal Program. The proposals in Connect the Coastside were evaluated and found to be consistent with these broad policies. Connect the Coastside supports previously identified goals to encourage walking, bicycling, and transit use, while reducing auto trips. Connect the Coastside will also facilitate implementation of Plan Princeton, a land use and shoreline management plan for the Princeton-by-the-Sea community. Although Plan Princeton does not envision a substantial increase in development, it could trigger redevelopment – adding to the need for the improvements outlined in the Connect the Coastside plan.

Connect the Coastside is one of several County efforts to improve mobility on the Midcoast. From 2009-2012, San Mateo County conducted the Highway 1 Safety and Mobility Improvement Study, a two-phase public participatory planning effort. The study developed conceptual plans to improve safety and mobility on Highway 1 between Half Moon Bay and Devil’s Slide. Phase 1, completed in February 2010, focused on Highway 1 from Half Moon Bay Airport south to Frenchman’s Creek Road, including Pillar Point Harbor, Princeton, and the communities of El Granada and Miramar. Phase 2, conducted during Spring and

Summer 2011 in Moss Beach and Montara, evaluated an approximately 5.5-mile segment of Highway 1 from Half Moon Bay Airport through the Devil’s Slide improvement project.

In 2011, the Board of Supervisors adopted substantial amendments to its Local Coastal Program regarding the Midcoast. As part of Coastal Commission certification of these amendments, policy 2.53 was incorporated into the LCP. This policy called for preparation of a comprehensive transportation management plan to address the cumulative impacts of Midcoast development, including the expansion of public transit, consideration of mandatory lot mergers and an in-lieu fee traffic mitigation program.

Implementation of many of the initiatives in Connect the Coastsides relies on active partnerships between the County and Caltrans. Most of the roadway improvements and significant segments of the Multi-modal Trail called for in Connect the Coastsides will be constructed within Caltrans’ right of way. The County will need Caltrans’ assistance for design, planning, funding and constructing these improvements.

Connect the Coastsides will also rely on a partnership with SamTrans, the transit agency for San Mateo County. SamTrans provides bus service to the Coastsides and broader county community. Any expansion of transit service will require investments by SamTrans in vehicles, maintenance and labor. In addition, SamTrans is currently conducting “Reimagine SamTrans,” a planning effort that could yield recommendations for improvements to Coastsides service.

Connect the Coastsides was developed over a five-year period – from late 2014 through 2019 – and is based on accurate traffic data and reasonable projections. Although the study is based on traffic data gathered in 2014, the data provides an accurate snapshot of existing conditions and helps to inform other aspects of the plan, such as projected development and mobility deficiencies.

1.4 Project Vision

The original objectives of Connect the Coastsides were based on the requirements described in LCP Policy 2.53. Based on input from community members and stakeholders gathered at public workshops and meetings, a shared project vision has evolved. The resulting Connect the Coastsides vision statement and goals were created from a combination of the requirements for LCP Policy 2.53, feedback from Midcoast community members, and the findings of the Highway 1 Safety and Mobility Improvement Study.

1.4.1 Vision Statement

Create a safe and functional multi-modal transportation system that preserves the existing character of the Midcoast, serves both Coastsides residents and visitors and accommodates existing and anticipated future traffic.

1.4.2 Goals & Objectives

The broad goals of Connect the Coast are listed below, each followed by specific objectives. The goals set the general direction of this plan and describe the community’s preferred future. The objectives describe specific measurable steps that contribute to reaching the goal.

Goal 1 Improve existing traffic and roadway conditions on the Midcoast.

Objective 1.1 Identify existing trouble spots on the Midcoast roadway system and propose mitigation measures.

Goal 2 Lessen the cumulative traffic impacts from future development on the Midcoast.

Objective 2.1 Evaluate the likely development potential of the Midcoast to identify future impacts to the transportation system and propose measures to offset those impacts.

Objective 2.2 Evaluate the feasibility of developing an in-lieu fee traffic mitigation program so projects can pay to offset traffic impacts.

Objective 2.3 Evaluate the development of a mandatory lot merger program that would reduce development potential by merging adjacently-owned substandard lots.

Objective 2.4 Evaluate the implementation of a lot retirement program for subdivisions to reduce development potential.

Goal 3 Increase opportunities for walking, biking, and riding transit on the Midcoast to provide an alternative to motor vehicles and reduce roadway traffic.

Objective 3.1 Propose pedestrian infrastructure projects that address safety and circulation concerns, while meeting relevant performance standards.

Objective 3.2 Propose bicycle infrastructure projects that address safety and circulation concerns, while meeting relevant performance standards.

Objective 3.3 Identify potential improvements to transit service and bus stops on the Midcoast.

Goal 4: Respect the character of Midcoast communities and protect coastal resources.

Objective 4.1 Integrate community input into plan proposals.

Objective 4.2 Ensure improvements do not detract from the visual character of Midcoast communities.

Goal 5: Maintain and improve access to coastal resources for both residents and visitors.

Objective 5.1 Identify popular Coastside destinations with access issues and propose solutions to improve access.

Objective 5.2 Evaluate project ideas for enhanced shoreline public access.

1.5 Guiding Principles

Several existing community plans and regulatory frameworks have guided the creation of Connect the Coastside, including the:

- California Coastal Act
- San Mateo County Local Coastal Program
- San Mateo County General Plan
- Montara - Moss Beach - El Granada Community Plan
- Highway 1 Safety and Mobility Study

The implementation of Connect the Coastside will continue to be guided by the principles and policies contained in these planning documents.

The *California Coastal Act* contains basic goals that call for protection of the coastal zone environment and maximum public access to the coast. The Coastal Act further recognizes that the public has a right to fully participate in decisions affecting coastal planning and development, and that the planning and

implementation of programs for coastal development should include the widest opportunity for public participation. Recognizing that public understanding and support is important for the success of any planning effort, a guiding principle of the Connect the Coast process was to follow a robust public outreach strategy and incorporate feedback from the public throughout the process.

The *Local Coastal Program (LCP)* contains Policy 2.53, which was the original stimulus for Connect the Coastside. Policy 2.53 requires the development of a comprehensive transportation management plan to address the cumulative traffic impacts of development on the Midcoast:

2.53 Transportation Management Plan

Develop a comprehensive transportation management plan to address the cumulative traffic impacts of residential development, including single-family, two-family, multi-family, and second dwelling units, on roads and highways in the entire Midcoast, including the City of Half Moon Bay. The plan shall be based on the results of an analysis that identifies the total cumulative traffic impact of projected new development at LCP buildout and shall propose specific LCP policies designed to offset the demand for all new vehicle trips generated by new residential development on Highway 1, Highway 92, and relevant local streets, during commuter peak periods and peak recreation periods; and policies for new residential development to mitigate for residential development's significant adverse cumulative impacts on public access to the beaches of the Midcoast region of San Mateo County.

The plan shall thoroughly evaluate the feasibility of developing an in-lieu fee traffic mitigation program, the expansion of public transit, including buses and shuttles, and development of a mandatory lot merger program.

The LCP also includes multiple other related policies, such as those requiring the phased development of public works facilities and requiring every new public works facility or expansion of capacity to go through the coastal development review process. The LCP also limits the expansion of roadway capacity to what is needed to accommodate commuter peak period traffic when buildout of the Land Use Plan occurs. The projects recommended in Connect the Coastside must conform to the Local Coastal Program, so an understanding of the policies of the LCP is important for the creation and implementation of this plan.

The *County of San Mateo General Plan* contains multiple policies related to transportation, including goals for the safe, efficient, and convenient movement of people and for complete streets that create a multimodal transportation system. The General Plan also has a specific policy to “seek methods to mitigate the impact of peak recreational traffic to and along the Coastside” (12.18 Recreational Traffic to the Coastside). The Montara - Moss Beach - El Granada Community Plan is an area plan that is considered part of the General Plan. It includes goals and objectives that direct the development of a road system that is compatible with the small-town character of the Midcoast community.

The *2009 and 2012 Highway 1 Safety and Mobility Studies* used an intensive community-based planning process to engage residents and stakeholders in developing transportation improvement strategies for the Midcoast. Therefore, it was important to consider the goals and findings of the Safety and Mobility Study in the Connect the Coastside plan. The goals of the study included increased safety, more transportation options, efficient traffic circulation, and consideration of both the natural and built contexts.

1.6 Community Input and Engagement

Stakeholder outreach was a critical part of the planning process, to ensure oversight for the assumptions, results of analysis, and final recommendations of the project. Each project deliverable was produced with considerable input from a Technical Advisory Committee, the Midcoast Community Council, an online public survey and public workshops.

Connect the Coastside also builds on the community outreach that informed the Highway 1 Safety and Mobility Study. This community-based process developed many of the ideas for transportation improvements included in Connect the Coastside. Community members provided input on Connect the Coastside by attending public workshops and presentations to the Midcoast Community Council, the Planning Commission and Board of Supervisors. In addition, an online survey was conducted to provide community members multiple ways to comment.

To engage specific stakeholders, the County formed a Technical Advisory Committee. Members of the committee met six times during the course of the project to provide input. The Technical Advisory Committee included representatives from transportation, infrastructure and public safety agencies, schools, businesses and community organizations. Members reviewed and helped refine plan proposals prior to public meetings and workshops.

Community input was instrumental in shaping the proposals in the plan. In addition to the community ideas from the Highway 1 Safety and Mobility Study, community ideas incorporated in Connect the Coastside include new transportation performance standards that avoid widening of Highway 1, a preference for roundabouts for any new intersection control, maintaining flexibility in the location of pedestrian crossings, limiting paving of the Coastal Trail, park and ride lots for transit riders, and additional transit service on weekdays and weekends.

1.7 Summary of Key Improvements

Connect the Coastside aims to improve transportation safety and mobility for Coastside residents by:

- Increasing transportation choices
- Making travel safer for pedestrians and cyclists
- Improving traffic flow at bottlenecks
- Increasing use of public transit

The transportation investments outlined in Connect the Coastside address the present and future mobility needs of Coastside communities. Near-term projects will increase transportation choices for residents and visitors. Bike lanes, trail improvements and crossings will make it easier and safer for people to walk or take their bike. Investments in bus stops and expanded weekend bus service will help reduce traffic and encourage people to take public transit. Traffic calming, turn lanes, and intersection improvements will make roadways safer and less congested.

The plan also ensures that Coastside communities are better prepared to meet future transportation needs. As new development occurs, additional transportation improvements will be constructed to address traffic impacts. New land use policies will also help reduce traffic and preserve coastal community character by limiting development. Lot mergers and lot retirements will concentrate development, helping to protect natural resources and preserve open space. A new traffic fee mitigation program would collect money from new Coastside development to help pay for future transportation improvements.

1.7.1 Infrastructure Improvements

Several infrastructure improvements for each mode were selected from a list of potential improvements compiled from the Technical Advisory Committee and community-suggested improvements, proposed projects identified in recent, relevant reports, as well as improvements suggested by the consultant team to address identified deficiencies. The final list of improvements in **Table 1** below were selected based on their feasibility, cost, ability to address deficiencies, and consistency with the LCP, including environmental considerations.

The following improvements will have the greatest impact on current traffic and safety conditions:

- Moss Beach corridor roundabouts
- Pedestrian crossings throughout the study area
- The Multi-modal Trail
- Increased transit service
- Transit-related park-and-ride lots

Connect the Coastside will enhance the quality of life for Coastside residents by making it safer for pedestrians to cross Highway 1, slowing traffic in Moss Beach through the installation of roundabouts, and installing acceleration lanes at key parking sites and intersections.

The plan will improve access to beaches and coastal recreational opportunities through proposed parking and coastal trail improvements, left turn pockets on SR-92, and the Multi-modal trail. Expanded transit service and roadway improvements would also help improve traffic flow on weekends for residents and visitors alike. The improvements in each community are specified in **Table 1**.

Table 1. Infrastructure Improvements Identified in Connect the Coastside

| Mode | Project Name | Location | Project Source ¹ |
|----------|---|----------------|---|
| Roadways | Gray Whale Cove Turn and Acceleration Lanes | Midcoast | Highway 1 Safety and Mobility Improvement Study and Highway 1 Congestion & Safety Improvement Project |
| | SR-1 Side-Street Stop Signs | El Granada | Connect the Coastside |
| | SR-1 Shoulder Treatment (Village Zones) | El Granada | Highway 1 Safety and Mobility Improvement Study |
| | SR-1 Shoulder Treatment (Fringe Zones) | El Granada | Highway 1 Safety and Mobility Improvement Study |
| | California Avenue Intersection Control (Signal or Roundabout) | Moss Beach | Connect the Coastside |
| | Cypress Avenue Intersection Control (Signal or Roundabout) | Moss Beach | Big Wave and Connect the Coastside deficiency |
| | 16th Street Roundabout | Moss Beach | Cypress Point Development |
| | Carlos Street Terminus Realignment | Moss Beach | |
| | SR-92/SR-35 Roundabout | SMC | Connect the Coastside |
| | Carlos Street Traffic Calming | Moss Beach | Highway 1 Safety and Mobility Improvement Study |
| | Main Street Traffic Calming | Montara | |
| | SR-92 Passing/Climbing Lanes | Unincorporated | Connect the Coastside |
| | SR-92 Truck Signs | SMC | Connect the Coastside |
| | SR-92 Left Turn Lanes | SMC | |

¹ Project Source identifies the public study or planning report from which each proposed project concept was developed.

Table 2. (cont.) Infrastructure Improvements Identified in Connect the Coastside

| Mode | Project Name | Location | Project Source |
|------------|--|--|---|
| Pedestrian | Striped Pedestrian Crossing with Beacons | Various | Highway 1 Safety and Mobility Improvement Study |
| | SR-1 Multi-modal Trail | El Granada and Moss Beach | Highway 1 Safety and Mobility Improvement Study |
| | SR-1 Multi-modal Trail | Moss Beach, El Granada, Half Moon Bay | |
| | SR-1 Multi-modal Trail | El Granada, Half Moon Bay, Montara, Moss Beach | |
| | Coronado Street and Obispo Road Sidewalk | El Granada | Connect the Coastside |
| | Coastal Trail | All | Highway 1 Safety and Mobility Improvement Study |
| | Bay Area Ridge Trail Crossing of SR- 92- Project Study | SR-92 at Upper SR-35 Intersection | Caltrans, County, San Francisco Public Utilities Commission |
| Bike | Capistrano Road Bicycle Facilities | Princeton | Highway 1 Safety and Mobility Improvement Study |
| | SR-92 Bike Lanes | SMC | |
| | SR-1 Bike Lanes | ALL | |
| | Airport Street Class I/II/III Bike Routes | Moss Beach | Connect the Coastside |
| | Bike/Ped improvements in downtown Moss Beach | Moss Beach | Connect the Coastside |
| Transit | Bus Stop Amenities | Various | Connect the Coastside |
| | Increased Weekend and Commute SamTrans Service | Various | |
| | Transit Stop Improvements | Moss Beach | Connect the Coastside |

Table 3. (cont.) Infrastructure Improvements Identified in Connect the Coastsides

| Mode | Project Name | Location | Project Source |
|---------|--|------------|--|
| Parking | Montara State Beach Parking Lot Improvements | SMC | Coastside Access Study |
| | Upper Gray Whale Cove Parking Lot Improvements | SMC | |
| | Wayfinding | ALL | |
| | Carlos Street On-Street Parking | Moss Beach | Highway 1 Safety and Mobility Improvement Study |
| | El Granada Diagonal Parking | El Granada | Highway 1 Safety and Mobility Improvement Study Phase 1 |
| | Park and Ride at Etheldore St (South Terminus) | Moss Beach | Highway 1 Safety and Mobility Improvement Study/ SamTrans Coastside Plan |

1.8 Land Use Policies

Land use patterns have a significant impact on travel patterns. In general, the Midcoast has a predominantly low density, suburban residential settlement pattern with small commercial areas adjacent to Highway 1 in each of the Midcoast communities. This settlement pattern and a range of other factors (the configuration of local streets, the limited access provided by Highway 1 and State Route 92, the dearth of multi-modal transportation choices) encourage automobile trips. The transportation improvements envisioned in Connect the Coastsides will expand mobility choices, while land use strategies to limit development can serve to reduce future traffic demand.

The lot merger program, lot retirement program and traffic fee mitigation program are strategies that can reduce future development potential, or in the case of in-lieu fees, provide a funding source for in Connect the Coastsides’ transportation improvements. The Midcoast community has expressed considerable support for policies that would limit future development to preserve the rural character of the Midcoast and moderate future traffic demand.

The lot merger program could reduce the development potential of existing single-family neighborhoods and result in some larger lots with more on-site, private open space. The lot retirement program will limit the development potential of rural lands on the Midcoast, preserving additional open space and natural resources. These programs support Coastal Act policies, such as concentrating development, protecting natural resources and protecting public access to coastal resources by limiting development and thereby reducing traffic.

A traffic fee mitigation program would collect fees for new residential and non-residential development on a per-housing-unit basis for residential and per-square-foot basis for non-residential development. The rates would be based on a specified list of projects needed to mitigate the impacts of the growth, the total estimated capital cost of those projects and the amount of new development expected. In addition to helping fund improvements proposed by this plan, a traffic fee mitigation program would also serve as a potential check on development.

1.9 Conclusion

Connect the Coastsides is a community-based plan containing transportation infrastructure proposals and land use policy options intended to improve mobility and safety for Coastsides residents and visitors. This plan will require ongoing community engagement to refine the infrastructure proposals into detailed designs, to ensure the guiding principles for implementation are adhered to and to advocate for funding necessary to construct these improvements.

2 EXISTING CONDITIONS AND TRANSPORTATION PERFORMANCE STANDARDS

Mobility in the Study Area is provided by the roadway, bikeway, transit and trail networks. The principal roadway corridors in the Study Area include Cabrillo Highway (Highway 1), in the north-south direction and San Mateo Road (SR-92) in the east-west direction. Highway 1 and SR-92 are owned and managed by Caltrans and provide regional connections to San Francisco (north), San Mateo (east) and Santa Cruz (south). The remainder of the roadway network is comprised of two-lane County roads that range from arterials, such as Airport Street in Princeton-Moss Beach, to narrow rural lanes, such as Beach Way in Moss Beach.

The roadway network serves to connect land uses and facilitates movement of persons and goods to and from, within, and through the region, with Highway 1 and SR-92 accommodating traffic and goods movement at higher speeds and all other roadways serving neighborhoods with narrower widths and lower speeds. The roadway network provides regional and local access to coastal resources, including beaches, marine reserves, harbors, surf breaks and other destinations. The current roadway network has no formal bicycle facilities, and cyclists share the roadways with motorists. The non-motorized trail network is discontinuous and primarily serves recreational users. Transit service in the area is provided by SamTrans, with links from Half Moon Bay to north County BART stations.

Each section in this chapter begins with the performance standards used to characterize the performance for every mode and facility under both existing and projected future conditions. Next, existing conditions and deficiencies are described to set the stage for consideration of conditions under projected future development (Chapter 3), and the improvements needed to ensure that each mode of travel can meet the performance standards (Chapter 4).

Roadway standards are better developed than those for other modes. The innovative design and operating standards for bicycle and pedestrian facilities in Connect the Coastside support the creation of a safe and comprehensive network that is fully integrated with the existing roadways. The standards are based, in part, upon traffic volume thresholds and potential pedestrian and bicycle demand from nearby land use. These standards assist in the identification of desired or necessary improvements for low, medium and high priority pedestrian or bicycle mobility. Transit performance standards are based on bus capacity utilization to prioritize bus routes and bus stops and standards for bus stop amenities and improvements. Some metrics used to determine whether to provide additional service and provision of bus stop amenities may change if SamTrans adopts a new Service Policy Framework in the spring of 2020.

2.1 Automobile Operations

2.1.1 Existing Operational Standards

The existing Level of Service (LOS) standards are based upon countywide and regional standards defined in the following three documents:

- *San Mateo County Congestion Management Program (SMC CMP)*²

² San Mateo County Congestion Management Program, 2011, San Mateo City/County Council of Governments (C/CAG)

- *San Mateo County Traffic Impact Study Requirements*³
- *Local Coastal Program (LCP)*⁴

2.1.1.1 Intersection Level of Service

Vehicle circulation concerns relate primarily to times of peak roadway use: the commute period and weekend recreational use, outside of the significant traffic demand generated during key events. Estimates of level of service (LOS) for key intersections along Highway 1 and SR-92 are provided in **Table 7** for the Weekday AM peak period (7AM-9AM) and PM peak period (4PM-6PM) and the Weekend Midday recreational peak period (10AM-2PM) conditions based on counts taken in 2012 and 2014.

LOS analysis was conducted using the criteria described in the City/County Association of Governments (C/CAG) 2011 San Mateo County Congestion Management Program (SMC CMP) and the County’s Local Coastal Program. LOS as defined in the Highway Capacity Manual (HCM) is a quality measure describing operating conditions within a traffic stream. It is generally described in such service measures terms as speed and travel time, freedom to maneuver, traffic interruptions, and comfort and convenience. The LOS evaluation indicates the degree of congestion that occurs during peak travel periods and is the principal measure of roadway and intersection performance. LOS can range from “A” representing free-flow conditions, to “F” representing extremely long delays. LOS D is typically considered acceptable for a peak hour in urban areas. LOS E is approaching capacity and LOS F represents conditions at or above capacity. LOS definitions, considering vehicle delay for signalized and unsignalized intersections, are shown in **Table 4**.

Table 4 - Level of Service Thresholds and Definitions

| Level of Service | Average Control Delay (sec/veh) | | Description |
|------------------|---------------------------------|---|--------------------------------------|
| | Signalized Intersections | Unsignalized Intersections ¹ | |
| A | ≤ 10 | ≤ 10 | Free flow/Insignificant Delay |
| B | > 10 and ≤ 20 | > 10 and ≤ 15 | Stable Operation/Minimal Delay |
| C | > 20 and ≤ 35 | > 15 and ≤ 25 | Stable Operation/Acceptable Delay |
| D | > 35 and ≤ 55 | > 25 and ≤ 35 | Approaching Unstable/Tolerable Delay |
| E | > 55 and ≤ 80 | > 35 and ≤ 50 | Unstable Operation/Significant Delay |
| F | > 80 | > 50 | Forced Flow/Excessive Delay |

Source: 2000 *Highway Capacity Manual, Transportation Research Board, 2000*.

Notes: ¹Worst Approach Delay (in seconds per vehicle) for Unsignalized Intersections

³ San Mateo County Traffic Impact Study Requirements, 2013, County of San Mateo, Department of Public Works, Roadway Services

⁴ County of San Mateo Local Coastal Program Policies, 2013, County of San Mateo, Planning and Building Department

2.1.1.2 Roadway Level of Service

Roadway segment analysis was conducted using the thresholds described in the City/County Association of Governments (C/CAG) Congestion Management Program. According to the SMC CMP, roadway capacity for multilane highways is assumed to be 2,200 vehicles per lane, per hour while capacity is 1,400 vehicles per lane per hour for two lane highways. Roadway segment LOS is defined based upon the peak traffic volume (v) relative to the capacity of the roadway or intersection (c). This is expressed as a v/c ratio and the amount of capacity filled by traffic volumes determines the level of service. If the demand (volume) exceeds the capacity (a v/c ratio greater than 1.0), traffic flow is unstable and excessive delay and queuing is expected. The County’s LCP Transportation policy 2.43 provides that when considering roadway expansion, level of service “D” is acceptable for peak periods, and Level of Service “E” is acceptable during recreational peak periods.

2.1.1.2.1 Two Lane Highway

Additionally, the SMC CMP defines a two-lane highway as a two-lane roadway with one lane for use by traffic in each direction. Passing of slower vehicles requires use of the opposing lane. As volumes or geometric constraints increase, the ability to pass decreases and platoons of vehicles are formed, increasing the delay experienced by motorists. The LOS for two-lane highways is based on mobility.

For two-lane highways, the volume and capacity used to calculate the v/c ratio combines both directions. The capacity for two-lane roads used is 2,800 vehicles per hour (1,400 vehicles per lane per hour in each direction) The specific LOS criteria used in the SMC CMP are based on thresholds from the Highway Capacity Manual (HCM) and are shown in **Table 5**.

Table 5 - Level of Service Criteria for Two-Lane Highways

| LOS | % Time Delay | Max v/c ratio ¹ | Average Travel Speed ² |
|-----|--------------|----------------------------|-----------------------------------|
| A | 30 | 0.00 – 0.04 | 54 |
| B | 45 | 0.04 – 0.16 | 51 |
| C | 60 | 0.16 – 0.32 | 48 |
| D | 75 | 0.32 – 0.57 | 46 |
| E | >75 | 0.57 – 1.00 | 41 |
| F | 100 | > 1.00 | < 41 |

Sources: San Mateo County Congestion Management Program, 2011

Notes:

1. Ratio of flow rate to an ideal capacity of 2,800 passenger cars per hour in both directions.

2. Average travel speed of all vehicles for highways with design speed 60 mph; for highways with lower design speeds, reduce speed by 4 mph for each 10-mph reduction in design speed below 60 mph; assumes that speed is not restricted to lower values by regulation.

2.1.1.2.2 Multilane Highway

There are some segments along SR-92 in the study area which have more than one lane in each direction. For those segments, the level of service criteria is for each direction separately. The specific LOS criteria used in the SMC CMP are based on thresholds from the HCM as shown in **Table 6**. The capacity used for multilane highway segments is 1,100 vehicles per hour per lane and is evaluated per lane and per direction, so a four-lane highway has a 4,400 vehicle per hour total capacity.

Table 6 - Level of Service Criteria for Multi-Lane Highways

| LOS | % Time Delay | Max v/c ratio ¹ | Average Travel Speed ² |
|-----|--------------|----------------------------|-----------------------------------|
| A | 30 | 0.00 – 0.30 | 50 |
| B | 45 | 0.30 – 0.50 | 50 |
| C | 60 | 0.50 – 0.70 | 50 |
| D | 75 | 0.70 – 0.84 | 49 |
| E | >75 | 0.84 – 1.00 | 47 |
| F | 100 | > 1.00 | < 47 |

Sources: San Mateo County Congestion Management Program, 2011

2.1.2 Proposed Operational Standards

In some cases, the existing standards generate solutions the community does not support, so these standards are outdated and lack the necessary nuance for the constrained transportation network, and potential improvements for the Midcoast. This document proposes updated metrics that are designed to better describe the ability of the transportation system in the Midcoast to accommodate growth across all transportation modes. Using the proposed metrics, this document then provides operating and design standards that combine existing LOS standards with standards that measure mobility and accessibility as well as how non-vehicular modes interact with traffic.

The majority of intersections evaluated within the study area are unsignalized minor approach roads intersecting with Highways 1 and 92 and are controlled by stop signs for minor approaches, or are uncontrolled. Therefore, any identification of deficiency or required mitigation should balance the need of the minor street traffic with the flow of traffic along Highway 1 and SR-92. To address this, the proposed intersection standard for the Midcoast requires unsignalized intersections to meet a Caltrans signal warrant to ensure that there is sufficient volume of traffic using the minor approach to warrant additional intersection control and the associated disruption to traffic flow along Highway 1.

The existing roadway network does not provide any alternative routes to Highway 1 or SR-92 with comparable capacity. To avoid mitigations that require additional lanes to increase capacity, the proposed roadway standard for the Midcoast in Connect the Coastside is based on travel time and expressed as an index of delay, allowing for a range of mitigations which can be focused on specific trouble spots and allowing modification of the threshold for multimodal corridors.

2.1.2.1 Intersection – LOS with change in standard

Signalized intersections have a standard of LOS C with no individual movement operating at worse than LOS D as defined by the *San Mateo County Traffic Impact Study Requirements* and the County's LCP. Unsignalized intersections not meeting the LOS standard are also required to meet a Caltrans peak hour signal warrant⁵ to be considered deficient.

2.1.2.2 Roadway Segment – Delay Index

Connect the Coastside evaluates traffic delay on the Highway 1 and SR-92 corridors using a Delay Index, a standard defined as the ratio of peak period travel time on a corridor to the free-flow travel time. The maximum acceptable Delay Index was set at 2.0 and calculated for the Highway 1 corridor and SR-92 corridor in the Study Area during the Weekday AM and PM commute peak hours and during the Weekend midday recreational peak hour. Under the delay index, a corridor that took 10 minutes to drive with no congestion would be deficient if it took over 20 minutes to drive during peak commute times. Connect the Coastside proposes that the Delay Index be increased to 3.0 for segments that have adjacent Class I bicycle facilities or Class II bicycle facilities along at least 80% of the length. This higher standard increases delay for motorists, but encourages improvements that provide mobility across multiple modes rather than road widening and is in accordance with statewide and County General Plan Complete Streets policies that encourage provision of capacity for all modes of travel.

2.1.3 Existing Conditions

2.1.3.1 Intersection Level of Service

Existing Condition LOS was calculated for 48 key intersections located within the Study Area. Most of the Study intersections are located along Highway 1 and SR-92. Delay and LOS are provided for Weekday AM and PM peak hour and Weekend peak recreational hour in **Table 7**. The operating standard for each intersection is also provided.

2.1.3.2 Corridor Delay

A Delay Index was calculated for the Highway 1 corridor extending the entire length of the Study Area and for SR-92 from west of Landfill Road to east of SR-92/SR-35 (Lower) intersection. The majority of Highway 1 intersections are uncontrolled, resulting in low off-peak free-flow travel times. Although certain segments have slower traffic during peak hours, the entire corridor is evaluated as one segment for the purpose of calculating Delay Index and determining impact of growth. This approach reflects that most of the delay occurs at intersections, which have a separate (LOS) evaluation metric and standard. Measures to reduce delay for these two highway segments are therefore best implemented and effective at the corridor level. Delay Index and Travel Times are provided for Weekday AM and PM peak hour and Weekend peak recreational hour for these two segments in **Table 8**. For comparison, the segment LOS

⁵ California MUTCD 2012, Chapter 4

under the v/c criteria are also provided. Analysis of intersection and corridor delay is discussed below in Section 2.1.4.

Table 7 - Existing Conditions Peak Hour Intersection Level of Service

| Street Names | Control Type | Operating Standard ¹ | AM Peak Hour | | | PM Peak Hour | | | Weekend Peak Hour | | |
|-----------------------------------|--------------|---------------------------------|--------------------|----------|---------------|--------------------|----------|---------------|--------------------|----------|---------------|
| | | | Delay ² | LOS | Meets Warrant | Delay ² | LOS | Meets Warrant | Delay ² | LOS | Meets Warrant |
| HWY 1 / 2nd St | TWSC | C(D) & Warrant | 16.1 (WB) | C | N | 15.7 (WB) | C | N | 22.4 (WB) | C | N |
| HWY 1 / 7th St | TWSC | C(D) & Warrant | 12.6 (EB) | B | N | 13.0 (EB) | B | N | 14.8 (EB) | B | N |
| HWY 1 / 8th St | TWSC | C(D) & Warrant | 18.7 (WB) | C | N | 32.5 (WB) | D | N | 45.3 (WB) | E | N |
| SR -1 / 16th St | TWSC | C(D) & Warrant | 31.6 (EB) | D | N | 39.5 (EB) | E | N | 42.6 (WB) | E | N |
| SR -1 / Carlos St | TWSC | C(D) & Warrant | 12.3 (WB) | B | N | 12.1 (WB) | B | N | 12.7 (WB) | B | N |
| HWY 1 / Vallemar St | TWSC | C(D) & Warrant | 17.6 (EB) | C | N | 24.5 (WB) | C | N | 21.8 (WB) | C | N |
| HWY 1 / California Ave | TWSC | C(D) & Warrant | 25.6 (WB) | D | N | 44.4 (WB) | E | N | >50 (WB) | F | N |
| HWY 1 / Virginia Ave | TWSC | C(D) & Warrant | 22.6 (WB) | C | N | 38.5 (WB) | E | N | >50 (WB) | F | N |
| HWY 1 / Vermont Ave (WB) | TWSC | C(D) & Warrant | 27.5 (WB) | D | N | 45.0 (WB) | E | N | >50 (EB) | F | N |
| HWY 1 / Cypress Ave (EB) | TWSC | C(D) & Warrant | 44.2 (EB) | E | N | >50 (WB) | F | N | >50 (EB) | F | N |

| Street Names | Control Type | Operating Standard ¹ | AM Peak Hour | | | PM Peak Hour | | | Weekend Peak Hour | | |
|---------------------------------|--------------|---------------------------------|--------------------|----------|---------------|--------------------|----------|---------------|--------------------|----------|---------------|
| | | | Delay ² | LOS | Meets Warrant | Delay ² | LOS | Meets Warrant | Delay ² | LOS | Meets Warrant |
| HWY 1 / St Etheldore St (South) | TWSC | C(D) & Warrant | 23.2 (WB) | C | N | 34.1 (WB) | D | N | 37.1 (WB) | E | N |
| HWY 1 / Capistrano Rd (North) | TWSC | C(D) & Warrant | 17.4 (EB) | C | N | 22.1 (EB) | C | N | 30.6 (EB) | D | N |
| HWY 1 / Coral Reef Ave | TWSC | C(D) & Warrant | 16.3 (WB) | C | N | 24.5 (WB) | C | N | 28.7 (WB) | D | N |
| HWY 1 / Capistrano Rd (South) | Signalized | C(D) | 19.1 | B | N/A | 17.5 | B | N/A | 20.7 | C | N/A |
| HWY 1 / Coronado St | Signalized | C(D) | 21.7 | C | N/A | 14.4 | B | N/A | 11.4 | B | N/A |
| Obispo Rd / Coronado St | TWSC | C(D) & Warrant | 12.9 (EB) | B | N | 10.2 (WB) | B | N | 12.3 (WB) | B | N |
| HWY 1 / Magellan Ave | TWSC | C(D) & Warrant | >50 (EB) | F | N | >50 (EB) | F | N | >50 (EB) | F | N |
| HWY 1 / Medio Ave | TWSC | C(D) & Warrant | >50 (WB) | F | N | >50 (WB) | F | N | >50 (WB) | F | N |
| HWY 1 / Miramar Dr | TWSC | C(D) & Warrant | 21.3 (EB) | C | N | >50 (EB) | F | N | 46.9 (EB) | E | N |
| SR-92 / Landfill Rd | TWSC | C(D) & Warrant | >50 (SB) | F | N | >50 (SB) | F | N | 33.5 (SB) | D | N |

| Street Names | Control Type | Operating Standard ¹ | AM Peak Hour | | | PM Peak Hour | | | Weekend Peak Hour | | |
|-------------------------------------|--------------|---------------------------------|--------------------|----------|---------------|--------------------|----------|---------------|--------------------|----------|---------------|
| | | | Delay ² | LOS | Meets Warrant | Delay ² | LOS | Meets Warrant | Delay ² | LOS | Meets Warrant |
| SR-92 / Skyline Blvd (Upper) | TWSC | C(D) & Warrant | 35.5 (NB) | E | N | >50 (NB) | F | N | >50 (NB) | F | N |
| SR-92 / SR-35 (Lower) | Signalized | C(D) | 11.7 | B | N/A | 22.0 | C | N/A | 41.9 | D | N/A |

¹ Standards provided within parenthesis are for individual movements.

² Signalized intersections and all-way stop controlled (AWSC) intersections are reported by the average delay and LOS for the intersection; two-way stop controlled (TWSC) intersections are reported with the worst approach's delay and LOS. Bolded intersections fall below the defined LOS standard.

Table 8. Existing Conditions Peak Hour Roadway Segment Delay Index

| Corridor and Direction | Operating Standard | Freeflow Travel Time (mm:ss) | AM Peak Hour | | PM Peak Hour | | Midday Peak Hour | | |
|---|--------------------|------------------------------|---------------------|-------------|---------------------|-------------|---------------------|-------------|-------------|
| | | | Travel Time (mm:ss) | Delay Index | Travel Time (mm:ss) | Delay Index | Travel Time (mm:ss) | Delay Index | |
| NB Highway 1 from Mirada Road to 1 st Street | 2.0 | 06:30 | 07:29 | 1.15 | 7:51 | 1.21 | 7:56 | 1.22 | |
| <i>Mirada Rd to Capistrano Rd (N)</i> | <i>N/A</i> | <i>02:37</i> | <i>03:14</i> | <i>1.24</i> | <i>3:36</i> | <i>1.38</i> | <i>3:39</i> | <i>1.4</i> | |
| <i>Capistrano Rd (N) to 16th St</i> | <i>N/A</i> | <i>02:59</i> | <i>03:15</i> | <i>1.09</i> | <i>3:18</i> | <i>1.11</i> | <i>3:18</i> | <i>1.11</i> | |
| <i>16th St to 1st St</i> | <i>N/A</i> | <i>00:54</i> | <i>01:00</i> | <i>1.11</i> | <i>0:57</i> | <i>1.04</i> | <i>1:00</i> | <i>1.1</i> | |
| SB Highway 1 from 1 st Street to Mirada Road | 2.0 | 06:30 | 07:22 | 1.14 | 7:37 | 1.18 | 7:50 | 1.21 | |
| <i>1st St to 16th St</i> | <i>N/A</i> | <i>01:00</i> | <i>01:00</i> | <i>1.00</i> | <i>1:06</i> | <i>1.11</i> | <i>1:07</i> | <i>1.12</i> | |
| <i>16th St to Capistrano Rd (N)</i> | <i>N/A</i> | <i>03:00</i> | <i>03:12</i> | <i>1.07</i> | <i>3:15</i> | <i>1.09</i> | <i>3:22</i> | <i>1.13</i> | |
| <i>Capistrano Rd (N) to Mirada Rd</i> | <i>N/A</i> | <i>02:30</i> | <i>03:10</i> | <i>1.27</i> | <i>3:16</i> | <i>1.31</i> | <i>3:21</i> | <i>1.35</i> | |
| SR-92 from Half Moon Bay city limits to I-280 on-ramp | NB | 2.0 | 08:42 | 13:16 | 1.52 | 13:04 | 1.50 | 13:05 | 1.50 |
| | SB | | | 12:48 | 1.47 | 15:32 | 1.79 | 19:02 | 2.19 |

Note: **Bolded** corridors fall below the defined LOS standard.

2.1.3.3 Recreational Parking

To ensure adequate parking at Coastside recreational destinations and to avoid overcrowding local street parking, the Midcoast parking standard is based on parking occupancy at the recreational parking facilities. Sufficient recreational parking needs to be provided, and when necessary the parking supply should be expanded to meet the standard, in part to prevent overflow parking into residential areas.

Beach access points should have no more than 85% parking occupancy in the associated recreational parking facility during an average weekend midday peak. These facilities and associated recreational destinations include:

- Gray Whale Cove Upper Lot – Gray Whale Cove State Beach
- Montara Beach Paved Parking – Montara Beach
- Rancho Corral de Tierra Parking – open space park
- Surfer’s Beach Parking – Surfer’s Beach

2.1.4 Existing Vehicular Transportation Problems and Deficiencies

Many Coastside residents feel that the traffic during peak hours and during nicer weather on weekends is intolerable. Identifying deficiencies in the transportation system is vital to prioritizing improvements according to community needs and investing capital funds responsibly. This section provides a detailed analysis of existing issues and deficiencies in the Midcoast area of San Mateo County.

2.1.4.1 Intersection LOS

All signalized intersections within the Midcoast region operate above the LOS C standard; however, several unsignalized intersections along Highway 1 have minor street approaches that operate below the LOS D standard during commuter peak periods or below LOS E during recreation peak periods. The following intersections do not meet the LOS standard during the listed peak hours:

- Highway 1 and 8th Street (Weekend)
- Highway 1 and California Avenue (PM, Weekend)
- Highway 1 and Virginia Avenue (PM, Weekend)
- Highway 1 and Vermont Avenue (PM, Weekend)
- Highway 1 and Cypress Avenue (AM, PM, Weekend)
- Highway 1 and Magellan Avenue (AM, PM, Weekend)
- Highway 1 and Medio Avenue (AM, PM, Weekend)
- Highway 1 and Miramar Drive (PM)

All of the intersections that operate below the standard are minor-street, stop-controlled and only have one lane of approach and only Cypress Avenue has more than 50 vehicles per hour on an approach turning onto Highway 1. None of the intersections operating below the standard would meet the peak hour signal warrant, except Cypress Avenue, which currently meets a signal warrant.

Between Half Moon Bay City limit and I-280, the following study intersections operate at LOS F:

- SR-92 and Ox Mountain Landfill Road (AM, PM)
- SR-92 and Skyline Boulevard (AM, PM, Weekend)

Ox Mountain Landfill Road has very low traffic volumes entering SR-92. Skyline Boulevard has a channelized-yield right turn onto SR-92 and less than 50 vehicles turning left onto SR-92. Neither intersection would meet the peak hour signal warrant under current conditions.

2.1.4.2 Roadway Segment Delay

Under existing conditions, both directions of travel along Highway 1 show a Delay Index within the standard of 2.0 for all time periods. While discrete segments along Highway 1 are not held to any defined standard, it can also be noted that none of them currently exceed the standard. For SR-92, the standard is only exceeded in the southbound direction during the weekend midday period. The other directions and time periods are within the Standard.

2.1.4.3 Recreational Parking Utilization

A recreational parking supply inventory and parking utilization survey was conducted during Summer and Fall 2014 as part of the *San Mateo County Coastside Access Study*. An inventory of available parking areas was conducted adjacent to every park site along the Coastside between Devil’s Slide to the north and El Granada to the south. The entire study corridor has an estimated 396 parking spaces in designated parking areas. When designated lots fill to capacity, visitors sometimes park on the roadside. While such parking is legally permitted as long as the vehicle is outside the right of travel way, it is not recommended. These vehicles parked on the roadside adjacent to designated or informal parking areas were counted as “overflow” parking to the designated parking area and were therefore counted as occupancy in excess of the designated capacity.

During the data collection period, a total of 423 parked vehicles were observed in the designated and overflow parking areas during maximum occupancy, leading to a utilization rate of 107% of the formal spaces. Out of all study locations, the highest overflow was observed at Montara State Beach and McNeen Ranch (137%), while Quarry, Wicklow and Mirada Surf saw the least occupancy rate at 26%. Many parking spaces are not striped and therefore may lead to inefficient use of the lots.

2.2 Pedestrian Movements

2.2.1 Performance Standards

Highway 1 and SR-92 create significant barriers for pedestrian crossings, and do not provide for safe, comfortable north-south or east-west pedestrian movement. Community members believe this precludes people from walking, despite a strong desire for safe walking routes. Midcoast residents embrace appropriate design and operating standards that lead to improvements that create a safe network for pedestrians that provides mobility and accessibility. The proposed volume-based pedestrian facility standard for the Midcoast, as defined below, focuses on key areas where there is a potential for pedestrian demand and identifies locations where pedestrian facilities are needed based on the level of adjacent vehicle volume, popular destination locations, e.g., beaches and parking areas; and shopping and service areas. The pedestrian crossing standard is aimed at improving pedestrian safety by providing enough pedestrian crossings along key highway segments to minimize uncontrolled crossing while still minimizing disruptions to vehicular traffic flows.

Other pedestrian goals include improving north-south & east-west pedestrian mobility along the highway corridors; improving the ability of residents to walk within and between town centers, including to transit

stops; and increasing pedestrian safety on Highway 1 and SR-92 during special events, such as the Pumpkin Festival or Mavericks surf competition.

2.2.1.1 Volume-based Pedestrian Amenity Standards

Pedestrian demand has been characterized as low, medium or high, and hotspots. Low demand correlates to roadways in the vicinity of vacant land. Medium to high pedestrian demand occurs along commercial and residential land use areas, while hotspots are key destinations that experience a high density of local as well as visiting pedestrians, such as beaches, commercial areas and view-points.

Based on the level of pedestrian demand and peak hour traffic volumes, pedestrian facility standards have been formulated to generate and evaluate infrastructure improvement recommendations. The standards and improvements are described in **Table 9** and **Table 10** . These Volume-based Amenity Standards do not warrant pedestrian improvements for areas with low pedestrian demand. Intersection and street segment improvements are needed at locations with pedestrian-oriented land use and at key destinations. At locations with medium to high pedestrian demand and at hot spots, safe pedestrian crossing locations should be located no more than a half mile apart. As traffic volumes increase, the level of improvement needed to provide a safe crossing increases, from a simple marked crosswalk at the lowest traffic volumes, to a high visibility crosswalk with curb extensions and a pedestrian activated signal or beacon at locations with high traffic volumes. The Multi-modal Trail is the primary improvement serving north-south pedestrian movements. The full listed of recommendations is depicted below.

Table 9 - Recommended Intersection Improvement Standards for Pedestrian Facilities

| Traffic Volumes (veh/hr) | Suggested Improvements | Pedestrian Demand | | |
|-----------------------------|---|-------------------|---------------------------------|------------------------------------|
| | | Low (Empty) | Medium to High (Land Use) | Hot Spots (Key Destinations) |
| 0-800 | Crosswalk (two parallel stripes only) | | X | X |
| 800-1600 | Crosswalk | | X | X |
| | Ladder Crosswalk (higher visibility) | | | X |
| | Intersection Lighting | | X | X |
| | Pedestrian Signal/PPB (Sig) | | X | X |
| | Countdown in Signal (Sig) | | | X |
| 1600-2000 | Crosswalk | | X | X |
| | Ladder Crosswalk | | X | X |
| | Intersection Lighting | | X | X |
| | Pedestrian Signal/PPB (Sig) | | X | X |
| | Countdown in Signal (Sig) | | X | X |
| | Beacon Signs for Pedestrians (Unsig) | | | X |
| > 2000 | Crosswalk | | X | X |
| | Ladder Crosswalk | | X | X |
| | Intersection Lighting | | X | X |
| | Pedestrian Signal/PPB (Sig) | | X | X |
| | Countdown in Signal (Sig) | | X | X |
| | Beacon Signs for Pedestrians (Unsig) | | X | X |
| | Curb Extensions | | | X |
| | Median Refuge (4+ lanes) | | | X |

(Sig) = Signalized
(Unsig) = Unsignalized

Table 10 - Recommended Street Segment Improvement Standards for Pedestrian Facilities

| Traffic Volumes (veh/hr) | Suggested Improvements | Pedestrian Demand | | |
|-----------------------------|---------------------------|-------------------|------------------------------|------------------------------------|
| | | Low (Empty) | Medium to High (Land Use) | Hot Spots (Key Destinations) |
| 0-800 | Walkways | | X | X |
| 800-1600 | Walkways | | X | X |
| | Curb | | X | X |
| | Ped scale street lighting | | X | X |
| > 1600 | Walkways | | X | X |
| | Curb | X | X | X |
| | Ped scale street lighting | | X | X |
| | Presence of buffer | | | X |

2.2.2 Existing Conditions and Deficiencies

The pedestrian network in western San Mateo County is generally comprised of local sidewalks, intermittent crossings at signalized town intersections, and the public multi-use trail system. The Midcoast Multi-Modal Trail (Trail), is a bicycle and pedestrian commuter trail that will provide an alternative means of transportation for residents of the Midcoast to safely access neighboring communities, town centers, schools and recreational destinations without having to travel on the highway. The Trail, commonly referred to as the Parallel Trail, was conceptualized in the community- developed Highway 1 Safety and Mobility Improvement Study, Phase 1. The Trail will be separated from the highway and have minimal interaction with vehicular traffic allowing it to serve residents of all ages and abilities.

Conceptually, the trail will span from Montara south to Miramar where it will connect with the Naomi Partridge Trail in Half Moon Bay. This will provide for a continuous, regional bicycle and pedestrian commuter trail. By providing residents the opportunity to walk and bicycle throughout the coastside, congestion on the highway should improve. The Trail would be easily accessed by residents living on the east side of Highway 1, require no highway crossings and act as a Safe Route to School for children at El Granada Elementary School.

In some locations, sidewalks require maintenance, while in others sidewalk or trail facilities are absent altogether. In these places without pedestrian facilities, pedestrians walk along paved or unpaved shoulders, or in the roadway. Given the higher traffic speeds, coastal access and community arterial function of Highway 1, this lack of pedestrian accommodation presents a safety concern in the area. It also conflicts with the County’s policy on Complete Streets and fails to comply with ADA guidelines for paths of travel to key locations (including transit stops).

2.2.2.1 Pedestrian Demand and Traffic Volumes

Pedestrian demand in the Midcoast region is spread along Highway 1, mainly near the coast. Destinations include beaches, trails, viewpoints and surfing destinations. Major pedestrian hotspots are portrayed in Figure 2.

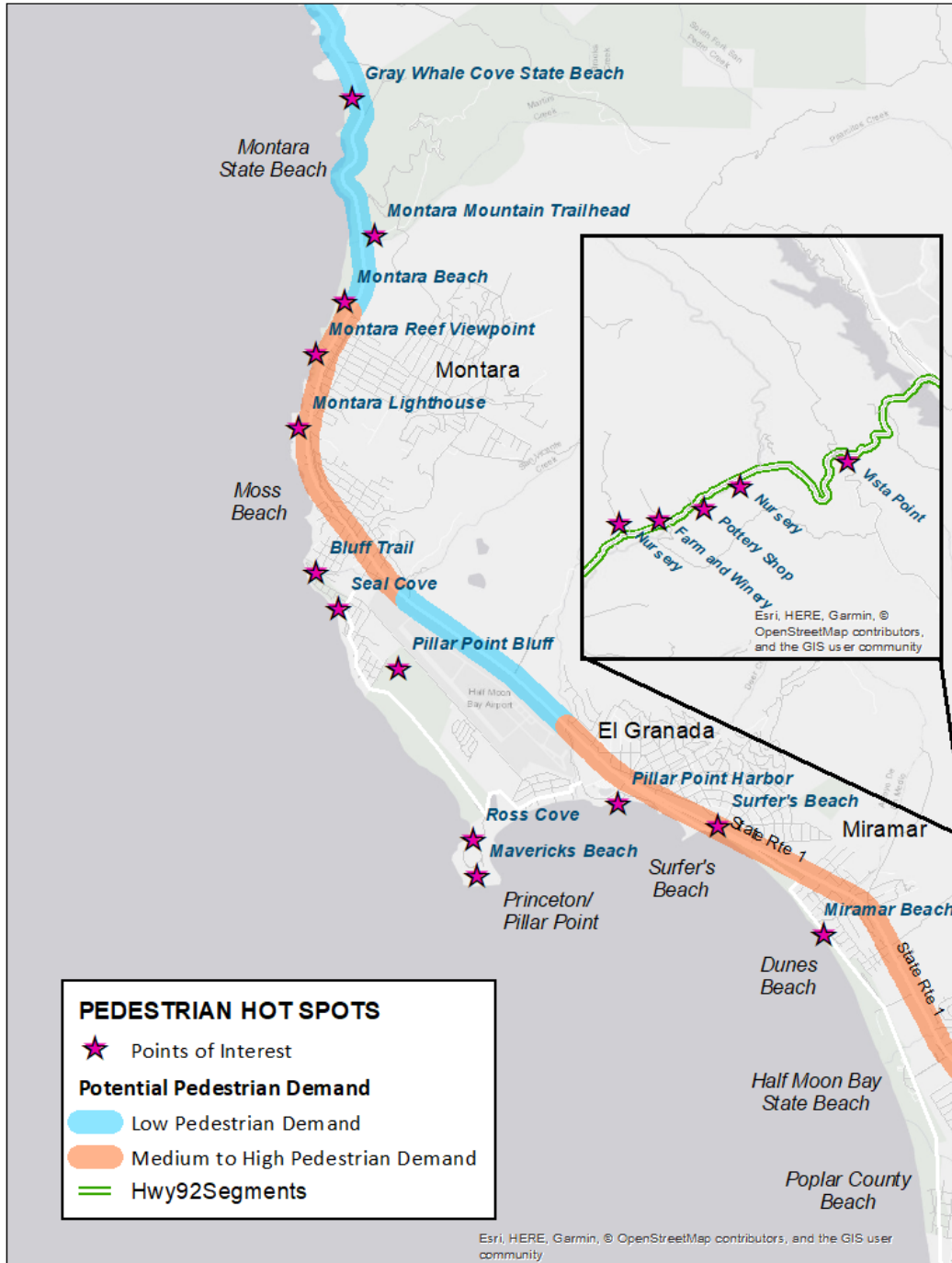


Figure 2. Pedestrian Hot Spots and Level of Demand

2.2.2.2 Pedestrian Crossings

All marked pedestrian crossings of Highway 1 and SR-92 within the Study Area are located at signalized intersections of Highway 1 and local streets in El Granada, and Princeton/Pillar Point Harbor. There are no marked crossings of Highway 1 in the communities of Moss Beach, Miramar and Montara. Existing crossings are primarily located at areas of dense residential and commercial land use and are notably missing from most recreational access points such as trailhead parking lots and designated vista points.

Visitors to coastal beaches who park east of Highway 1, including on the eastern shoulder are required to cross Highway 1 without marked crossings or sidewalks located on the eastern edge, often with little or signage to alert drivers of potential pedestrian crossings. Despite these unsafe conditions and high weekend traffic volumes, coastal visitors often cross Highway 1 to access beaches and other points of interest. Potential sites for new crossings include on Highway 1 between Coronado Street and Pillar Point Harbor, trailhead parking north of Martini Creek, and beach parking at Gray Whale Cove State Beach. Improved pedestrian crossings and accessibility are a priority, given that these highways serve as major arterials in coastal communities, and separate neighborhoods, commercial areas and parking areas from the coast or from one another.

There is a general lack of marked pedestrian crossings in the Study Area. A number of vehicle/pedestrian collisions have occurred at unmarked crossings, including at the potential improvement locations named above. Existing crossings use two parallel transverse lines, a design that has since been shown to have lower visibility for drivers than alternatives, such as continental crosswalks (also known as zebra striping). Some studies have claimed that low-visibility treatments can be even more dangerous than no crossing treatment because they provide pedestrians with a false sense of security and expectation that motorists will yield. More frequent crossings (of no more than 600 feet in developed or recreational areas) were called for on Highway 1 in the 2011 CBPP, but these have not yet been implemented. Robust pedestrian crossing treatments and beacons are also needed at key locations along SR-92, including at the two three-way intersections with Skyline Boulevard.

Given the high-speed conditions and the triple purpose of Highway 1 (as State Route, local arterial and coastal access facility), more effective crossing facilities are needed at all key crossing points along the route. The Congestion Management Project, which is currently being developed, has investigated potential long-term solutions to address this issue which include continental crossings in combination with Pedestrian Hybrid Beacons (also known as HAWK beacons), PELICAN signals, Rectangular Rapid Flash Beacons (RRFBs), or in-road warning lights. For example, high visibility continental crossings combined with RRFBs have been successfully used for coastal highway crossings in Astoria, Oregon. To prevent potential rear end collisions when motorists stop for pedestrians in the crosswalk, installation of two warning beacons separated by 150 feet are recommended. This gives drivers advance notice that someone is in the crossing and provides time to effectively decelerate from highway speeds.

The Southern Skyline Boulevard Ridge Trail Extension Project led by the San Francisco Public Utilities Commission will extend the Ridge Trail south from SR-92 alongside Skyline Boulevard to Henrik Ibsen Road. A safe crossing of SR-92 is needed to connect over 100 miles of continuous Ridge Trail from Marin County to southern San Mateo County. In the project vicinity, neither S.R. 35 nor S.R. 92 provides pedestrian facilities (sidewalks and crosswalks) or bikeways. Shoulders are present along some segments of both highways, but the shoulders are narrow or discontinuous in some locations from vegetation and roadway width constraints. In general, pedestrian activity in the area is minimal and limited to recreational users of nearby trails and paths who were assumed to travel to the area by automobile. The Upper SR-92/SR-35 intersection has poor visibility due to steep grades on approach to both sides of the summit.

Connect the Coastside

A safe pedestrian crossing will require careful planning and design which is beyond the scope of this plan. Therefore, Connect the Coastside proposes a collaborative planning and design effort led by Caltrans in partnership with San Mateo County, the San Francisco Public Utilities Commission, Bay Area Ridge Trail and other stakeholders. The planning process should consider a range of alternatives, including crosswalks with lighting

2.2.2.3 Trails and Coastal Access

The California Coastal Trail (CCT) is a scenic, recreational public trail system envisioned to be continuous along the California coast. The CCT is comprised of several different facility types within the Study Area, ranging from sporadically paved multi-use paths in Half Moon Bay to unpaved dirt trails north of Princeton to connections along the roadway shoulder in Montara. The CCT is intended to serve pedestrians primarily, but also accommodates many other users, including cyclists, wheelchair users, and equestrians where feasible.

Existing portions of the CCT run in a north-south direction west of Highway 1 north of Montara, in Princeton, and El Granada and Miramar. The trail is currently paved and separated from the highway between the City of Half Moon Bay and Pillar Point Harbor, transitioning to an on-street route through Princeton, to a multi-purpose dirt path along the Pillar Point bluffs to Seal Cove in Moss Beach.

There are a number of beaches, scenic viewpoints, and other attractions along the coastline. When these destinations are accessed by foot, pedestrians often walk directly there via local streets, parking lots, or, at times, privately owned property (where owners allow users to access public beaches, for example).

2.3 Bicycle Operations

2.3.1 Performance Standards

The Midcoast communities and County policies promote the creation and use of design and operating standards that allow a safe and complete network for bicycles that improves cyclists' mobility and accessibility. The bicycle facility standard for the Midcoast focuses on removing gaps and requiring bicycle facilities based on the level of adjacent vehicle speed and congestion. The bicycle parking standard is aimed at providing a sufficient expectation of bicycle parking at key destinations.

2.3.1.1 Volume-based Amenity Standards

Based on level of bicycle demand and traffic volumes, the need for bicycle facilities varies. **Table 11** shows volume-based standards for bicycle facilities influenced by traffic volumes and three levels of bicycle user demands. Low traffic and low bicycle demand can be usually met by Class III bikeways, while a medium bicycle demand is better met with Class II bikeways. A high user demand coupled with high traffic volumes require separated bikeways and additional safety features such as bike detection, dashed bike lanes and left-turn bike lanes at intersections. As per the National Association of City Transportation Officials (NACTO), on streets with posted speed limit greater than or equal to 25 mph bike lanes are most helpful, while on streets with higher speed limits of 35 mph or more, treatments that provide greater separation between bicycles and motor traffic should be considered. These include left-side bike lanes, buffered bike lanes and bike paths.⁶

⁶ <https://nacto.org/publication/urban-bikeway-design-guide/bike-lanes/conventional-bike-lanes/>

Table 11 - Volume-based Amenity Standards for Bicycle Facilities

| Traffic Volumes (veh/hr) | Suggested Improvements | Bicycle Demand | | |
|--------------------------|--------------------------------------|----------------|--------|------|
| | | Low | Medium | High |
| 0-800 | Class III bikeway | X | X | X |
| 800-1600 | Class III bikeway | X | | |
| | Class II bikeway | | X | X |
| 1600-2000 | Class II bikeway | X | X | |
| | Class IV separated bikeway | | | X |
| | Intersection bike detection (Signal) | | | X |
| > 2000 | Class II bikeway | X | | |
| | Class IV separated bikeway | | X | X |
| | Intersection bike detection (Signal) | | X | X |
| | Dashed intersection bike lane | | | X |
| | Left-turn intersection bike lane | | | X |

2.3.1.2 Bicycle Parking

Bicycle storage at any beach access point, and any major trip generator or recreational facility located on Highway 1 or SR-92 should have no more than 85% occupancy during an average weekend midday peak. As per the *San Mateo County Zoning Regulations*⁷, “Bicycle parking spaces shall be provided at a rate of one locker, rack, or other device to secure and park bicycles for ten vehicle spaces required, but in no case less than one bicycle parking space per parcel.”

2.3.2 Existing Conditions and Deficiencies

Bicycle infrastructure has been classified into the following types of facilities, with design guidance provided by the Caltrans Highway Design Manual:

- Class I Multi-use, paved paths that are separated from vehicular traffic, and enable two-way travel for bicyclists and pedestrians
- Class II On-street striped and signed lanes for bicyclists
- Class III Shared right-of-way for bicyclists and motorists, with “sharrow” symbols on the pavement to indicate that the roadway is to be shared with bicyclists
- Class IV Cycle-track. With a physical separation between traffic and an on-street striped and signed lane for bicycles

⁷ Zoning Regulations, Section 6254.4 (11), Planning and Building Department, County of San Mateo; https://planning.smcgov.org/sites/planning.smcgov.org/files/SMC_Zoning_Regulations.pdf

In 2014, Caltrans endorsed the National Association of City Transportation Officials (NACTO)'s Urban Street Design Guide, as a supplement to the state's Highway Design Manual and Manual of Uniform Traffic Control Devices (MUTCD). Assembly Bill 1193 also allowed jurisdictions to choose other guidelines such as the NACTO's Urban Bikeway Design Guide for design of their bicycle facilities. In particular, AB1193 permitted construction of cycle tracks (also known as protected bicycle lanes) and required Caltrans to provide design guidelines on these new "Class IV" facilities.

The California Coastal Trail ranges from Class I facilities to unclassified dirt paths in various sections along the coastline. Class I, II, and III facilities are present in Half Moon Bay, along Highway 1, SR-92, and Kelly Avenue.

Cyclists use Highway 1 as an intercommunity route along the coast, since it is the only direct and continuous north-south connection. Highway 1 has wide paved shoulders (typically 8-foot wide) in some areas, but no defined bicycle accommodation through the most dangerous points along the route, and allowed parking often provides an impediment to safe bicycle use. Intersections along Highway 1 often feature large corner radii and wide cross-sections that are designed to optimize conditions for fast-moving motor vehicles but make the facility more dangerous, inaccessible and uncomfortable for bicycles and pedestrians.

Some avid recreational cyclists use SR-92 as one of a handful of potential coastal access routes from eastern San Mateo County. SR-92 has wide paved shoulders in some areas, but these are narrow or disappear along significant segments of the route, and the roadway has portions of steep grades.

There is a lack of bicycle parking at recreational and other destinations within the Study Area. The addition of bicycle parking at major waterfront destinations can provide cyclists with more secure places to store their bicycles and can help encourage cycling throughout the Study Area. Additionally, other amenities such as lockers and showers can make cycling a more viable option for workers commuting to workplaces within the Study Area.

Again, the lack of bicycle facilities providing safe bicycle accommodation along key routes and through intersections in the Study Area conflicts with the County's Complete Streets Policy and exacerbates the problems of automobile dependence and motor vehicle congestion within the area, especially during commute hours and peak summer tourist times.

2.4 Transit Operations

2.4.1 Performance Standards

Transit ridership on the Coastsides is quite low, therefore overcrowding standards are not relevant under current and near-term future conditions. SamTrans as part of its *Reimagine SamTrans* project is developing a new Service Policy Framework, which will include identifying goals for metrics such as load and utilization tailored to the unique characteristics of areas of lower density development, such as the Midcoast. In lieu of performance standards, Connect the Coastsides establishes a goal of increasing transit ridership in the Study Area both to alleviate traffic congestion, and to reduce greenhouse gas emissions.

Connect the Coastsides recommends that all bus stops have a paved waiting platform, and bus stops with a minimum average daily ridership of 25 should have a bench while bus stops with a minimum average daily ridership of 100 passengers should have a full shelter.

2.4.1.1 Transit Capacity Utilization

SamTrans 2018 Coastside Transit Study reports that 96-99% of Midcoast households own cars, indicating there is a very low “transit-dependent” population there and thus a lower propensity to ride transit. The study also found that “ridership declined on all Coastside routes in 2017 compared to 2016. Routes 110 and 118 are the most productive in terms of ridership per service hour at 27 and 24 riders per service hour respectively. The 294 is the least productive at 7.1 riders per service hour, followed closely by the FLX Pacifica route at 8 passengers per service hour. The 17 route had the largest drop in ridership with a 28 percent drop in the ridership per service hour.” Fares systemwide on SamTrans buses represent about 16% of the total cost of providing the service, requiring deep taxpayer subsidies. Midcoast and HMB stakeholders indicated that frequent and direct access to BART stations was the highest transit service priority while some requested mid-day and weekend service to reach other activities.

2.4.2 Existing Conditions

Existing transit service to and within the Study Area is provided by the San Mateo County Transit District, which operates SamTrans, the regional bus service; and RediCoast, a paratransit service.

SamTrans 2018 study of its Coastside transit service found that ridership declined on all Coastside routes in 2017 compared to 2016. “Routes 110 and 118 are the most productive in terms of ridership per service hour at 27 and 24 riders per service hour respectively. The 294 is the least productive at 7.1 riders per service hour, followed closely by the FLX Pacifica route at 8 passengers per service hour. The 17 route had the largest drop in ridership with a 28 percent drop in the ridership per service hour. Ridership was also evaluated starting in 2012, and most routes are currently less productive than they were in 2012. Some routes saw an increase in riders per service hour in 2016 but have again experienced a decline since then.”

2.4.2.1 Fixed Route Transit Service

Just two fixed-route transit services operate in or near the Study Area as shown in **Table 12**. These services provide north-south and east-west transit access within the Study Area at headways that range from 30 minutes in the peak to 120 minutes in the off-peak. While providing mobility options for some travelers, transit does not function as a primary mode of transportation for most discretionary transit riders because of its limited coverage and long headways. Key features of existing fixed-route transit services are summarized in **Table 12**.

Table 12 - Fixed Route Transit Services

| Route | Agency | Description | Peak Headway (min) | Off Peak Headway (min) | Span of Service |
|--------------------|------------------|---------------------------------------|--------------------|---------------------------------|--|
| 17 | SamTrans | Pacifica – Pescadero (weekday) | 30 | 60 (weekdays) 120 (weekends) | 5:30 AM – 9:30 PM |
| 18 | SamTrans | Montara – Half Moon Bay | 30 | N/A | 7:00 AM – 9:20 AM 3:15 PM – 4:20 PM |
| 294 | SamTrans | Pacifica – Miramontes Point (weekend) | 60 | 120 | 5:30 AM – 9:00 PM |
| Devil's Slide Ride | City of Pacifica | Half Moon Bay – Hillsdale Caltrain | 75 | 100 | 8:00 AM – 5:00 PM |

SamTrans Route 17

Route 17 is a coastal community service bus that runs weekday service connecting Pacifica (just north of the Study Area) to Montara, Moss Beach, El Granada, Half Moon Bay, and Pescadero. Weekend service terminates at Miramontes Point, before reaching Pescadero. Route 17 operates along Cypress Avenue, Airport Street, and Capistrano Road in the Study Area, operating all days of the week between 5:30 AM and 9:30 PM. This line has 30-minute headways during weekday mornings, which increase up to 2 hours for the rest of the day as well as on weekends. Ridership studies conducted for the SamTrans Coastside Study reveal that between 2016 and 2017 ridership per service hour on Route 17 fell by 28 percent.

No shelters or benches are provided in the study area.

SamTrans Route 18

Route 18 is a school day only bus that runs service connecting Montara, Moss Beach, El Granada, and Half Moon Bay. Route 18 operates along 6th Street/Harte Street, Sunshine Valley Road, Etheldore Street, Cypress Avenue, Airport Street, Capistrano Road, Ave Alhambra, and Highway 1 in the Study Area, operating school days between 7:00 AM and 9:20 AM and between 3:15 PM and 4:20 PM. This line runs four buses in the AM and two buses in the PM with approximately 30-minute headways.

No shelters or benches are provided in the study area.

SamTrans Route 294

Route 294 is a regional express bus that connects Half Moon Bay to the other half of San Mateo County located along the San Francisco Bay. It is a vital link to the Hillsdale Caltrain station in San Mateo and the rest of the Bay Area. Route 294 operates along California State Route 92 between Half Moon Bay and the City of San Mateo. This line operates all days of the week between 5:30 AM and 9:00 PM, with headways that range from 1 to 2 hours.

No shelters or benches are provided in the study area.

Private Shuttle Services

The City of Pacifica offers the Devil's Slide Ride, a free shuttle which runs every 75 minutes on weekends from 8:00 AM to 5:00 PM between the Linda Mar Shopping Center in Pacifica, Devil's Slide Trail north of Montara in the south, and Oceana Terrace Senior Housing in the north.

Dial-a-Ride

Limited, demand-responsive transit services are available to the public residing within the Study Area under certain conditions of eligibility.

SamTrans OnDemand

SamTrans OnDemand is a pilot microtransit service that launched on May 6, 2019, and serves a five square-mile area around the Linda Mar community in Pacifica. It replaced the FLX Pacifica shuttle. Trips are requested via smartphone application, and cloud-based technology enables real-time dispatching and routing of vehicles to pickup and drop-off locations. The SamTrans customer service center is also equipped to process trip requests over the phone. Service is available between 6:15 AM and 6:30 PM.

RediCoast

RediCoast is a paratransit service managed by the San Mateo County Transit District as the coastal complement to Redi-Wheels on the bay side of the county. The service is provided under the Americans with Disabilities Act of 1990 (ADA).

RediCoast provides curb-to-curb transportation for disabled citizens living between Devil's Slide in the north and the border of Santa Cruz County in the south, including Princeton, Moss Beach, El Granada, and several other coastal communities. Travel outside of these areas is possible through arrangement with respective paratransit providers (e.g. Redi-Wheels for eastern San Mateo County, Outreach for Santa Clara County, etc.). RediCoast operates weekdays between 6:30 AM and 8:00 PM, and weekends and holidays between 8:00 AM and 5:00 PM. As of 2013, the cost for a one-way trip is \$3.75.

Disabled citizens qualify for RediCoast services if any of the following conditions are met:

- The person is unable to meet the physical, visual, or communicatory requirements to safely and efficiently complete their trip using a fixed route bus; or
- The bus service is not accessible to the person; or
- The person cannot independently travel from their home to the bus stop.

Personal attendants are allowed to ride free with proper certification and notice, and other companions are allowed to ride on a space-available basis with fare payment and prior notice.

2.4.3 Existing Transit Problems and Deficiencies

Currently, none of the fixed route transit has enough ridership to meet SamTrans goals for fare box cost recovery, or minimum ridership goals. Additionally, none of the fixed route service bus stops provide any amenities and no bus stops have sufficient ridership to meet SamTrans thresholds to provide amenities.

Additional transit service (particularly for major visitor events), improved stop access, enhanced bus stop amenities, and targeted marketing could serve to increase transit ridership within the area. Every transit stop should also be viewed as an opportunity to provide an enhanced and effective pedestrian crossing, since transit users typically need to cross the street at either the beginning or the end of their trips.

To increase transit service utilization, the County in partnership with SamTrans and C/CAG needs to actively engage in the *Reimagine SamTrans* planning effort to identify the service changes, amenity provision, marketing and other measures necessary to increase transit ridership in the Midcoast. This partnership can produce a relevant goal that will measure transit capacity utilization of buses.

3 BUILDOUT CONDITIONS

3.1 Development of Buildout Forecasts

An important early step in development of Connect the Coastside was to evaluate the existing and future development potential of the Study Area, by creating a buildout forecast. This forecast was needed to develop travel demand forecasts using these development projections and utilize the resulting traffic forecasts to identify needed transportation improvements. These processes are summarized below and described in more detail in the “Development Forecast for the San Mateo County Comprehensive Transportation Management Plan Public Review Draft” (October 2015) and “Buildout Analysis and Traffic Projections Report” (September 2014).

3.1.1 Buildout Land Use Projections

First maximum buildout projections were developed using currently adopted plans and zoning for each area and an inventory of vacant or underutilized parcels in the Study Area, which includes the Midcoast, Half Moon Bay and certain surrounding rural lands. Using assessments of current, and future potential development, the 2006 San Mateo County LCP Buildout Estimate, Association of Bay Area Governments projections for job growth, Plan Princeton and City of Half Moon Bay development projections, and GIS analysis of vacant and underdeveloped sites, the consultant team developed a maximum development projection or maximum buildout forecast for the study area. This forecast represents the best professional estimate of the full build out of all land authorized for development, including rural lands within the Study Area.

The Maximum Buildout Forecast (MBF) projects a total of 12,352 housing units (9,691 single-family and 2,661 multi-family) within the study area. The MBF also projects a maximum of about 8.53 million square feet of total commercial development and 9,897 jobs in the Study Area. Travel forecasts were developed based on the MBF by modeling 2014 and 2040 Study Area traffic and computing local growth in traffic demand due to the MBF for the Study Area for both intersections and roadway links. This involved updating the 2040 cumulative model with the local MBF land use. This analysis identified many deficiencies for both intersections and roadway segments; i.e., they would not meet the LOS standards under projected MBF conditions. The MBF land use was also used for the fee study portion of this project in estimating the productivity of a development fee for transportation impacts, which is discussed in Chapter 5.

These MBF projections were then refined to reflect constraints represented by the San Mateo County Midcoast Local Coastal Plan, the City of Half Moon Bay’s Measure D, existing zoning, development trends, regional growth projections and the market analysis conducted in 2014 for the Half Moon Bay General Plan update. The Constrained Development Forecast (CDF) represents the most appropriate and accurate projection of future development because it is based on concrete, enforceable policies constraining future residential development in the San Mateo County Midcoast LCP and the City of Half Moon Bay’s Measure D.

This CDF was subsequently incorporated into the land use inputs for the C/CAG travel demand model 2040 scenario, updating forecasts of housing units and employment in the study area. The model was then used to produce forecasts of peak hour traffic volumes for further analysis. Table 13 summarizes the existing and estimated residential units at buildout. **Table 14** presents the forecast for new non-residential construction and number of jobs expected at buildout.

Table 13 - Constrained Residential Development Forecast for Connect the Coastside (2040)

| Subarea | Existing (2014) | | | Pipeline ⁴ | | | Forecast (2040) | | | Total (2040) | | | Percent Change |
|-------------------------|--------------------------|----------------------------|---------------------------|-----------------------|---------------|--------------|---------------------------|----------------------------|---------------------------|--------------------|---------------|--------------|----------------|
| | Total Units ¹ | Single-Family ² | Multi-family ³ | Total Units | Single-Family | Multi-family | New Units ^{5, 6} | Single-Family ⁷ | Multi-family ⁷ | Total Units (2040) | Single-Family | Multi-family | |
| Unincorporated Midcoast | 4,300 | 4,005 | 295 | 22 | 19 | 3 | 1,094 | 716 | 378 | 5,416 | 4,740 | 676 | 26% |
| Half Moon Bay | 4,481 | 3,493 | 988 | 160 | 105 | 55 | 694 | 508 | 186 | 5,335 | 4,106 | 1,229 | 19% |
| Total | 8,781 | 7,498 | 1,283 | 182 | 124 | 58 | 1,787 | 1,224 | 564 | 10,750 | 8,846 | 1,905 | 22% |

Notes:

1 Existing development in Half Moon Bay has been corrected since November 2014 Existing Conditions Report. Existing mobile homes were not accounted for in that report; this results in increase of 409 single-family units.

2 Includes manufactured homes.

3 Includes housing in mixed-use buildings, and caretaker units.

4 Development projects under review as identified by San Mateo County in 2013-15 and the City of Half Moon Bay in 2014.

5 New residential development in unincorporated Midcoast limited to 40 units per year under Policy 1.23 of the San Mateo County Midcoast LCP (2014). In City of Half Moon Bay, Measure D limits residential growth to 1% per year, or 1.5% in downtown area. However, in Half Moon Bay, existing zoning is projected to result in less development than would be allowed under Measure D.

6 Future development in Half Moon Bay is based on 0.7% average annual growth rate projected in the Economic and Real Estate Conditions and Trends report (2014) prepared by EPS for the Half Moon Bay General Plan Update.

7 Assumes single-family/multifamily split found for zoning-based buildout analysis: 67% single-family and 33% multifamily in the Unincorporated Midcoast, and 75% single-family and 25% multifamily in Half Moon Bay.

Sources: San Mateo County Assessor, 2014; San Mateo County, 2014; City of Half Moon Bay, 2014; EPS, 2014; Dyett & Bhatia, 2015.

Table 14. Constrained Non-Residential Development Forecast for Connect the Coastside (2040)

| Subarea | Existing (2014) | Pipeline ³ | | Forecast (2040) | | Total (2040) | Percent Change |
|-------------------------|----------------------------|----------------------------------|----------------------------|--|----------------|---------------|----------------|
| | Existing Jobs ² | Pipeline Non-Residential Sq. Ft. | Pipeline Jobs ² | Potential Non-Residential Sq. Ft. ^{4,5} | Potential Jobs | Total Jobs | |
| Unincorporated Midcoast | 2,551 | 303,000 | 439 | 851,800 | 2,003 | 4,994 | 96% |
| Half Moon Bay | 5,334 | 37,500 | 94 | 294,000 | 276 | 5,704 | 7% |
| Total | 7,885 | 340,500 | 533 | 1,145,800 | 2,279 | 10,698 | 36% |

Notes:

1 Combines the total "known" square footage from County Assessor data with estimated square footage of other parcels with non-residential development. Estimated intensity of existing development is based on that of "known" development.

2 Estimated based on assumed ratio of jobs per square foot of non-residential development in different categories (e.g., visitor-serving commercial, retail commercial, industrial).

3 Development projects under review as identified by San Mateo County in 2013-15 and the City of Half Moon Bay in 2014.

4 Future development in Unincorporated Midcoast based on analysis of vacant and underutilized land; average FAR of existing development in each zoning district, and other factors. Development in Neighborhood Commercial and comparable zones is adjusted downward in proportion to reduction in residential units, to account for lower demand.

5 Future development in Half Moon Bay is based on average annual growth rates by employment sector as projected in the Economic and Real Estate Conditions and Trends report (2014) prepared by EPS for the Half Moon Bay General Plan Update. Square footage estimated based on assumed ratio of jobs per square foot of non-residential development in different categories (e.g., visitor-serving commercial, retail commercial, industrial), using same assumptions as in zoning-based buildout analysis.

Sources: San Mateo County Assessor, 2014; San Mateo County, 2014; City of Half Moon Bay, 2014; EPS, 2014; Dyett & Bhatia, 2015.

3.1.2 Travel Demand Forecast and Buildout LOS

Using projected development results from the updated buildout analysis, traffic forecasts for a 25-year buildout horizon were prepared using the San Mateo County C/CAG-VTA travel demand model. Model outputs include forecasted traffic volumes on major roadway facilities within the study area for peak hours and periods. These future travel volumes were used to forecast future year intersection and roadway segment LOS, as described in the Buildout Analysis and Traffic Projections Report.

Because the C/CAG-VTA model represents an average weekday condition, weekend forecasts were developed by calculating factors to convert the model's standard midday period traffic volumes to weekend volumes. These factors were derived from 7-day tube counts along Highway 1 and SR-92.

3.2 Buildout Transportation Problems and Deficiencies

There are two ways growth under Buildout Conditions affects transportation conditions within the study area: 1) Increase in number of vehicles wanting to access Highway 1 within the Study Area, causing increased delays at these intersections, and 2) Growth in regional pass-through traffic along Highway 1

leading to congestion. Development within the Study Area increases the number of vehicles wanting to turn on Highway 1 and SR-92 from arterials and collector streets within the Study Area. This growth is spread along multiple access points but can result in increased delay at intersections along Highway 1 and SR-92, most of which only have a single lane of access and are controlled by minor-street stop signs. While development within the Study Area also results in an increase in traffic volumes along Highway 1, some growth is also due to regional pass-through trips which do not originate or terminate within the Study Area.

A comparison of intersection LOS between Existing Conditions and Buildout (2040) Conditions is provided for Weekday AM and PM peak hour and Weekend peak recreational hour in **Table 15**. A comparison of roadway segment LOS is given in **Table 16**.

3.2.1 Intersection LOS

The *San Mateo County Traffic Impact Study Requirements* defines the intersection LOS standard for San Mateo County as LOS C with no individual movement operating at worse than LOS D. There is no definition of peak periods, however it is noted that a standard of LOS D during a peak period may be allowed for dense urban conditions per County's discretion. No differentiation is made between signalized and unsignalized intersections besides the LOS standard defined for individual movements.

Policy 2.43 in the LCP sets LOS D as the acceptable level of service for roadway segments during commuter peak periods, and LOS E as acceptable during recreational peak periods, when assessing the need for road expansion. The LCP also has an intersection standard of LOS D. Connect the Coastside uses LOS D to evaluate intersections and roadway segments during the weekday peak hour and LOS E for weekends.

The operation of study intersections is shown under Buildout Conditions as compared to Existing Conditions in **Table 15**. Under Buildout Conditions the signalized intersection of Highway 1 & Coronado Street will operate at LOS D during the AM peak hour and LOS E during the PM peak hour, which is below the standard. The other signalized intersections within the Midcoast region operate above the LOS C standard. The majority of unsignalized intersections along Highway 1 have minor street approaches that operate below the LOS D standard. The following intersections do not meet the LOS standard during the listed peak hours:

- Highway 1 and 2nd Street (AM, PM, Weekend)
- Highway 1 and 8th Street (AM, PM, Weekend)
- Highway 1 and Vallemar Street (PM, Weekend)
- Highway 1 and California Avenue (AM, PM, Weekend)
- Highway 1 and Virginia Avenue (AM, PM, Weekend)
- Highway 1 and Vermont Avenue (AM, PM, Weekend)
- Highway 1 and Cypress Avenue (AM, PM, Weekend)
- Highway 1 and St. Etheldore Street (South Terminus) (AM, PM)
- Highway 1 and Coral Reef Avenue (AM, PM, Weekend)
- Highway 1 and Magellan Avenue (AM, PM, Weekend)
- Highway 1 and Medio Avenue (AM, PM, Weekend)
- Highway 1 and Miramar Drive (AM, PM, Weekend)
- SR-92 and Ox Mountain Landfill Road (PM, Weekend)
- SR-92 and Skyline Boulevard (Upper) (AM, PM, Weekend)

-
- SR-92 and SR 35 at Crystal Springs (Lower) (PM, Weekend)

All of the unsignalized intersections that will operate below the standard are minor-street, stop-controlled and only have one lane of approach. Of these intersections, California Avenue and Cypress Avenue have more than 75 vehicles per hour on an approach turning onto Highway 1 and satisfy the peak-hour signal warrant. While adding additional approach lanes may facilitate the movement of right-turning vehicles onto Highway 1, the main source of the failing LOS for these intersections is the high through volume along Highway 1. This results in left-turning vehicles on the minor street needing to wait a long time for a sufficient gap between cars to safely enter Highway 1. This could be mitigated by controlling intersections with high minor street volumes and combining low volume minor street approaches using either a roundabout or a signal.

3.2.2 Roadway Segment Delay

The previous roadway performance standard evaluated roadway segment operations based upon LOS standards defined by the LCP for assessing the need for road expansion. However, this plan proposes the use of delay index to analyze roadway segments because of the constrained nature of roadways and the desire for not widening roads. Similar to Existing Conditions, delay index was calculated for study segments along entire lengths of Highway 1 and SR-92 within the study area.

Delay Index and Travel Times for study segments along Highway 1 and SR-92 under Buildout Conditions are shown in **Table 16**. Highway 1 southbound segment exceeds the delay index standard of 2.0 during the weekday PM peak hour, mainly focused on the southern portion of the route. SR-92 exceeds the delay index standard for all hours and for both segments in the buildout condition with most delay concentrated at the signalized lower intersection of SR-92 and SR-35

Table 15 - Buildout Conditions Peak Hour Intersection Level of Service

| Street Names | Control Type | LOS Standard ¹ | AM Peak Hour LOS | | PM Peak Hour LOS | | Weekend Peak Hour LOS | |
|-----------------------------|--------------|---------------------------|-----------------------|-----------------------|-----------------------|-----------------------|-----------------------|-----------------------|
| | | | Existing ² | Buildout ² | Existing ² | Buildout ² | Existing ² | Buildout ² |
| SR-1 / 2nd St | TWSC | C(D) | C | F | C | F | C | F |
| SR-1 / 7th St | TWSC | C(D) | B | C | B | C | B | C |
| SR-1 / 8th St | TWSC | C(D) | C | F | D | F | E | F |
| SR -1 / 16 th St | TWSC/Sig | C(D) | D | A | E | A | E | A |
| SR -1 / Carlos St | TWSC | C(D) | B | C | B | C | B | C |
| SR-1 / Vallemar St | TWSC | C(D) | C | D | C | F | C | E |
| SR-1 / California Ave | TWSC | C(D) | D | F | E | F | F | F |
| SR-1 / Virginia Ave | TWSC | C(D) | C | F | E | F | F | F |
| SR-1 / Vermont Ave (WB) | TWSC | C(D) | D | F | E | F | F | F |
| SR-1 / Cypress Ave (EB) | TWSC | C(D) | E | F | F | F | F | F |
| SR-1 / Etheldore St (South) | TWSC | C(D) | C | F | D | F | E | C |

| Street Names | Control Type | LOS Standard ¹ | AM Peak Hour LOS | | PM Peak Hour LOS | | Weekend Peak Hour LOS | |
|------------------------------|--------------|---------------------------|-----------------------|-----------------------|-----------------------|-----------------------|-----------------------|-----------------------|
| | | | Existing ² | Buildout ² | Existing ² | Buildout ² | Existing ² | Buildout ² |
| SR-1 / Capistrano Rd (North) | TWSC | C(D) | C | C | C | C | D | D |
| SR-1 / Coral Reef Ave | TWSC | C(D) | C | F | C | F | D | F |
| SR-1 / Capistrano Rd (South) | Signalized | C(D) | B | C | B | C | C | C |
| SR-1 / Coronado St | Signalized | C(D) | C | D | B | C | B | E |
| Obispo Rd / Coronado St | TWSC | C(D) | B | B | B | B | B | B |
| SR-1 / Magellan Ave | TWSC | C(D) | F | F | F | F | F | F |
| SR-1 / Medio Ave | TWSC | C(D) | F | F | F | F | F | F |
| SR-1 / Miramar Dr | TWSC | C(D) | C | E | F | F | E | F |
| SR-92 / Landfill Rd | TWSC | C(D) | F | E | F | F | D | F |
| SR-92 / Skyline Blvd (Upper) | TWSC | C(D) | E | F | F | F | F | F |

| Street Names | Control Type | LOS Standard ¹ | AM Peak Hour LOS | | PM Peak Hour LOS | | Weekend Peak Hour LOS | |
|----------------------------------|-------------------|---------------------------|-----------------------|-----------------------|-----------------------|-----------------------|-----------------------|-----------------------|
| | | | Existing ² | Buildout ² | Existing ² | Buildout ² | Existing ² | Buildout ² |
| SR-92 / SR-35 (Lower) | Signalized | C(D) | B | D | C | F | D | F |

¹ Standards provided within parenthesis are for individual movements.

² Signalized intersections and all-way stop controlled (AWSC) intersections are reported by the average delay and LOS for the intersection; two-way stop controlled (TWSC) intersections are reported with the worst approach's delay and LOS. Bolded intersections fall below the defined LOS standard.

Table 16 - Buildout Conditions Peak Hour Roadway Segment Delay Index

| Corridor and Direction of Travel | Operating Standard | Freeflow Travel Time (mm:ss) | AM Peak Hour | | PM Peak Hour | | Midday Peak Hour | | |
|---|--------------------|------------------------------|---------------------|-------------|---------------------|-------------|---------------------|-------------|---------------|
| | | | Travel Time (mm:ss) | Delay Index | Travel Time (mm:ss) | Delay Index | Travel Time (mm:ss) | Delay Index | |
| NB Highway 1 from Mirada Road to 1 st Street | 2.0 | 06:30 | 07:53 | 1.22 | 9:02 | 1.39 | 9:22 | 1.44 | |
| <i>Mirada Rd to Capistrano Rd (N)</i> | <i>N/A</i> | <i>02:37</i> | <i>03:37</i> | <i>1.39</i> | <i>4:41</i> | <i>1.8</i> | <i>5:02</i> | <i>1.93</i> | |
| <i>Capistrano Rd (N) to 16th St</i> | <i>N/A</i> | <i>02:59</i> | <i>03:07</i> | <i>1.04</i> | <i>3:15</i> | <i>1.09</i> | <i>3:12</i> | <i>1.07</i> | |
| <i>16th St to 1st St</i> | <i>N/A</i> | <i>00:54</i> | <i>01:09</i> | <i>1.28</i> | <i>1:06</i> | <i>1.21</i> | <i>1:08</i> | <i>1.25</i> | |
| SB Highway 1 from 1 st Street to Mirada Road | 2.0 | 06:30 | 09:51 | 1.52 | 14:59 | 2.32 | 12:35 | 1.94 | |
| <i>1st St to 16th St</i> | <i>N/A</i> | <i>01:00</i> | <i>01:08</i> | <i>1.14</i> | <i>1:17</i> | <i>1.28</i> | <i>1:29</i> | <i>1.48</i> | |
| <i>16th St to Capistrano Rd (N)</i> | <i>N/A</i> | <i>03:00</i> | <i>03:00</i> | <i>1.00</i> | <i>3:03</i> | <i>1.02</i> | <i>3:21</i> | <i>1.12</i> | |
| <i>Capistrano Rd (N) to Mirada Rd</i> | <i>N/A</i> | <i>02:30</i> | <i>05:43</i> | 2.30 | <i>10:39</i> | 4.28 | <i>7:45</i> | 3.12 | |
| SR-92 from Half Moon Bay city limits to I-280 on-ramp | NB | 2 | 08:42 | 43:44 | 18.82 | 15:39 | 36.28 | 39:27 | 59.71 |
| | SB | | | 17:51 | 22.74 | 08:22 | 118.20 | 36:07 | 135.19 |

¹ **Bolded** segments fall below the defined LOS standard.

4 TRANSPORTATION AND LAND-USE STRATEGIES

4.1 Determination of Improvement Strategies

The list of improvements provided below are based on the set of standards identified in this document and a revised Buildout Forecast. The Buildout Forecast used to model future traffic, including projected traffic from recent projects, such as the affordable housing project in Moss Beach. The infrastructure improvements described in this chapter were identified to mitigate deficiencies under Existing and Buildout traffic conditions.

4.1.1 Land Use Strategies

Connect the Coastside proposes the implementation of two strategies for managing land use to reduce the buildout potential so as to mitigate future traffic on Midcoast roadways. A lot merger program will be implemented, modeled on a policy adopted by the San Mateo County Board of Supervisors in 2006 but never implemented. The lot merger program will be initiated with incentives for voluntary mergers for an 18-month period, followed by a mandatory merger program.

Implementation of a mandatory lot merger program, generally following the policy adopted by the Board in 2006, would have limited to no effect on the number of buildable parcels along the Midcoast as the reduction in lots is already accounted for in the estimate of Buildout potential, because lot mergers were assumed to take place in the Midcoast LCP. A mandatory lot merger program could be challenging to carry out in the context of the uncertain legal status of many of the substandard lots in the Midcoast study area.

Based on the outcome of the lot merger program, the County may initiate a lot retirement program, where subdivisions of Midcoast lands zoned for Planned Agricultural District (PAD) and Resource Management (RM), both in the Coastal Zone, would be required to retire an equal number of lots as those to be developed to extinguish development rights on the retired lots.

4.1.2 Evaluation of Alternatives

A list of improvements was selected from a list of potential improvements compiled from TAC and community-suggested improvements, proposed projects identified in recent, relevant plans and reports, as well as improvements suggested by the consultant team to address identified deficiencies. Some improvements are proposed to address, in part, project-specific automobile traffic concerns although these improvements also contribute to achieving the mobility goals of this plan. To address the capacity needs generated by the Buildout projections, all improvements, except those necessary to address project specific impacts, were considered for implementation over the next 30 years to respond to planned growth in the study area. The complete list of potential improvements was analyzed based upon the ability to address deficiencies, feasibility concerns, cost estimates, and consistency with the LCP, including environmental considerations.

Improvements were then grouped into three alternatives for evaluation:

- High Impact/High Cost
- Medium Impact/Medium Cost
- Low Impact/Low Cost

For the most part, however, the improvements in each alternative were not designed to be mutually exclusive, unless specifically stated to be. Each alternative was based on cost, expected impact to existing infrastructure, environment and surrounding communities, as well as the overall effect on residents of the Midcoast Communities.

4.2 Infrastructure and Operational Improvements

This section describes the infrastructure and operational improvements for roadways, walkways, bikeways, parking and transit as identified under Connect the Coastside. The source of each project is listed in **Table 1** of Chapter 1.

4.2.1 Roadway Facilities

The following roadway improvements are included in Connect the Coastside to improve vehicular mobility by meeting relevant performance standards as well as addressing safety and circulation concerns. Proposed improvements to Highway 1 and SR-92 are shown in Figure 3 while local roadway improvements are shown in Figure 4 and Figure 5.

- These popular recreation destinations often attract large numbers of visitors and these improvements will alleviate congestion and improve safety for vehicles entering Highway 1 as identified in the *Highway 1 Safety and Mobility Improvement Study*:
 - **Turn Lanes and Acceleration Lanes** on Highway 1 and SR-92 will improve circulation and prevent turning vehicles from restricting flow along the highway in a cost-effective way.
 - Turns and acceleration lanes at Gray Whale Cove to make access to the beach safer and reduce slowdown along Highway 1
 - Left turn lanes at major businesses along SR-92 to improve business access and to promote safe, efficient highway traffic flow
 - **Standardized shoulder and edge treatment** (at Montara, Moss Beach and El Granada)

Currently, the roadway cross-section of Highway 1 throughout the study area is made up of inconsistent roadway and lane widths, narrow or non-existent shoulders, and variable painted medians. The Highway 1 Safety and Mobility Improvement Study identified three Context Zones throughout the study area representing areas with similar characteristics and user needs. A consistent cross-section for each context zone, with defined lane widths, shoulders and striped on-street bike lanes, would contribute to driver understanding of the environment. Easily identifiable roadway edges, intersections, and driveways will provide a clear and safe environment for auto, bicycle, and pedestrians. The Context Zones definitions and proposed improvements are defined as follows:

 - **Rural Zones** – Sparsely developed and primarily agricultural or recreational use with high vehicles speeds: No improvements proposed
 - **Fringe Zones** – Transitional segments approaching or leaving coastal communities with increased pedestrian and bicycle activity and side street access/egress with lower vehicle speeds: Valley gutter to define roadway edge and consistent lane widths less than 12 feet on segments where speeds are below 45 mph
 - **Village Zones** – Coastal communities with potential for multimodal conflicts due to parking, retail and restaurant use, transit stops, and controlled intersections with

lower vehicle speeds: Curb and gutter to define roadway edge, consistent lane widths less than 12 feet, and raised medians where currently striped.

- **Installation of a Traffic Signal or Roundabout**, pending the outcome of a Caltrans Intersection Control Evaluation process. In order to maintain the village character of Moss Beach by managing vehicular traffic and improving pedestrian and bicycle safety, Connect the Coastside recommends installation of intersection control at three intersections there. In addition, intersection control exists at the eastern (lower) SR 92/Hwy 35 intersection and will be needed based on buildout projections at the upper (western) SR92/Hwy 35 intersection. Based on community preference and safety, the Connect the Coastside preference is for roundabouts at these five intersections. Roundabouts eliminate left turns by requiring traffic to exit to the right of the circle and reduce vehicular speeds, thereby improving safety at intersections. These especially improve bicycle and pedestrian safety from lower vehicle speeds, shorter crossing distances and greater visibility⁸. Funding availability for right of way purchase and construction are key considerations in choosing the ultimate improvement. These improvements will be necessary to address current intersection performance deficiencies and support planned development in Moss Beach and Princeton:
- Highway 1 and California Avenue (Moss Beach)
 - Highway 1 and 16th Street (Moss Beach)⁹
 - Highway 1 and Cypress Avenue (Moss Beach)

The SR-92 and SR-35 (lower) intersection currently has a traffic signal, but Connect the Coastside recommends replacing it with a roundabout to further improved operations. In addition, the SR-92- and SR-35 (upper) intersection will perform below LOS standards, and will meet a signal warrant under projected build out conditions, necessitating future intersection control there. Constructing a roundabout at the SR-92 and SR-35 (upper) intersection presents unique challenges, given the grade of SR-92 approaching the intersection, and the topography of the land surrounding the intersection. While a roundabout is the preferred improvement, feasibility is challenging and may necessitate use of a signal.

- SR-92 and SR-35 (lower) (At Crystal Springs near Interstate 280)
 - SR-92 and SR-35 (upper)
- **Carlos Street Reconfiguration**
- Close the terminus of Carlos Street with a guard rail; Acquire right-of-way and extend Carlos Street north to 16th Street (western edge of realigned right of way will be approximately 150 feet from eastern edge of Highway 1). Access to Carlos Street at its northern terminus will be via 16th Street, and the Highway 1/16th Street Roundabout.

⁸ https://safety.fhwa.dot.gov/intersection/innovative/roundabouts/case_studies/rounds4peds.pdf

⁹ Intersection Control at 16th Street would be done in conjunction with closure of the Highway 1/Carlos Street intersection to facilitate, in part, development of the affordable housing site in Moss Beach at Cypress Point. A roundabout would likely require right of way acquisition.

-
- Install bus stop on Carlos Street with bus shelter. Buses will access Carlos Street at the north via the 16th Street/Highway 1 Roundabout, stop on Carlos at Cypress Point, and then proceed south on Carlos Street to Moss Beach. This will require coordination and agreement with SamTrans.
 - **Roadway Widening:** Providing passing lane/climbing lane allows cars to pass the high volume of trucks on this roadway segment as well as provides a passing lane to go around right-turning cars:
 - Passing Lane/Climbing Lane on SR-92 (between Half Moon Bay City limit and existing two-lane segment)
 - **Traffic Calming:** The implementation of traffic calming improvements such as speed display units and speed humps along the following streets is a cost-effective way to slow traffic and improve safety for bicycle and pedestrians.
 - Main Street (Montara): Project would construct curb extensions and crosswalks at the intersections of Main and 7th, Main and 8th, and Main and 9th. The project would also install a mini traffic calming circle at Main and 9th. Additionally, sidewalks (where they do not currently exist) and ADA curb ramps would be constructed on both sides of Main Street from 7th Street to 9th Street, and on the east side of the roadway from 9th Street to 10th Street.
 - Carlos Street (Moss Beach): The project would convert Carlos Street into a one-way (southbound) between Etheldore and California, reverse and restripe the existing diagonal parking along Carlos Street between Etheldore Street and California Avenue and . One or both utility poles on the north side of the roadway may need to be relocated; this cost is not assumed in this estimate.
 - **Signage** providing accurate information makes drivers and pedestrians more alert and improves the safety of intersections and roadway segments. The *Highway 1 Safety and Mobility Improvement Study* acknowledges the lack of signage along Highway 1 for bicycle and pedestrian safety. The following signage have been identified in Connect the Coastside:
 - Addition of stop signs where missing on stop-controlled side streets on Highway 1 (Montara, El Granada)
 - “Trucks Keep Right” signage on SR-92 (between Half Moon Bay City limit and I-280)

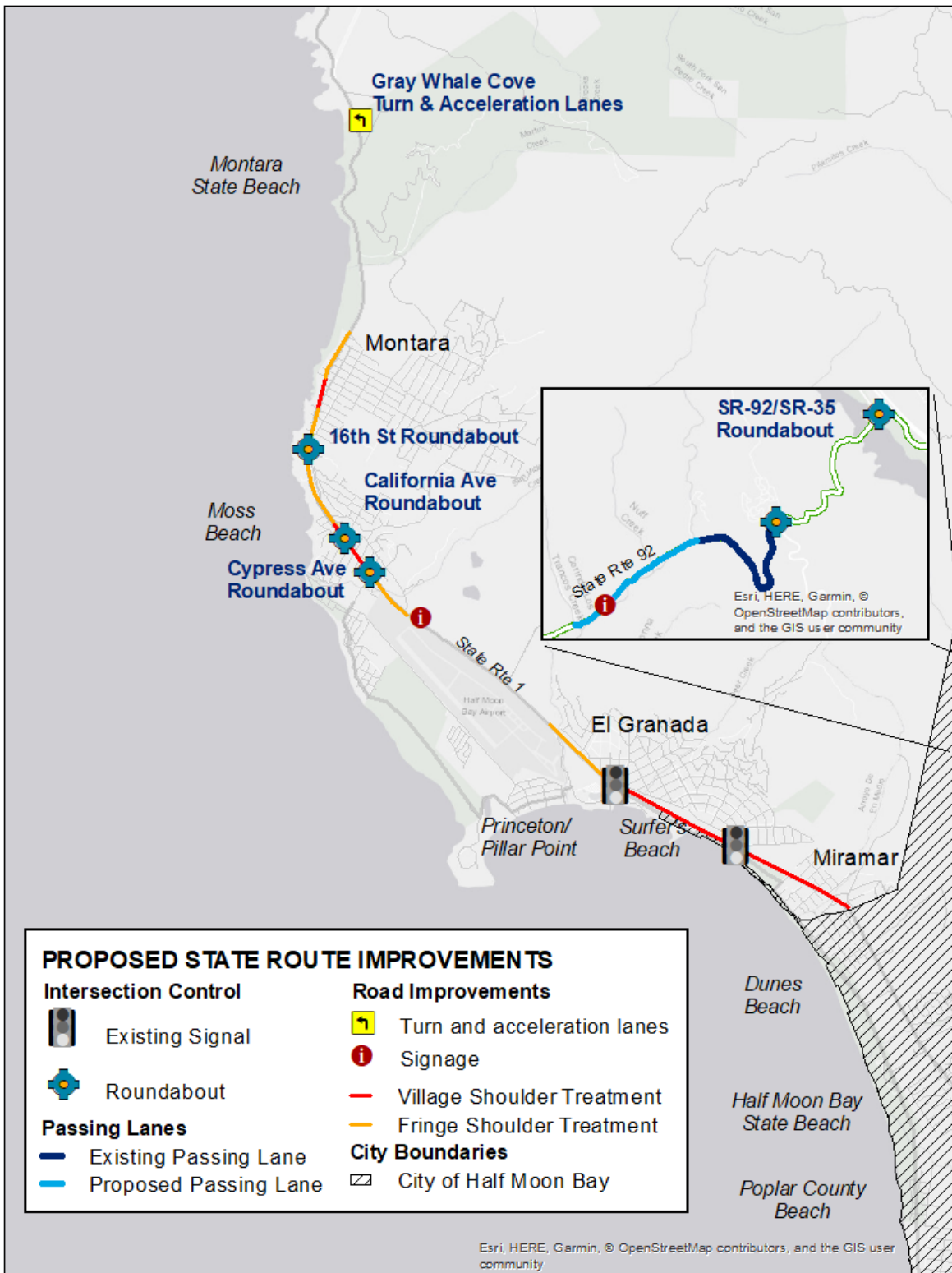


Figure 3. State Highway Roadway Facility Improvements



Figure 4. Local Roadway Facility Improvements (North)



Figure 5. Local Roadway Facility Improvements (North)

4.2.2 Bicycle and Pedestrian Facilities

Priorities for enhanced bicycle and pedestrian facilities throughout the Study Area are identified in the San Mateo Comprehensive Bicycle and Pedestrian Plan (CBPP), Highway 1 Safety and Mobility Improvement Study, 2000 San Mateo County Comprehensive Bicycle Route Plan, and specifically the Design Guidelines for Pedestrian Focus Areas. These improvements will provide facilities where few currently exist, and address mobility and accessibility barriers often encountered by low-income riders, agricultural workers, and transit users (for first- and last-mile trips) who are already biking and walking along Highway 1 and SR-92.

4.2.2.1 Planned Facilities along Highway 1 / California Coastal Trail / Multi-modal Trail

Proposed improvements to the Highway 1 corridor, the California Coastal Trail, and the planned Multi-modal Trail will improve its bicycle “level of service” as a countywide bicycle corridor, and enhance mobility for pedestrians in the area. The California Coastal Trail (CCT) is a network of public trails for walkers, bikers, equestrians, wheelchair riders and others along the 1200-mile California coastline. In the Midcoast, the planned CCT route is approximately 10 miles long from Point San Pedro to Half Moon Bay. The existing Midcoast trail alignment and configuration varies considerably. In some areas, it traverses roadways on the landside of Highway 1, such as in Montara where it is designated on Pedro Mountain Road, and in others it is an earthen blufftop hiking trail, such as in Pillar Point Bluff County Park. Some existing segments of the trail will be improved over time from what are currently Class II or Class III facilities to Class I trails, proximate to the Ocean. Connect the Coastside prioritizes completion of the CCT in the Midcoast.

The Midcoast Multi-Modal Trail (Parallel Trail), is a bicycle and pedestrian commuter trail that will provide an alternative means of transportation for residents of the Midcoast to safely access neighboring communities, town centers, schools and recreational destinations without having to travel on the highway. The Multi-modal Trail was conceptualized in the community-developed Highway 1 Safety and Mobility Improvement Study in Phase 1. The Trail will be separated from the highway and have minimal interaction with vehicular traffic allowing it to serve residents of all ages and abilities.

The Multi-Modal Trail (See Appendix A for proposed alignment) will span from Montara south to Miramar where it will connect with the Naomi Patridge Trail in Half Moon Bay. This will provide for a continuous, regional bicycle and pedestrian commuter trail. By providing residents the opportunity to walk and bicycle throughout the coastside, congestion on the highway should improve. The Multi-modal Trail would be easily accessed by residents living on the east side of Highway 1, require no highway crossings and act as a Safe Route to School for children at El Granada Elementary School. Connect the Coastside also proposes a Class I or Class II pedestrian and bicycle facility along Airport Street between Princeton and Moss Beach.

Together, these facilities will be part of the proposed North Coast Bikeway in the CBPP, connecting Daly City, Pacifica, and Half Moon Bay.

The Ohlone-Portola Heritage Trail in San Mateo County is a proposed recreation trail route system is a braided system of Class I, II and III bikeways, multi-use trails, riding and hiking trails and sidewalks. In select locations, the route is a share-the-road bicycle, pedestrian and equestrian route. Trail alignments for bicycles differ in places from those for pedestrians and equestrians because of management restrictions. The Heritage Trail begins at the Año Nuevo State Park Visitor Center, passes over Sweeney Ridge, and extends to Portolá’s journey’s end at State Historic Landmark in Menlo Park. Once completed the recreation route will be an approximately 97-mile-long regional trail that will link the bayside of San Mateo County with its coastside. For bicyclists, there is a near-term opportunity travel the entire route by

Connect the Coastside

using existing streets in selected locations where off-road recreation route gaps currently exist. In Midcoast, the segment gaps and streets that could be used as an alternative in the near-term includes the segment following the California Coastal Trail to Cypress Avenue, Etheldore Street, Carlos Street, Farallone Avenue, and Kanoff Street that connects with Rancho Corral De Tierra trail system and Pedro Mountain Road.

Key pedestrian elements for the Highway 1 corridor that are identified in the plans include building new pedestrian pathways and more frequent and consolidated crossings for residents and visitors alike. Sidewalks would be constructed in developed areas or along access routes to recreational areas. According to the 2011 Comprehensive Bicycle and Pedestrian Plan (CBPP), pathways that range in width from just 4-feet to 12-feet would be built in undeveloped areas that feature pedestrian activity. It should be noted that under the ADA standards, 5 feet is the minimum width required to allow two wheelchairs (or strollers) to pass. Where sidewalks are less than 5-feet wide, the ADA standards require passing spaces to be constructed at least every 200 feet.

In conjunction with the development of these sidewalk facilities, current plans propose existing sidewalks to be upgraded with vertical curb and gutters. Uncontrolled crossings would be enhanced with high-visibility striping and infrastructure, and median islands used as refuges to shorten crossings where possible. Developed areas would have a limit of 600 feet between crossings along corridors.

Other focus areas identified in the above plans include pedestrian-scale lighting in developed areas, landscaped buffers at a minimum width of 5 feet where possible and clearing pathways of debris for both cyclists and pedestrians.

4.2.2.2 Planned Facilities along SR-92

The CBPP identifies SR-92 as a key corridor for bicycle and pedestrian facilities. Proposed improvements include a Class I bicycle facility in the form of 7-foot shoulders along SR-92 between Highways 1 and 35 (Upper), and a multiuse pathway along SR-92 between Highway 35 (upper) and the Interstate 280 bicycle-pedestrian overcrossing.

Key Projects identified in the 2000 San Mateo County Comprehensive Bicycle Route Plan and 2011 Comprehensive Bicycle and Pedestrian Plan are listed in **Table 17**. These include planned regional improvements both within and outside the Connect the Coastside study area.

Table 17 - Planned Bicycle and Pedestrian Facilities

| Project | Jurisdictions | Description of Identified Priorities | Status |
|----------------------------|---------------------------------|--|---|
| Coastside Bikeway Projects | San Mateo County, Half Moon Bay | Improvements to SR-92 between Half Moon Bay and Highway 280, including: - Improvements to upper and lower SR-92/SR 35 intersections - 7-foot shoulders on SR-92 between Hwy 1 and SR 35 - Pathway along SR-92 between SR 35 to I-280 bike-ped overcrossing. | San Mateo County: portions of the coast side trail project. Half Moon Bay: Construction of multi-use path along Highway 1. |

| Project | Jurisdictions | Description of Identified Priorities | Status |
|---|------------------------------------|---|--|
| | | Extension of multiuse coastal trail along Highway 1 north and south from Half Moon Bay. | |
| North Coast Bikeway | Pacifica, Daly City, Half Moon Bay | Pathway, wide shoulders, directional signing connecting Daly City, Pacifica and Half Moon Bay, including multiuse path on Highway 1. | Daly City: Bike lanes on Southgate Avenue Pacifica: Bike lanes on Palmetto Avenue, bike path along most of Highway 1 to San Pedro Mountain Road |
| Highway 1/ Multi-modal Trail Improvements | | <p>Sidewalks in developed areas or access routes to recreation areas; 4' – 12' pathway in undeveloped areas with pedestrian activity</p> <p>Vertical curb and gutter where sidewalks exist</p> <p>Obstacles removed from pedestrian way</p> <p>ADA-compliant curb ramps</p> <p>Pedestrian-scale lighting in developed areas</p> <p>Minimum 5' landscape buffer where possible</p> <p>On-street parking buffer in developed areas</p> <p>Crossings at key desire lines</p> <p>High visibility, enhanced crossings at uncontrolled locations</p> <p>Median islands</p> <p>Max 600' between crossings in developed areas</p> <p>Regular transit service during peaks in developed areas</p> <p>Convenient transit stops in developed areas</p> <p>Connected bike network</p> | |
| Coastal Trail Improvements | | <p>Sidewalk improvements in Montara, Moss Beach (both coterminous with Multi-modal Trail) and Princeton</p> <p>Safe pedestrian crossings of Highway 1 at 1st Street in Montara, and 16th Street in Moss Beach</p> | |

| Project | Jurisdictions | Description of Identified Priorities | Status |
|-------------------------|---------------|--|--------|
| | | Beach or top-of-bank trail in Princeton from Pillar Point wetland east to Capistrano Road, with bridge over Denniston Creek Grading, paving (where appropriate) at various blufftop locations throughout study area | |
| Major Barrier Crossings | | 10' - 20' paths or minimum 5' detached sidewalks with wider pathways where high pedestrian and/or bicycle demand is expected Minimum 12' path if there is vertical enclosure Obstacles removed from pedestrian way ADA-compliant curb ramps Pedestrian-scale lighting, at crossings at a minimum Maximum of 1 mile between crossings Marked crossings at signalized and stop controlled locations on access routes to barrier crossing Connected bike network | |

4.2.2.3 Bicycle Parking

The CBPP identifies secure bicycle parking as a key element of the bicycle network especially at end-trip locations. The following types and relevant locations, within the study area, of bicycle parking have been identified to be of countywide significance and will be eligible for funding through the CFP process:

- Bicycle racks at regional destinations including:
 - Transit stations
 - No relevant locations
 - Transit hubs
 - No relevant locations
 - Community downtowns
 - Princeton, Moss Beach
 - Pillar Point Harbor Beach Area
 - Miramar Beach Area
 - Montara State Beach Area
 - Gray Whale Cove State Beach
 - Public hospitals
 - Seton Coastside Medical Center
 - Regional Parks
 - Rancho Corral De Tierra
 - McNee Ranch State Park
 - Fitzgerald Marine Reserve
 - Quarry Park

-
- Pillar Point Bluff
 - Bicycle lockers or similar long-term secure parking facility at the following regional destinations:
 - Transit stations
 - No relevant locations
 - Transit hubs
 - No relevant locations
 - Community downtowns
 - Not identified for lockers and long-term parking
 - Public Hospitals
 - Seton Coastside Medical Center
 - Regional Parks
 - Rancho Corral De Tierra
 - McNee Ranch State Park
 - Fitzgerald Marine Reserve
 - Quarry Park
 - Pillar Point Bluff

4.2.2.4 Proposed Pedestrian and Bicycle Facilities

Based on an evaluation of proposals from prior plans, community and stakeholder input, and application of the bicycle and pedestrian performance standards, the following pedestrian and bicycle improvements have been proposed to address safety and circulation concerns as well as to meet relevant performance standards.

- **Pedestrian Facility Improvements** such as walkways ensure connectivity and access by providing a more continuous walking path for pedestrians. Multi-use trail along Highway 1 and the coastal trail will provide pedestrians and bicyclists with a direct connection to communities and locales along Highway 1 and the Coast.
 - Coronado Street and Obispo Road/Ave Alhambra walkway (El Granada)
 - Multi-modal Trail Completion
 - Multi-modal (Parallel) Trail adjacent to Highway 1 for the entire study area. This trail is mainly for transportation purposes, such as Safe Routes to Schools.
 - Complete the Coastal Trail throughout the entire study area (paving of existing earthen portions to be determined by community planning efforts). In Montara, the Coastal Trail will be located primarily on streets (Second and Main), connecting to Moss Beach on the Multi-modal Trail between 11th and 16th, and then, crossing Highway 1 at 16th, along the coast side of Highway 1 along Vallemar Street, Weinke Way, California Avenue and Lake Street where it is aligned along the Bluff Trail until it joins Beach Way at Cypress Avenue in Princeton, and thence along Ocean Boulevard until it joins the Jean Lauer Bluff Trail in Pillar Point Bluff Park. In Princeton, the Coastal Trail extends west from the Bluff Trail along the West Beach Trail to Mavericks Beach, and east along West Point and Princeton Avenues, Broadway, Prospect Way, Capistrano Road, and thence along Pillar Point Harbor

Boulevard to its terminus at the boat launch ramp, where it connects to the shoreline Coastal Trail in Half Moon Bay. This trail is intended to be of recreational nature as transportation uses will be channeled through Multi-modal Trail to avoid overwhelming of the Coastal Trail.

- Striped pedestrian crossings of Highway 1 throughout the entire study area including Gray Whale Cove
- **Bicycle Facility Improvements** in addition to the Parallel and Coastal trails include bicycle lanes along SR-92 which greatly improve connectivity for bicyclists between San Mateo and Half Moon Bay. A bike lane along Capistrano Road would improve bicycle connectivity along Highway 1 and the Airport Street bike route provides a safe and cost-effective alternative route for recreational bicyclists away from the heavier traffic on Highway 1. Bike lanes along SR-92 can potentially have some form of separation measures such as flex posts or buffers for increased safety of bicyclists.
 - Highway 1 Class II Bike Lanes throughout the entire study area
 - SR-92 Class II Bike Lanes (between Half Moon Bay City limit and I-280)
 - Capistrano Road Class II Bike Lanes (Princeton)
 - Airport Street Class I/II/III Bike Trail/Lanes/Routes (Moss Beach) with the following options for each:
 - Class I trail on the Half Moon Bay Airport east side of Airport Street. Wetlands bridging is necessary for this option
 - Class II bike lane with sidewalk in developed areas on west side of Airport
 - Class II or III bike lanes or route on Airport Street
 - Bicycle and pedestrian improvements in “downtown” Moss Beach
 - Install sidewalks, curbs and driveway delineations

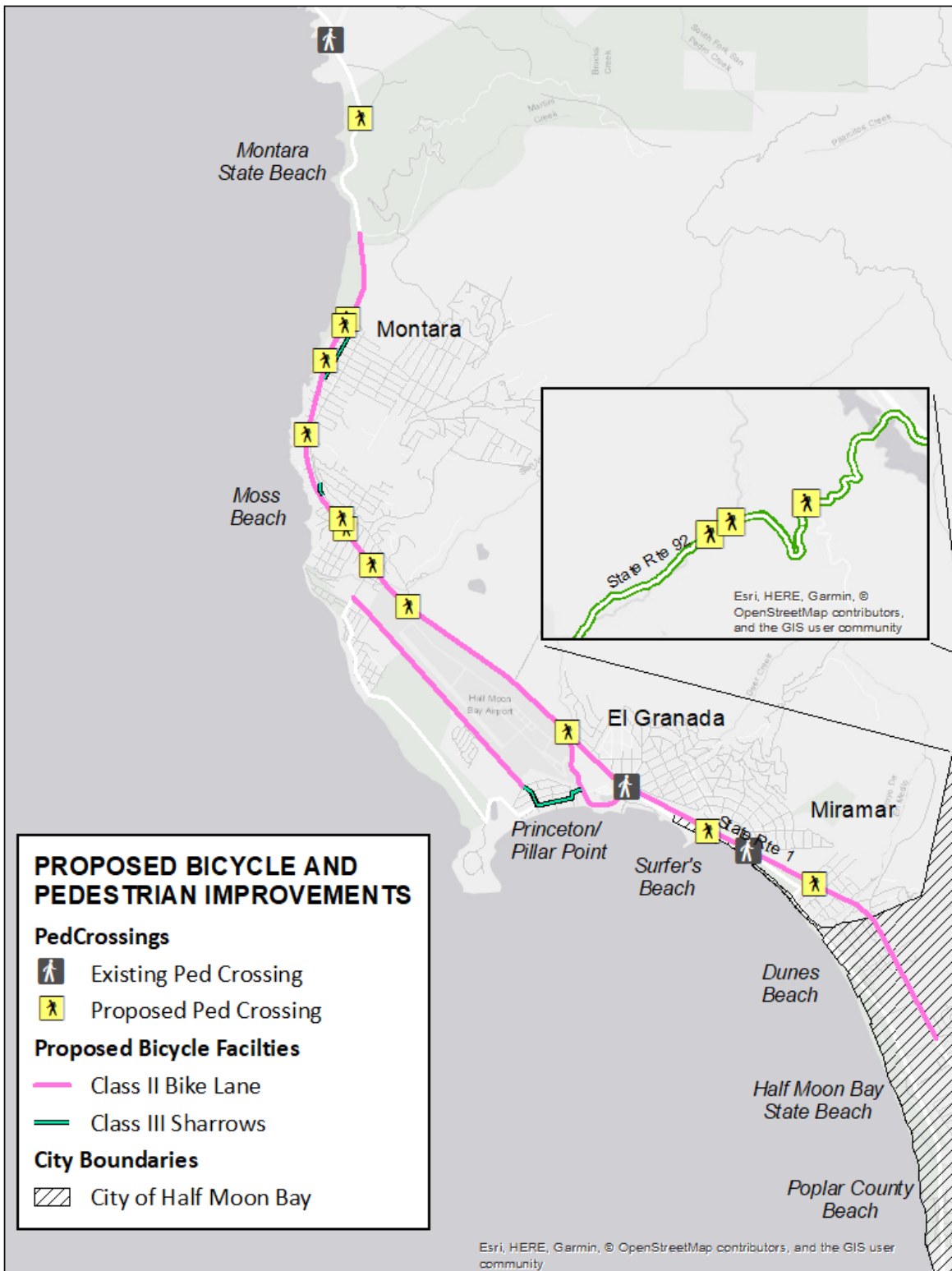


Figure 6. Proposed Bicycle and Pedestrian Facility Improvements

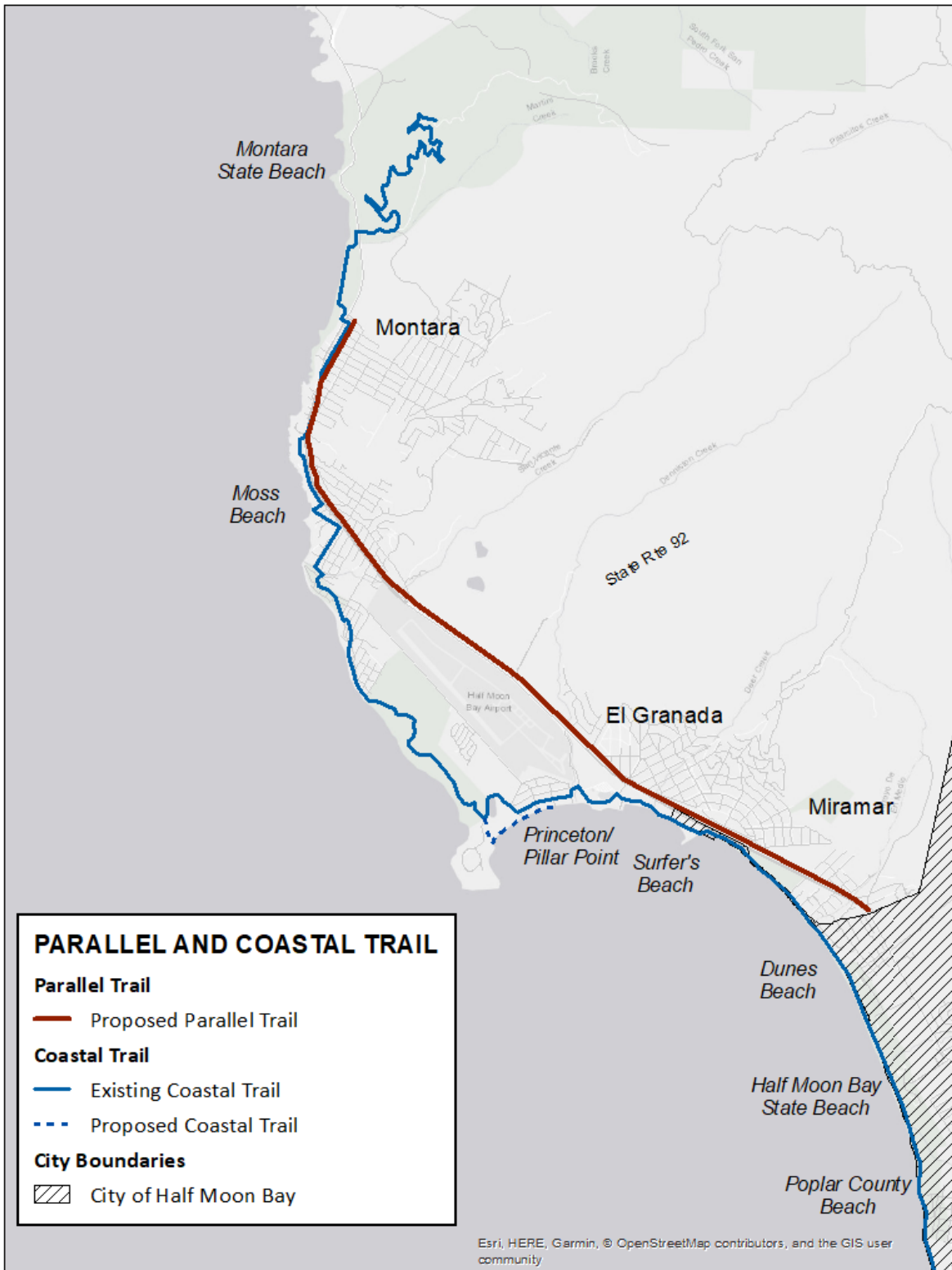


Figure 7. Existing and Proposed Coastal Trail

4.2.3 Transit Facilities and Operations

Increases in transit service will be based upon future discussions and negotiations with SamTrans. Connect the Coastside identified several potential improvements to transit service for the Midcoast, including increased operational frequency of weekend bus service for Line 17 and Line 294. These services will provide connections to regional transit providers during the morning and evening peak periods with limited stops. Another potential improvement involves rerouting Line 17 to directly serve the site in Moss Beach designated in the LCP for affordable housing development. This could involve routing the bus past the site via a new roundabout at 16th Street, or routing north from downtown Moss Beach, turning around at the site and returning to the regular route.

The SamTrans Coastside Study 2018 recognizes a near-term improvement to evaluate opportunities for better timed transfers to and from Coastside routes. This will help minimize transfer and wait times, improving connectivity where a transfer is needed. Longer term service improvements include a potential extension of Route 118 south to Half Moon Bay, and improve the Linda Mar Park and Ride in Pacifica. It also recommends the evaluation of additional park-and-ride facilities in the mid-coast or Half Moon Bay area, providing parking for passengers on the extended 118 route for weekday commutes as well as a parking place for weekend coastal visitors. The County will explore these and other transit service improvement options by collaborating with SamTrans on its Reimagine SamTrans Planning effort, an 18-month project to Improve the experience for existing SamTrans customers; grow new and more frequent ridership on SamTrans; and build SamTrans' efficiency and effectiveness as a mobility provider.

Bus stop improvements i.e. addition of bus stop benches, shelters and other facilities are proposed at various locations, including at Carlos Street and California Avenue in Moss Beach. Many Coastside residents state that pedestrian improvements, such as safe highway crossings, sidewalk, and pedestrian trails will increase transit ridership, because people can safely and conveniently access bus stops.

Near-term improvements:

- **Extension of the 118 route to serve Daly City BART.** Route 118 previously only served Colma BART. By extending service to Daly City BART, passengers have new access to a BART station with twice as much train service and a slightly less expensive fare for trips heading north into San Francisco. Other portions of the current route will not change. *This change took effect on January 21, 2018.*
- **Introduction of one additional trip in the evening on the 118.** The new trip will depart Daly City BART around 7:30 pm. This will offer more flexibility to passengers that require a later connection between BART and SamTrans. *This change took effect on January 21, 2018.*
- **Educate the community on how to use the FLX service in Pacifica.** We heard there was confusion around how to use the FLX service in Pacifica. This outreach will seek to spread information on how to call SamTrans to request a deviation on the FLX route, as well as how to use the FLX route as a normal service with stops and timetables. *This campaign was conducted in 2018.*
- **Evaluate opportunities for better timed transfers to and from Coastside routes.** This improvement will be an ongoing effort to evaluate scheduling with respect to the transfer experience, minimizing wait times and improving connectivity where a transfer is required. *This is an ongoing effort.*
- **Add a new northbound bus stop at the current Carlos Street intersection with Highway 1 in northern Moss Beach.** This facility could serve the proposed affordable housing project nearby. Both north and southbound stops south of the proposed 16th Street roundabout should be improved with concrete platforms and benches, and with sufficient ridership, shelters.

Longer-term improvements, requiring significant resource investment, which will be further studied individually by SamTrans staff:

- **Expand Route 118 to new places such as Half Moon Bay and increase frequency.** This route would offer a direct ride from points south of Pacifica to BART and increase the frequency of trips from Pacifica directly to BART. This service is envisioned to run on weekdays at 20 or 30-minute frequency in the peak commute periods and hourly in the midday periods.
- **Invest in physical improvements at Linda Mar park-and-ride and new park-and-rides if needed, such as secure bike parking, better waiting areas, restrooms, and other amenities.** This would require significant coordination with Caltrans, the owner of the Linda Mar park-and-ride, and the cities of Pacifica and Half Moon Bay.
- **Continue to assess the potential for non-traditional transportation** options to solve mobility challenges on the Coast, such as bike share, microtransit, and on-demand services. As a result of this recommendation, the SamTrans OnDemand pilot was launched in Pacifica in 2019 and is currently still operating as of January 2020 while staff evaluates the pilot's performance and whether the service could be viable in other locations.

4.2.4 Recreational and Transit Parking Facilities

The lack of parking capacity for weekday commuters and the large amount of weekend recreational parking demand discourages commuter use of transit, reducing service viability and results in a spillover of recreational demand into community parking areas. The *San Mateo County Coastside Access Study* conducted in 2015 demonstrates a need for additional parking in the Midcoast. The following parking improvements were identified in prior studies and included in Connect the Coastside:

- Development of a Park and Ride Shuttle to operate during weekends and events
- Development of pricing strategies to maintain an 85% lot occupancy
- Wayfinding signage
- Upper Gray Whale Cove Parking Lot Improvements
- Carlos Street On-Street Parking (between Vermont Avenue and Etheldore Street)
- El Granada Diagonal Parking (between Highway 1 and Avenue Alhambra, specific location to be determined)
- Moss Beach Park and Ride/Weekend Recreational Parking (South side of Etheldore at south end of Moss Beach)
- Collaboration with SamTrans and HMB on a new Park and Ride in HMB to support transit

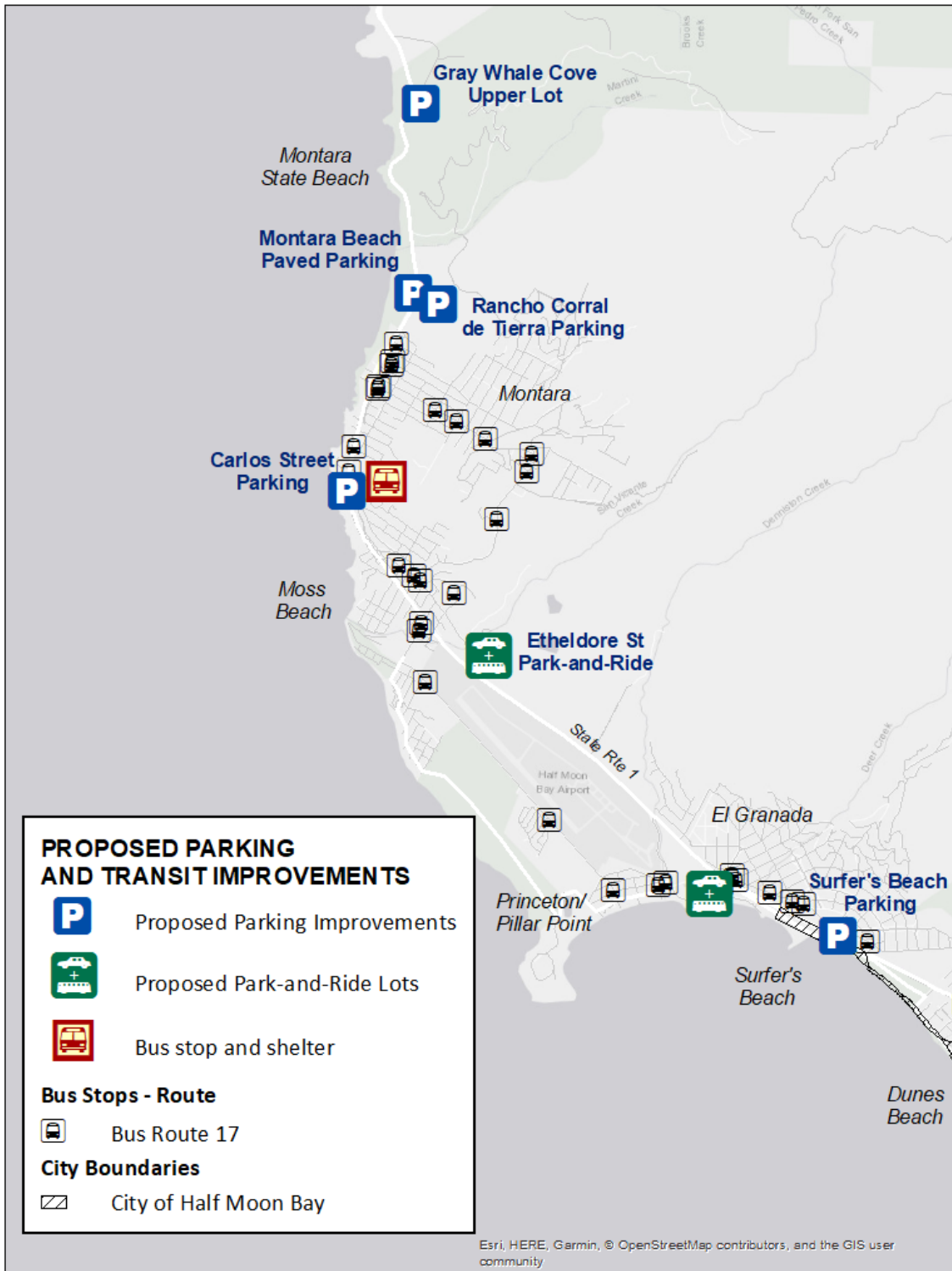


Figure 8. Proposed Parking and Transit Improvements

4.2.4.1 Lot Merger Program

The program would establish a process for contiguous substandard parcels under the same ownership to be merged, in the R-1, R-3, and RM-CZ zoning districts on the Midcoast. The lot merger program would apply as follows:

- At least two contiguous parcels in the same ownership;
- At least one parcel is undeveloped;
- The area of at least one lot is less than 4,500 square feet in the R-1 or R-3 districts, and less than 5,000 square feet in the RM-CZ district.

Lots meeting these criteria would be merged to create a parcel or parcels that meet the minimum parcel size requirements in R-1 or R-3 districts, or with a goal to reach at least 5 acres in area in the RM-CZ district.

4.2.4.2 Lot Retirement Program

Connect the Coastside recommends implementation of a lot retirement program that requires one-to-one retirement of development rights on existing lots in exchange for new lots, reducing Buildout potential and lessening the effect of new development on the transportation network.

The recommended lot retirement program would be designed to provide flexibility to project applicants by allowing them to either:

- Directly purchase existing lots from willing sellers, and extinguish development rights;
- Donate lots to a land trust or similar organization that would do the same; or
- Pay an in-lieu fee to the City or County to acquire and retire development rights from willing sellers at a 1:1 ratio. For the in-lieu fee to function properly, an appropriate price per development credit would need to be established, periodically reviewed and updated.

Acquisition of lots for lot retirement would be through donation or purchase. No property owner would be forced to sell their land for the purposes of this program. Lot retirement would be required only when new residential subdivisions are proposed. This would further support a priority for infill development and for visitor-serving and other commercial development.

4.2.4.3 Traffic Fee Mitigation Program

Connect the Coastside recommends a traffic Fee Mitigation Program that would collect fees for new residential and non-residential development on a per-housing-unit basis for residential and per-square-foot basis for non-residential development. The rates would be based on a specified list of projects needed to mitigate the impacts of the growth, the total estimated capital cost of those projects and the amount of new development expected. An assessment of the portion of total project need attributable to growth will determine what a legally defensible rate structure might be for a Transportation Impact Mitigation Fee Program.

4.3 Land Use Policies

Land Use policies such as lot merger and lot retirement programs could reduce transportation demand by reducing buildout potential. Attaching a mitigation fee to development can lead to reduced development as a result of additional costs. For the developments that do occur, these fees assist in providing a portion of funding for projects identified in this document.

LCP Policy 2.53 requires, among other things that Connect the Coastside “evaluate the feasibility of developing an in lieu fee traffic mitigation program...and development of a mandatory lot merger program.” Connect the Coastside evaluates these strategies and includes recommendations for implementation.

4.3.1 Lot Merger Program

The County will establish a process for merging contiguous substandard parcels under the same ownership. This would apply to lots with an area less than 4,500 square feet (R-1 or R-3 residential districts), or less than 5,000 square feet (RM-CZ districts). The program would initially be voluntary with incentives for 21 months and then become mandatory with a hearing and appeal process.

Implementation of a mandatory lot merger program would reduce the number of undeveloped parcels along the Midcoast. The effect of this reduction in lots is already accounted for in the estimate of Buildout potential, because lot mergers were assumed to take place in the Midcoast LCP.

4.3.2 Lot Retirement Program

A lot retirement program requiring one-to-one retirement of development rights on existing lots in exchange for new lots resulting from subdivision would have the effect of reducing Buildout potential and lessen the effect of new development on the transportation network. Mandatory lot retirement at a one-to-one ratio (1:1) as a condition of approval for some proposed residential subdivisions to mitigate impacts to the transportation system and public access to the coast could be an effective strategy, but will be reevaluated following implementation of the Lot Merger program to determine its efficacy, and whether a lot retirement program is feasible and would be appreciably effective at reducing traffic .

4.3.3 Development Review and Transportation Mitigation Fee Program

Development of a fee program collect fees for new residential and non-residential development on a per-housing-unit basis for residential and per-square-foot basis for non-residential development. The rates would be based on a specified list of projects needed to mitigate the impacts of the growth, the total estimated capital cost of those projects and the amount of new development expected.

5 COST AND FUNDING OF IMPROVEMENTS

To inform fee study analysis and project funding sources, cost estimates were carried out as presented in this chapter. Existing and potential funding sources and mechanisms have been described in Section 5.2.

5.1 Cost Estimate of Improvements

Table 18 gives an overview of cost and impact for each identified project. These metrics were identified from the cost-impact analysis carried out as part of Connect the Coastside report for Evaluation of Transportation Alternatives to Address Buildout Deficiencies.

Table 18 - Cost and Impact of Recommended Projects

| Mode | Project # | Recommended Project List | Jurisdiction | Deficiency from a standard | Safety/Circulation Issue | Cost | Network Impact |
|---------|------------------------------|---|--------------|----------------------------|--------------------------|--------|----------------|
| Roadway | R1 | Gray Whale Cove Turn and Acceleration Lanes | Midcoast | | X | Low | Low |
| | R2 | SR-1 Side-Street Stop Signs | El Granada | | X | Low | Low |
| | R3A | SR-1 Shoulder Treatment | El Granada | | X | Low | Low |
| | R3B | SR-1 Shoulder Treatment | El Granada | | X | Medium | Medium |
| | R4 | California Avenue Signal/Roundabout | Moss Beach | X | X | High | High |
| | R5 | Cypress Avenue Multi-Lane Signal/Roundabout | Moss Beach | X | | High | High |
| | R6 | 16th Street Roundabout | Moss Beach | X | | Medium | Medium |
| | R7 | Carlos Street Closure | Moss Beach | | X | Low | Low |
| | R8 | SR-92/SR-35 Roundabout | Montara | X | | High | High |
| | R9 | Carlos Street Traffic Calming | Moss Beach | | X | High | High |
| | R10 | Main Street Traffic Calming | Montara | | X | High | High |
| R11 | SR-92 Passing/Climbing Lanes | SMC | | X | High | High | |

| Mode | Project # | Recommended Project List | Jurisdiction | Deficiency from a standard | Safety/Circulation Issue | Cost | Network Impact |
|------------|-----------|--|--|----------------------------|--------------------------|--------|----------------|
| | R12 | SR-92 Truck Signs | Half Moon Bay | | X | Low | Low |
| | R13 | SR-92 Left Turn Lanes | SMC | | X | Medium | Medium |
| Pedestrian | Pe1 | Striped Pedestrian Crossing with Beacons | Varies | | X | Low | Low |
| | Pe2A | SR-1 Multi-modal Trail | El Granada and Moss Beach | X | | Low | Low |
| | Pe2B | SR-1 Multi-modal Trail | Moss Beach, El Granada, Half Moon Bay | X | | Medium | Medium |
| | Pe2C | SR-1 Multi-modal Trail | El Granada, Half Moon Bay, Montara, Moss Beach | X | | High | High |
| | Pe3 | Coronado Street and Obispo Road Sidewalk | El Granada | X | | Medium | Medium |
| | Pe4 | Coastal Trail | All | | X | Medium | Medium |
| | Bicycle | B1 | Capistrano Road Bicycle Facilities | Princeton | | X | Medium |
| B2 | | SR-92 Bike Lanes | SMC | | X | High | High |
| B3 | | SR-1 Bike Lanes | ALL | X | | Medium | Medium |
| B4 | | Airport Street Class I/II/III Bike Routes | Moss Beach | | X | Medium | Medium |
| B5 | | Bike/Ped improvements in downtown Moss Beach | Moss Beach | | X | Low | Low |
| Transit | T1 | Bus stop amenities | Various | X | | Low | Low |
| | T2 | Increased weekend SamTrans service | Various | X | | Low | Low |
| | T3 | Transit Stop Improvements | Moss Beach | X | | Low | Low |

| Mode | Project # | Recommended Project List | Jurisdiction | Deficiency from a standard | Safety/Circulation Issue | Cost | Network Impact |
|---------|-----------|--|--------------|----------------------------|--------------------------|------|----------------|
| Parking | Pa1 | Montara State Beach Parking Lot Improvements | SMC | X | | Low | Low |
| | Pa2 | Upper Gray Whale Cove Parking Lot Improvements | SMC | | X | Low | Low |
| | Pa3 | Wayfinding | ALL | | X | Low | Low |
| | Pa4 | Carlos Street On-Street Parking | Moss Beach | | X | High | High |
| | Pa5 | El Granada Diagonal Parking | El Granada | X | | High | High |
| | Pa6 | Park and Ride at Etheldore St (South Terminus) | Moss Beach | X | | Low | Low |

The following sections summarize the cost estimates of recommended improvements of roadways, pedestrian and bicycle, and parking facilities. The total cost for these three categories of improvements is approximately \$100 million. The cost of transit service improvements is indeterminate at this time and requires developing cost estimates in consultation with SamTrans.

5.1.1 Roadway Facilities

Table 19 - Cost Estimates of Roadway Projects

| No. | Project Name | Project Location | Total Project Cost |
|---|---|--|----------------------|
| R1 | Gray Whale Cove Turn and Acceleration Lanes | Gray Whale Cove Parking Lot | \$1,495,000 |
| R2 | SR-1 Side-Street Stop Signs | Montara and El Granada | \$30,000 |
| R3A | SR-1 Paved Shoulder and Curb (Phase A) | El Granada, Moss Beach, Montara | \$1,297,200 |
| R3B | SR-1 Paved Shoulder and Curb (Phase B) | El Granada, Moss Beach, Montara | \$1,622,400 |
| R4 | California Avenue Roundabout | SR-1 and California Avenue, Moss Beach | \$5,700,000 |
| R5 | Cypress Avenue Multi-Lane Roundabout | SR-1 and Cypress Avenue, Moss beach | \$7,219,815 |
| R5A | Cypress Avenue Single Lane Roundabout | SR-1 and Cypress Avenue, Moss beach | \$5,500,000 |
| R6 | 16th Street Roundabout | Montara | \$5,700,000 |
| R7 | Carlos Street Closure and Realignment | Moss Beach | \$2,588,000 |
| R8 | SR-92/SR-35 (Lower) Roundabout | SR-92 and SR-35 (Lower) | \$7,325,000 |
| R9 | Carlos Street Traffic Calming | Carlos Street from Etheldore Street to California Avenue in Moss Beach | \$344,000 |
| R10 | Main Street Traffic Calming | Main Street in Montara | \$588,000 |
| R11 | SR-92 Passing/Climbing Lanes | Between Half Moon Bay and San Mateo | \$12,802,000 |
| R12 | SR-92 Truck Signs | Between Pilarcitos Creek and SR-35 | \$12,300 |
| R13 | SR-92 Left Turn Lanes | At select business locations along SR-92 | \$471,000 |
| R14 | SR-92/SR-35 (Upper) Roundabout | SR-92 and SR-35 (Upper) | \$7,325,000 |
| Recommended Alternative Roadway and Intersection Total | | | \$ 60,019,715 |

¹The projects are still being developed and cost estimates do not include details such as extensive grading, land acquisition, retaining walls etc.

5.1.2 Pedestrian and Bicycle Facilities

Table 20 - Cost Estimates of Pedestrian and Bicycle Facilities Projects

| No. | Project Name | Project Location | Total Project Cost |
|--|--|-----------------------------------|----------------------|
| Pe1 | Striped Pedestrian Crossing with Beacons | Varies | \$5,000,000 |
| Pe2A | SR-1 Multi-Modal Trail | Miramar, El Granada | \$5,512,000 |
| Pe2B | SR-1 Multi-Modal Trail | El Granada, Moss Beach | \$9,000,000 |
| Pe2C | SR-1 Multi-Modal Trail | Moss Beach, Montara | \$7,000,000 |
| Pe3 | Coronado Street and Obispo Road Sidewalk | El Granada | \$9,000,000 |
| Pe4 | Coastal Trail | All | TBD |
| Pe5 | Bay Area Ridge Trail Crossing of SR-92 | SR-92 at Upper SR-35 Intersection | TBD |
| B1 | Capistrano Road Bicycle Facilities | Princeton | \$1,000,000 |
| B2 | SR-92 Bike Lanes | SMC | \$46,085,000 |
| B3 | SR-1 Bike Lanes | ALL | \$3,066,000 |
| B4 | Airport Street Class I Bike Route | Moss Beach | \$1,900,000 |
| B5 | Bike/Ped improvements in downtown Moss Beach | Moss Beach | \$ 250,000 |
| Recommended Alternative Pedestrian and Bicycle Facilities Total | | | \$ 87,813,000 |

5.1.3 Transit Facilities and Operations

The cost for transit improvements are indeterminate at this time. The cost of bus benches, rerouting of current service and establishing and operating new service will be determined in consultation and collaboration with SamTrans. These improvements will most likely involve cost-share between the County and SamTrans, grants or other funding sources.

5.1.4 Parking Facilities

Table 21 - Cost Estimates of Parking Facilities Projects

| No. | Project Name | Project Location | Total Project Cost |
|---|--|--|---------------------|
| Pa1 | Montara State Beach Parking Lot Improvements | Northernmost Montara State Beach Parking Lot | Completed |
| Pa2 | Upper Gray Whale Cove Parking Lot Improvements | Upper Gray Whale Cove Parking Lot | \$1,184,000 |
| Pa3 | Wayfinding Signage | Midcoast | \$341,000 |
| Pa4 | Carlos Street On-Street Parking | Carlos Street from Etheldore Street to California Avenue in Moss Beach | \$40,000 |
| Pa5 | El Granada Diagonal Parking | Current SR-1 alignment from Coronado Street to Capistrano Road | TBD |
| Pa6 | Park and Ride at Etheldore St (South Terminus) | Moss Beach | TBD |
| Recommended Alternative Parking Facilities Total | | | \$ 1,565,000 |

5.2 Funding Sources and Mechanisms

Funding for the proposed improvements can be potentially sourced from the grants and funds listed in the following sections, as well as from other sources not mentioned here.

5.2.1 Existing Funding Sources

The following section contains a listing of current and planned transportation improvement projects identified in regional and local level planning documents in San Mateo County. These projects are segregated into the following categories in **Table 22**:

- Highway Improvements/Roadway Maintenance
- Transit Capital/Operations
- Bicycle and Pedestrian Improvements
- Enhancement/Transit Oriented Development/Transportation for Livable Communities/Congestion

The estimated cost of each category is matched with the most applicable potential source of funds in **Table 23**.

Table 22 – Recommended Projects Categorization for Funding

| Project # | Project Name | Highway Improvements/ Roadway Maintenance | Transit Capital/ Operations | Enhancement/ TOD/ Livable Communities/ Congestion Management | Bike and Pedestrian Improvements |
|-----------|--|--|--------------------------------|--|----------------------------------|
| R1 | Gray Whale Cove Turn and Acceleration Lanes | X | | | |
| R2 | SR-1 Side-Street Stop Signs | X | | | |
| R3A | SR-1 Shoulder Treatment | X | | | |
| R3B | SR-1 Shoulder Treatment | X | | | |
| R3C | SR-1 Shoulder Treatment | X | | | |
| R4 | California Avenue/Hwy 1 Signal or Roundabout | X | | | |
| R5 | Cypress Avenue Multi-Lane Roundabout | X | | | |
| R6 | 16th Street Roundabout | X | | | |
| R7 | Carlos Street Closure | X | | | |
| R8 | SR-92/SR-35 Roundabout | X | | | |
| R9 | Carlos Street Traffic Calming | X | | | |
| R10 | Main Street Traffic Calming | X | | | |
| R11 | SR-92 Passing/Climbing Lanes | X | | | |
| R12 | SR-92 Truck Signs | X | | | |
| R13 | SR-92 Left Turn Lanes | X | | | |
| Pe1 | Striped Pedestrian Crossing with Beacons | | | | X |
| Pe2A | SR-1 Multi-modal Trail | | | | X |
| Pe2B | SR-1 Multi-modal Trail | | | | X |
| Pe2C | SR-1 Multi-modal Trail | | | | X |
| Pe3 | Coronado Street and Obispo Road Sidewalk | | | | X |

| Project # | Project Name | Highway Improvements/ Roadway Maintenance | Transit Capital/ Operations | Enhancement/ TOD/ Livable Communities/ Congestion Management | Bike and Pedestrian Improvements |
|-----------|--|--|--------------------------------|--|--|
| Pe4 | Coastal Trail | | | | X |
| B1 | Capistrano Road Bicycle Facilities | | | | X |
| B2 | SR-92 Bike Lanes | | | | X |
| B3 | SR-1 Bike Lanes | | | | X |
| B4 | Airport Street Class I/II/III Bike Routes | | | | X |
| B5 | Bike/Ped improvements in downtown Moss Beach | | | | X |
| T1 | Bus stop amenities | | X | X | |
| T2 | Increased weekend SamTrans service | | X | | |
| T3 | Transit Stop Improvements | | X | | |
| Pa1 | Montara State Beach Parking Lot Improvements | X | | | |
| Pa2 | Upper Gray Whale Cove Parking Lot Improvements | X | | | |
| Pa3 | Wayfinding | X | | | |
| Pa4 | Carlos Street On-Street Parking | X | | | |
| Pa5 | El Granada Diagonal Parking | X | | | |
| Pa6 | Etheldore Park and Ride | X | | | |

Table 23 - Potential Funding Sources for Project Categories

| | Highway Improvements / Roadway Maintenance | Transit Capital/ Operations | Enhancement/TOD/ Transportation for Livable Communities/Congestion Management | Bike and Pedestrian Improvements | Agency |
|---|--|-----------------------------|---|----------------------------------|----------|
| Federal Programs | | | | | |
| Better Utilizing Investments to Leverage Development (BUILD) Grant (Formerly TIGER) | X | | | X | US DOT |
| Congestion Management & Air Quality (CMAQ) | | | X | X | FHWA |
| Surface Transportation Block Grant (STBG) Program | X | | | X | FHWA |
| State Programs | | | | | |
| Active Transportation Program (ATP) Grant | | | | X | Caltrans |
| Sustainable Communities Grant | | | X | X | Caltrans |
| Strategic Partnerships Grant | X | X | X | | Caltrans |
| Adaptation Planning Grant | X | X | X | X | Caltrans |
| State Highway Operation and Protection Program (SHOPP) | X | | | X | Caltrans |
| Highways Safety Improvement Program (HSIP) Grant | X | | | X | Caltrans |
| Transit and Intercity Rail Capital Program (TIRCP) | | X | | X | CTC |
| State Transportation Improvement Program (STIP) | X | | | X | CTC |
| Trade Corridor Enhancement Program (TCEP) | X | | | X | CTC |
| State-Local Partnership Program (LPP) | X | | X | X | CTC |
| Office of Traffic Safety Grants | | | X | | OTS |

| | Highway Improvements / Roadway Maintenance | Transit Capital/ Operations | Enhancement/TOD/ Transportation for Livable Communities/Congestion Management | Bike and Pedestrian Improvements | Agency |
|---|--|-----------------------------|---|----------------------------------|---------------------------------------|
| Recreational Trails Program (RTP) | | | | X | CA Department of Parks and Recreation |
| Affordable Housing and Sustainable Communities (AHSC) Program | | | X | X | CA Strategic Growth Council |
| Transformative Climate Communities (TCC) Program | | X | X | X | CA Strategic Growth Council |
| Environmental Enhancement and Mitigation (EEM) Grant Program | | | | X | CA Natural Resources Agency |
| Urban Greening Grant Program | | | | X | CA Natural Resources Agency |
| Stormwater Management Program | | | | X | State Water Resources Control Board |
| Regional Programs | | | | | |
| OBAG | X | | X | X | MTC |
| TDA Article 3 | | | X | X | MTC |
| Regional Measure 1, 2, 3, and Future Regional Measures | X | X | X | X | MTC |
| Regional Active Transportation Program | | | X | X | MTC |
| Bicycle Rack Voucher Program | | | | | BAAQMD |

| | Highway Improvements / Roadway Maintenance | Transit Capital/ Operations | Enhancement/TOD/ Transportation for Livable Communities/Congestion Management | Bike and Pedestrian Improvements | Agency |
|---|---|-----------------------------|---|----------------------------------|---------------------------|
| Measure A | X | X | X | X | C/CAG of San Mateo County |
| AB 664 Net Toll Revenue Reserves | | X | X | | MTC |
| Transportation Funds for Clean Air (TFCA) | | X | X | X | BAAQMD |
| Measure M San Mateo County Vehicle License Fee | | | X | | C/CAG of San Mateo County |
| Congestion Relief Plan (C/CAG Member Agency Dues) | X | X | X | | C/CAG of San Mateo County |
| Peninsula Corridor Joint Powers Board (JPB) Member Fees | | X | X | | |
| Transportation Mitigation Fees | | | X | | |
| Clean Air Funds (CAFs) - Category: Alternative Transportation and or Public Education | | | | X | |
| Regional Transportation Fees (RTIF) | X | X | | | |
| Lifeline Transportation Program (LTP) | | | X | | |
| Vehicle Registration Fees | X | | | X | Local Jurisdictions |
| Local Programs | | | | | |
| Developer Fees/Transportation Impact Fees | Varies per jurisdiction and specific impact fee programs. | | | | Local Jurisdictions |

5.2.2 Potential Transportation Impact Mitigation Fee Program

The level of funding that might be available from a transportation impact mitigation fee program has been estimated based on the following inputs:

- The list of transportation projects and cost estimates to be included in the fee program
- Forecast of future land use – the potential for new residential units and new nonresidential uses within the study area
- Allocation of Costs to New Development – the percentage of project costs that can be associated with new development
- A traffic forecast for the year 2040 – this includes the volume of traffic over specific roadway segments during the peak period as well as vehicle trip origins and destinations

5.2.2.1 Fee Program Project List

The list of projects that would be included in the mitigation fee program is consistent with those described in **Table 19**, **Table 20** and **Table 21**. Of the total project costs, only a portion can be allocated to the fee program by demonstrating a nexus between the project need and new development.

5.2.2.2 Growth Potential

The quantity of new development expected in the study area was derived from the land use buildout analysis. Factors scaling the transportation impact of other land uses to the single-family dwelling unit have been developed using standard trip generation rates, average trip lengths, and pass-by trip percentages (see **Table 24**). These Dwelling Unit Equivalent (DUE) rates are applied in dividing improvement costs on an equivalent unit basis for transportation impact fee calculation.

5.2.2.3 Fee Program Cost Allocation

The four roadway intersection or segment improvements included in the Connect the Coastsides project list all address existing level of service (LOS) deficiencies. In this case, the fair share allocation of the improvement project costs is the portion of total traffic at each project location accounted for by new trips due to growth (excluding any new through trips).

Bicycle and pedestrian improvements are localized improvements serving trips that have their origin or destination within the study area rather than through trips. Lack of bicycle and pedestrian facilities is an existing deficiency in the study area; hence the improvements will benefit both existing and future residents. Since the improvements will serve the existing and future bicycle and pedestrian demand, the cost of those projects allocated to new development will equal the new development's proportional share of the total future development (existing plus new development) in the Connect the Coastsides study area (measured in Dwelling Unit Equivalents or DUEs).

Similarly, there are a number of projects which address safety concerns, design standard deficiencies, or benefit multiple modes of transportation. Examples of these include installation of stop signs, parking lot improvements, and roadway shoulder and curb improvements. Since these types of projects also benefit both existing and new development, the cost of those projects allocated to new development is also the new development's proportional share of the total future development (existing plus new development) in the Connect the Coastsides study area.

5.2.2.4 Traffic Forecast

Traffic forecasts were generated with the City/County Association of Governments (CCAG) travel demand model, using the same set of outputs employed for the Buildout Analysis and Traffic Projections Report.

The CCAG travel model was used to perform select link assignments of future (2040) PM peak period traffic passing through roadway project locations. These select link assignments are used to produce an origin destination matrix of the vehicle trips passing through model network links or nodes representative of the roadway project locations. The vehicle trip origins and destinations were then categorized as internal or external to the Connect the Coasts study area to separate through traffic from trips starting or ending in the study area (local traffic). The percentage of local traffic attributable to growth was estimated by multiplying the local trips by the percentage of growth DUEs within the study area (**Table 25**).

5.2.2.5 Estimated Fees

Table 26 summarizes the transportation impact fee calculation. A total of approximately \$15.7 million has been allocated to the fee program. The total allocated costs are distributed across an expected 2,620 DUEs, resulting in a fee of \$5,979 for each single-family dwelling unit, \$3,382 for each multifamily dwelling unit, and costs of \$8.04, \$7.00, and \$6.52 per square foot for retail, office and industrial development, respectively.

Table 24 - Dwelling Unit Equivalent (DUE) Rates

| Land Use Category | PM Peak Hour Trip Rate per Unit | Unit | Trip Length (miles) | Percent New Trips | VMT per Unit | DUE per Unit |
|-------------------|---------------------------------|---------------|---------------------|-------------------|--------------|--------------|
| Single-Family | 0.99 | Dwelling Unit | 5.0 | 100 | 4.95 | 1.00 |
| Multi-Family | 0.56 | | 5.0 | 100 | 2.80 | 0.57 |
| Retail | 3.81 | Square Feet | 2.3 | 76 | 6.66 | 1.35 |
| Office | 1.40 | | 4.5 | 92 | 5.80 | 1.17 |
| Industrial | 1.15 | | 5.1 | 92 | 5.40 | 1.09 |

Source: DKS Associates, 2017

Table 25 - Percentage of Local Growth Traffic at Selected Project Locations, 2040 PM Peak Period

| | Total Trips | | | Local Trips | | Local Growth % |
|-----------------------------------|-------------|---------|--------|-------------|--------|----------------|
| | Local | Through | Total | Existing | Growth | |
| Hwy 1 near California and Cypress | 4,767 | 17 | 4,785 | 3,799 | 969 | 0.20 |
| SR-92 east of Half Moon Bay | 11,734 | 536 | 12,271 | 9,350 | 2,385 | 0.19 |
| Intersection of SR-92 and SR-35 | 11,706 | 1,196 | 12,902 | 9,327 | 2,379 | 0.18 |

Table 26 - Nexus-Based Fee Rates for the Connect the Coasts Area

| Cost of Improvements Allocated to Coasts Area Growth | | | \$15,666,863 |
|--|------------------|--------------|---------------------------|
| Growth in Dwelling Unit Equivalents (DUEs) | | | 2,620 |
| Cost per DUE | | | \$5,978.83 |
| Land Use | Units | DUE per Unit | Fee per Unit ¹ |
| Single-Family | Dwelling Unit | 1.00 | \$5,979 |
| Multi-Family | | 0.57 | \$3,382 |
| Retail | 1000 Square Feet | 1.35 | \$8,044 |
| Office | | 1.17 | \$7,001 |
| Industrial | | 1.09 | \$6,517 |

1) Fee per Unit = (Cost per DUE) x (DUE per Unit)

6 PLAN IMPLEMENTATION

The purpose of the **Comprehensive Transportation Management Plan “Connect the Coastside”** is to identify and prioritize actions that the County, Caltrans, SamTrans, and others can take to address transportation challenges along the Highway 1 corridor throughout the urbanized Midcoast (Montara to Half Moon Bay), the Highway 92 corridor from Interstate 280 to Highway 1, and within the town centers, as well as provide a future vision for these corridors and communities. Since there is insufficient funding to implement all of the projects identified in this plan, this chapter synthesizes the results of the project evaluations and recommendations for implementation priorities, including a discussion of potential funding sources.

6.1 Actors, Partners and Stakeholders

Implementation of Connect the Coastside will require independent and collaborative action by a number of actors with responsibility for improving and maintaining transportation infrastructure and service within the Highway 1 corridor. Support from stakeholders is a key ingredient to successful implementation of Connect the Coastside.

6.1.1 Actors

Caltrans, the State’s transportation agency, and manager of Highway 1, has several roles to play in supporting the implementation of Connect the Coastside, including making investments that will:

- Improve coastal community safety & mobility with consistent roadway edges, shoulders, pedestrian crossings and roundabouts
- Monitor and plan for sea level rise
- Support completion of CA Coastal Trail
- Improve the pedestrian environment
- Implement new Traffic Operations Systems elements including Closed Circuit TV and Variable Message Signs
- Maintain & improve Park & Ride lots

Caltrans must approve all modifications within the Highway 1 right of way, and most likely will construct many of the improvements within the right of way envisioned in Connect the Coastside. Caltrans can also provide funding for improvements from state and federal funding sources.

San Mateo County maintains the County’s General Plan which establishes a goal to plan for a transportation system that provides for the safe, efficient, and convenient movement of people and goods throughout San Mateo County. The General Plan includes policies that guide County participation in regional and local transportation planning, articulating an active role within the County to achieve transportation improvements that support all modes of travel.

6.1.2 Partners

California Coastal Commission (CCC) implements the California Coastal Act and oversees development within the Coastal Zone. The County has a CCC-certified Local Coastal Program (LCP) that includes a policy requiring preparation of this plan. The LCP includes Public Works policies, including Roads and Transit sections to be used in evaluating transportation projects within the Coastal Zone, policies promoting

coastal access, and protection of coastal resources, including sensitive habitats, public access and coastal dependent development.

San Mateo County Parks and California State Parks Departments provide wonderful recreational opportunities at beaches, parks and nature preserves at lands they manage. Some of the improvements in Connect the Coastsides, including Coastal Trail and Multi-modal Trail segments, and recreational parking lots will be located in state or county parks. Park managers can obtain grant funds for certain Connect the Coastsides improvements, and can secure entitlements, conduct environmental review, construct, maintain, and manage these improvements.

City of Half Moon Bay (HMB) could coordinate with San Mateo County on key transportation investments and management strategies. The City of HMB is a key partner in alleviating the congestion on HWYs 1 and 92 that hampers coastal access. HMB can collaborate with the county, plan, design and fund improvements, including obtaining grant funding for HMB.

San Mateo County Transportation Authority (TA) administers the proceeds from Measure A to fund a broad spectrum of transportation-related projects and programs. Many of the Connect the Coastsides recommended improvements could be funded in whole or in part with Measure A funds.

City County Association of Governments, Congestion Management Agency (C/CAG-CMA): The Congestion Management and Environmental Quality Committee (CMEQ) provides advice and recommendations to the C/CAG Board of Directors on all matters relating to traffic congestion management, travel demand management, coordination of land use and transportation planning, mobile source air quality programs, energy resources and conservation, and other environmental issues facing the local jurisdictions in San Mateo County.

6.1.3 Stakeholders

Midcoast Community Council (MCC) is an elected Municipal Advisory Council to the San Mateo County Board of Supervisors. The MCC represents Montara, Moss Beach, El Granada, Princeton, and Miramar and provides the Midcoast Community with a more effective means to express its views to the County of San Mateo and other governmental agencies. The MCC assists the Midcoast Community in developing and expressing a long range vision of the Midcoast Community, which meets the goals of its residents for an improved quality of life, protection of the environment, and sound economic planning. The MCC was instrumental in the preparation of Connect the Coastsides and will play an important guiding role in its implementation.

6.2 Project Evaluation Criteria

6.2.1 Project Evaluations

In order to prioritize projects for feasibility and funding opportunities, six metrics were identified to evaluate and score each project. **Table 27** presents the metrics and range of performance scores ranging from Low (L) to High (H) or High to Low depending on their characteristics.

Table 27. Project Evaluation Metrics

| Project Evaluation Metrics | | |
|----------------------------|--|-----------------|
| Project Cost | Project design, capital and permitting cost | 1 to 3 (H to L) |
| Ease of Implementation | Funding, permitting, and environmental | 1 to 3 (H to L) |
| Multimodal Connectivity | Measures extent that a project fills a gap in existing bicycle, pedestrian or transit networks | 1 to 3 (L to H) |
| Safety and Circulation | Safety Bonus | 1 to 3 (L to H) |
| Shoreline Access | Bonus for enhanced shoreline public access | 1 to 3 (L to H) |
| Annual Cost | Operations and Maintenance | 1 to 3 (H to L) |
| Overall Score | Total obtained score | 1 to 3 (H to L) |

Project cost was considered to be high, medium or low based on alternatives feasibility study and cost estimation study as portrayed in **Table 18** of Section 5.1. Similarly, ease of implementation was obtained from Project Feasibility Considerations carried out as part of Alternatives Evaluation.

A project is said to provide multimodal connectivity based on whether or not it serves multiple modes of transportation. For example, Highway 1 Natural Pathway will provide access to pedestrians alongside the existing roadway, making it a high performing multimodal project. However, the Gray Whale Cove Turn and Acceleration Lanes will only serve motor vehicles and do nothing to improve bike, pedestrian or transit connectivity, thus leading to a low score in “Multimodal Connectivity”.

As part of the Evaluation of Recommended Alternatives, certain projects were identified to improve safety and circulation along the Midcoast area. Projects like adding bus stop amenities and some parking lot improvements do not impact safety or circulation and are therefore allocated low performance scores for this category.

Projects such as Highway 1 pedestrian improvements, increased SamTrans weekend service, and coastal parking lot improvements will increase shoreline connectivity. Most projects are historically known to have high operation and maintenance costs, leading to low performance scores throughout.

These implementation performance scores are depicted in **Table 28**. A score of 1 is given to projects with low performance, 2 for medium and 3 for high performance in a category. If the sum of all categories for a given project is more than 12, it has a high overall score. A sum of less than 8 leads to low overall score, while between 8 and 12 has a medium overall score. All projects have either high or medium overall performance scores.

6.2.2 Project Prioritization

This report section proposes a preliminary priority schedule for mobility improvements recommended in Connect the Coastside, based on an evaluation of their individual performance relative to the criteria in

Connect the Coastside

Table 28, and reflecting that certain improvements are needed in the near term to address traffic impacts from approved and proposed development. The project scores in **Table 28** are qualitative rather than quantitatively derived, but do reflect their overall ranking. Rankings within the short, medium and long term categories, and between project types are not determinative and projects will be pursued in the order that reflects available resources, environmental constraints, public priorities and other considerations. These preliminary priorities may shift as conditions change, including available funding, safety concerns, and other factors affecting community mobility needs and priorities.

Maintenance will continue to be an overarching priority for public investment in mobility. Maintaining existing facilities is critical for preserving existing mobility as well as for accommodating future development, including ongoing, regular maintenance of roadways, bike paths and trails, transit stop furnishings and buses, and sidewalks and other mobility infrastructure throughout the Midcoast should be funded and conducted. Whenever feasible and appropriate, Connect the Coastside projects should also be incorporated as maintenance projects are implemented. The following are the short, medium and long-term priorities for mobility improvements and land use policy implementation in the Midcoast. Project implementation priorities are summarized in **Table 29**.

Table 28. Project Implementation Performance Scores

| Project # | Project Name | Project Cost | Ease of Implementation (funding, permitting, and environmental) | Multimodal Connectivity | Improved Safety/Circulation | Shoreline Access | O&M Cost | Overall Score |
|-----------|--|--------------|---|-------------------------|-----------------------------|------------------|----------|---------------|
| R1 | Gray Whale Cove Turn and Acceleration Lanes | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| R2 | SR-1 Side-Street Stop Signs | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| R3A | SR-1 Paved Shoulder and Curb | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| R3B | SR-1 Paved Shoulder and Curb | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| R4 | California Avenue Roundabout/ Signal | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| R5 | Cypress Avenue Multi-Lane Roundabout/ Signal | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| R6 | 16th Street Roundabout | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| R7 | Montara Lighthouse Median and Left Turn Bay | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| R8 | SR-92/SR-35 (Lower) Roundabout | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| R9 | Carlos Street Traffic Calming | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| R10 | Main Street Traffic Calming | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| R11 | SR-92 Passing/Climbing Lanes | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| R12 | SR-92 Truck Signs | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| R13 | SR-92 Left Turn Lanes | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| R14 | SR-92/SR-35 (Upper Roundabout) | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| Pe1 | Striped Pedestrian Crossing with Beacons | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| Pe2A | SR-1 Natural Pathway | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| Pe2B | SR-1 Natural Pathway | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| Pe2C | SR-1 Natural Pathway | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| Pe3 | Coronado Street and Ave Alhambra Sidewalk | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| Pe4 | Coastal Trail | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| B1 | Capistrano Road Bicycle Facilities | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| B2 | SR-92 Bike Lanes | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| B3 | SR-1 Bike Lanes | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| B4 | Airport Street Class I/II/III Bike Routes | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| B5 | Bike/Ped improvements in downtown Moss Beach | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| T1 | Bus stop amenities | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| T2 | Increased weekend Samtrans service | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| T3 | Transit Stop Improvements | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| Pa1 | Montara State Beach Parking Lot Improvements | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| Pa2 | Upper Gray Whale Cove Parking Lot Improvements | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| Pa3 | Wayfinding | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| Pa4 | Carlos Street On-Street Parking | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| Pa5 | El Granada Diagonal Parking | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |
| Pa6 | Park and Ride at Etheldore St (South Terminus) | ★ | ★ | ☆ | ★ | ☆ | ☆ | ★ |

| | | | | | |
|---|------------------------|---|--------------------------|---|-----------------------|
| ★ | High Performance Score | ★ | Medium Performance Score | ☆ | Low Performance Score |
|---|------------------------|---|--------------------------|---|-----------------------|

Table 29. Term-wise Project Implementation Priorities

| Modes | Project Implementation Priorities | | |
|------------|---|---|--|
| | Short Term | Medium Term | Long Term |
| Transit | T3: Cypress Point Housing Transit on Carlos | T1: Bus Stop Amenities (throughout study area) | |
| | T1: Bus Stop Amenities (Moss Beach at California and Carlos Street) | T2: Increased SamTrans Commute Service | |
| | T2: Increased Weekend SamTrans Service | | |
| Bicycle | B1: Capistrano Road Bicycle Facilities | B3: Class II Bicycle Lanes on Highway 1 | B2: SR-92 Class I Bike Trail Princeton to Cypress Ave |
| | | B4: Airport Street Class II Bike Routes | B3: SR-1 Class II Bicycle lanes from Half Moon Bay to Pacifica (B10) |
| | | | B2: Class II Bike Lanes on Highway 92 |
| Pedestrian | Pe1: Striped Pedestrian Crossing with Beacons at Coronado Street and Avenue Alhambra | Pe2: SR-1 Multi-modal Trail in Montara and Second Street to 16 th Street | |
| | Pe2: SR-1 Multi-modal Trail at Moss Beach | Pe2: Multi-modal Trail Capistrano to South Terminus of Etheldore | |
| | Pe2: SR-1 Multi-modal Trail in Moss Beach from 16 th Street to Etheldore South Terminus, and in Miramar and El Granada-Mirada Road to Capistrano | Pe4: Coastal Trail Montara Lighthouse to Northern Vallemar Terminus | |
| | Pe2: SR-1 Multi-modal Trail on Carlos Street Moss Beach and Coastal Trail Connections | | |
| | Pe4: Coastal Trail Improvements (Study Area) | | |
| Roadway | R1: Gray Whale Cove Turn and Acceleration Lanes | | R8: SR-92/SR-35 Roundabout |
| | R6: 16 th Street/Hwy 1 Intersection Control | | |

| Modes | Project Implementation Priorities | | |
|----------|---|---|----------------------------------|
| | Short Term | Medium Term | Long Term |
| | R4: California Avenue Intersection Control | | |
| | R5: Cypress Avenue Intersection Control | | |
| | R7: Realignment of Carlos Street northern terminus (with R6) | | |
| Land Use | LU1: Initiate and complete lot merger program (2-3 year effort) | LU2: Implement a lot retirement program if feasible | |
| Parking | Pa1: Montara State Beach Parking Lot Improvements | Pa3: Wayfinding Signage Program | Pa5: El Granada Diagonal Parking |
| | Pa2: Upper Gray Whale Cove Parking Lot Improvements | Pa4: Carlos Street On-Street Parking | |

6.3 Project Implementation

6.3.1 Overview of Project Implementation

Each of the projects proposed in Connect the Coastsides will require separate funding, design, permitting, environmental review, and construction. The Connect the Coastsides Plan is the first step in positioning these projects for funding because it establishes a coherent mobility vision for the Midcoast that articulates the Board of Supervisor’s transportation infrastructure investment priorities. The projects in Connect the Coastsides also need to be in local, regional and state transportation plans to secure funding, particularly any improvements within the State right of way, including:

Local

- San Mateo County Transportation Authority Strategic Plan
- San Mateo County Congestion Management Plan (C/CAG)
- San Mateo County Road Fund

Regional

- Plan Bay Area (Sustainable Communities Strategy and Regional Transportation Plan)

State

- California Transportation Commission, State Transportation Improvement Program

In addition to the incorporation of Connect the Coastsides Projects in local, state and regional plans, the projects can qualify for competitive grant funds from federal, state, regional sources, or in special cases, funding directly from the State Legislature in the Governor’s budget. The County of San Mateo’s Five-Year

Capital Improvement Plan (CIP) is a planning tool designed to identify short- and long-term capital improvement needs of the County and align those needs with appropriate financing, scheduling, and implementation. The County may commit general funds to Connect the Coasts projects. The County Department of Public Works administers the County's Road Fund, which includes state and federal tax monies returned to the County. Potential funding sources for each project are described in Chapter 5.

Local governments often seek grant funding to prepare project designs. These are necessary before permitting and environmental review can commence. Although Connect the Coasts is evaluated pursuant to the requirements of the California Environmental Quality Act at a programmatic level, individual projects will need specific assessments of environmental impact as part of the Coastal Development Permit process. Each project will require a Coastal Development Permit, issued by the County.

Once funding is secured and design, permitting, and environmental review are completed, projects can be published for bids. This competitive public process allows firms to compete for a project by responding to a request for proposals (bids) issued by the County. Once a contract is awarded, the contractor can mobilize and build the project.

6.3.2 Considerations for Project Implementation

Implementation of Connect the Coasts and the projects that have been identified as priorities will occur incrementally, in a variety of ways, over several decades, as funding becomes available. While some projects or components of projects could be implemented fairly quickly, some high priority projects will be longer-term, especially since implementing transportation projects can be challenging. Significant obstacles can include securing construction, design and maintenance funding, project area topography, right-of-way property acquisition, coastal erosion and flooding encroachment permits or easements, providing access for all roadway users, and meeting environmental and design standards. Implementing this plan will require partnerships between Caltrans and multiple county agencies, as well as ongoing support from the community. Partners will also need to work together to find common ground on project designs, locations, and funding mechanisms.

It is anticipated that many projects identified in this plan will be implemented independently as stand-alone projects. However, some projects or components of projects will instead be incorporated into other transportation or non-transportation projects in the Midcoast, which may be implemented by public or private entities. This may include projects under the Caltrans State Highway Operation and Protection Program (SHOPP), San Mateo County maintenance, operational, and preservation projects, land use developments, or major infrastructure modifications.

New developments and storm damage repair work provide opportunities to implement portions of some of the recommendations, especially bicycle and pedestrian improvements, even if these implementations may result in "piecemeal" facilities. Finally, outside funding (such as grants) may be available to finance the design and construction of other projects and programs that fit the criteria of those funding programs. This could result in some lower priority projects moving forward more quickly than others.

For major priority projects on Highway 1, the first step is development of a Caltrans Project Initiation Document (or PID), which includes more detailed project scoping – including development of cost estimates and identification of varying design options. The County of San Mateo, Caltrans, the San Mateo County Transportation Authority and SamTrans are also expected to continue to apply for eligible grant funding opportunities and may utilize Measure A sales tax revenues to serve as match to leverage other

funds. Transportation agencies (Caltrans, County, SMCTA, MTC, etc.) should consider partnering with one another, as well as MCC, non-profits, and private sector partners to better compete for Federal and State funding opportunities.

6.4 Next Steps – Priority and Ongoing Actions

Following adoption of the Connect the Coasts by the Board of Supervisors, County staff will pursue a number of implementation actions independently and in partnership with others to advance the projects identified in Connect the Coasts. The Priority Actions will focus on low hanging fruit, and high-priority projects, however the following preliminary list of priority actions will begin following implementation:

Table 30. Priority and Ongoing Implementation Actions

| Priority Actions | | |
|--|--|--------------------------------|
| Implementation Action | Lead and Partners | Project Completion Date |
| Implement Lot Merger Program | County Planning Staff | June 2020 |
| Complete Project Implementation Documents for Moss Beach Roundabouts | County Planning Staff with Consultant Assistance | June 2020 |
| Complete Construction of Phase 1 of Multi-modal Trail | San Mateo County Department of Planning and Building | December 2021 |
| Develop Transportation Impact Fee Ordinance for Public Review and Board Adoption | San Mateo County Departments of Planning and Building and Public Works | December 2020 |
| Integrate Connect the Coastside projects in local and state transportation plans | County Planning Staff, City/County Association of Governments and California Transportation Commission | December 2020 |
| Ongoing Actions | | |
| Transportation Facility Maintenance | Caltrans, San Mateo County Departments of Parks and Public Works, California State Parks, GGNRA, Private Land Owners | |
| Monitor Auto Traffic | Caltrans, Public Works, Project Sponsors | |
| Monitor Building Permits for New Construction | San Mateo County Departments of Planning and Building | |
| Seek, Obtain and Provide Grant Funds for Connect the Coastside Projects | San Mateo County Departments of Planning and Building, Parks, and Public Works; California State Parks, C/CAG | |
| Collaborated with SamTrans and C/CAG on Bus Service Improvements | San Mateo County Departments of Planning and Building | |

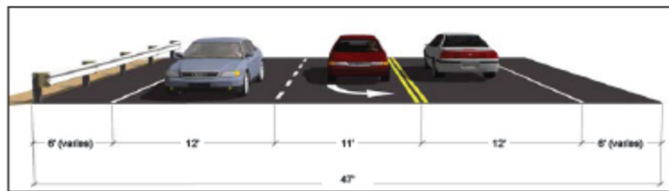
Appendix A

Project Concept Diagrams

Project Concept Diagrams

| Project # | Project Name | Page |
|-----------|---|------|
| R1 | Gray Whale Cove Turn and Acceleration Lanes | 1 |
| R2 | SR-1 Side-Street Stop Signs | 2 |
| R3 | SR-1 Shoulder Treatment | 3 |
| R4 | California Avenue Intersection Control (Signal or Roundabout) | 5 |
| R5 | Cypress Avenue Intersection Control (Signal or Roundabout) | 6 |
| R6 | 16th Street Roundabout | 8 |
| R7 | Carlos Street Terminus Realignment | 9 |
| R8 | SR-92/SR-35 (Lower) Roundabout | 10 |
| R9 | Carlos Street Traffic Calming | 11 |
| R10 | Main Street Traffic Calming | 12 |
| R11 | SR-92 Passing/Climbing Lanes | 13 |
| R12 | SR-92 Truck Signs | 14 |
| R14 | SR-92/SR-35 (Upper) Roundabout | 15 |
| Pe1 | Striped Pedestrian Crossing with Beacons | 16 |
| Pe2 | SR-1 Parallel Trail | 17 |
| Pe3 | Coronado Street and Obispo Road Sidewalk | 31 |
| B1 | Capistrano Road Bicycle Facilities | 32 |
| B2 | SR-92 Bike Lanes | 33 |
| B4 | Airport Street Class I/II/III Bike Routes | 34 |
| Pa1 | Montara State Beach Parking Lot Improvements | 35 |
| Pa2 | Upper Gray Whale Cove Parking Lot Improvements | 36 |
| Pa3 | Wayfinding | 37 |
| Pa4 | Carlos Street On-Street Parking | 38 |
| Pa5 | El Granada Diagonal Parking | 39 |

Project R1: Gray Whale Cove Turn and Acceleration Lanes



Project R2: SR-1 Side-Street Stop Signs

Locations Missing Stop Signs:

El Granada

SR-1 and Furtado Lane

Montara

SR-1 and 16th Street

SR-1 and 13th Street

SR-1 and 11th Street

SR-1 and 7th Street

SR-1 and Seacliff Ct

SR-1 and 1st Street

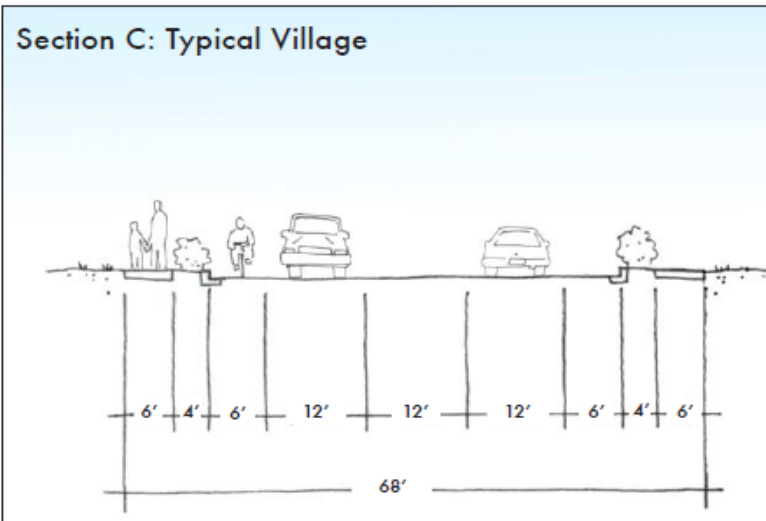
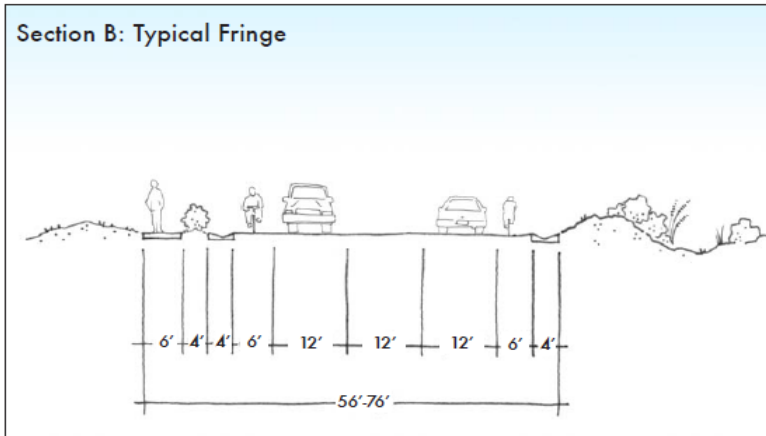


R1-1

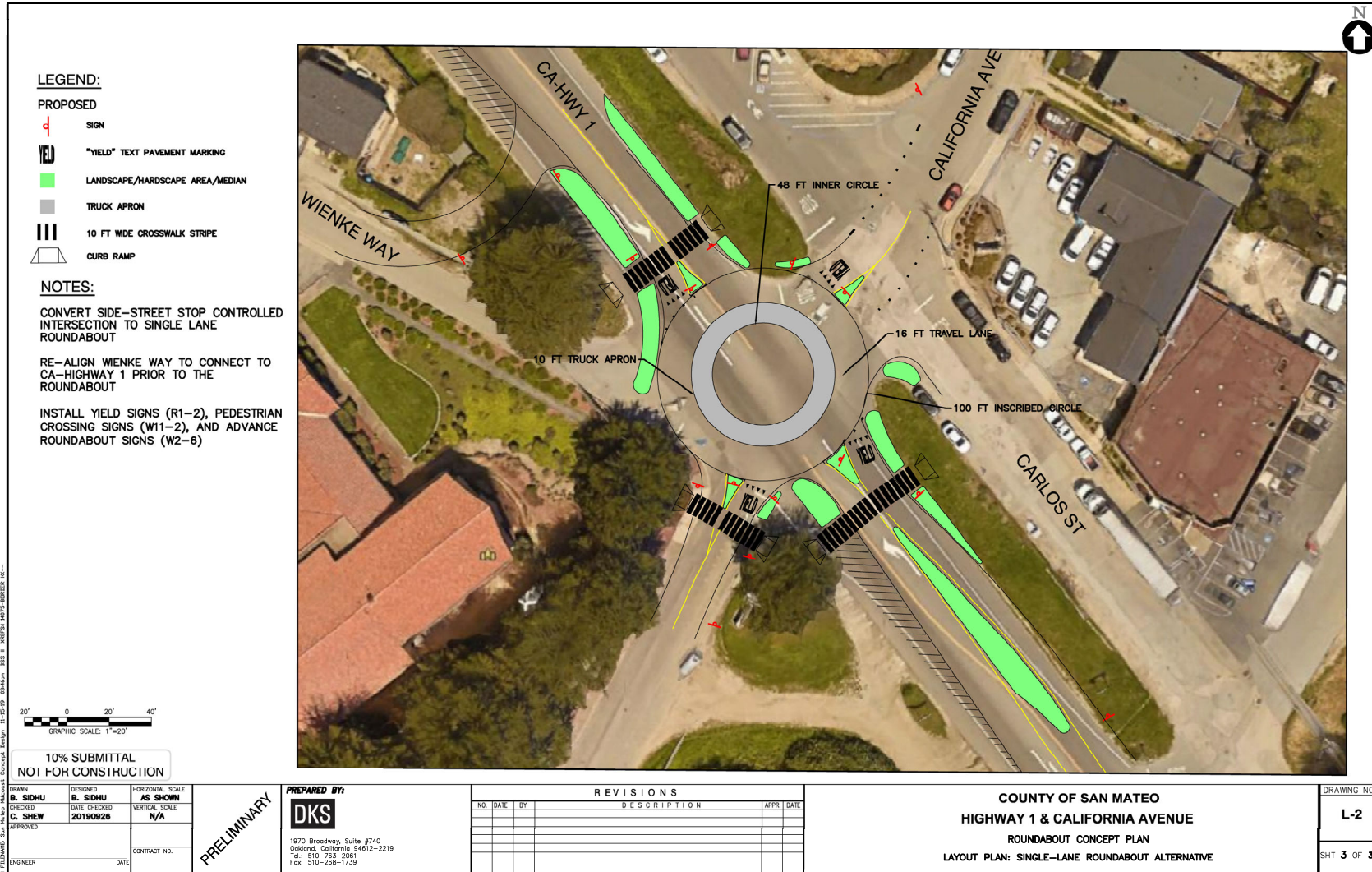
Highway 1 Context Zones



Project R3: SR-1 Paved Shoulder and Curb



Project R4: California Ave Roundabout



LEGEND:

PROPOSED

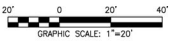
- SIGN
- "YIELD" TEXT PAVEMENT MARKING
- LANDSCAPE/HARDSCAPE AREA/MEDIAN
- TRUCK APRON
- 10 FT WIDE CROSSWALK STRIPE
- CURB RAMP

NOTES:

CONVERT SIDE-STREET STOP CONTROLLED INTERSECTION TO SINGLE LANE ROUNDABOUT

RE-ALIGN WIENKE WAY TO CONNECT TO CA-HIGHWAY 1 PRIOR TO THE ROUNDABOUT

INSTALL YIELD SIGNS (R1-2), PEDESTRIAN CROSSING SIGNS (W11-2), AND ADVANCE ROUNDABOUT SIGNS (W2-6)



**10% SUBMITTAL
NOT FOR CONSTRUCTION**

1. PREPARED BY: M. N. M. CONSULTING ENGINEERS, INC. 11-20-09 2010/09/26 SEE 8. 10/27/10 10/27/10/09/26/10

| | | |
|---------------------------|-----------------------------------|-------------------------------------|
| DRAWN B. SIDHU | DESIGNED B. SIDHU | HORIZONTAL SCALE AS SHOWN |
| CHECKED C. SHEW | DATE CHECKED 2010/09/26 | VERTICAL SCALE N/A |
| APPROVED | | |
| ENGINEER | DATE | CONTRACT NO. |

PRELIMINARY

PREPARED BY:

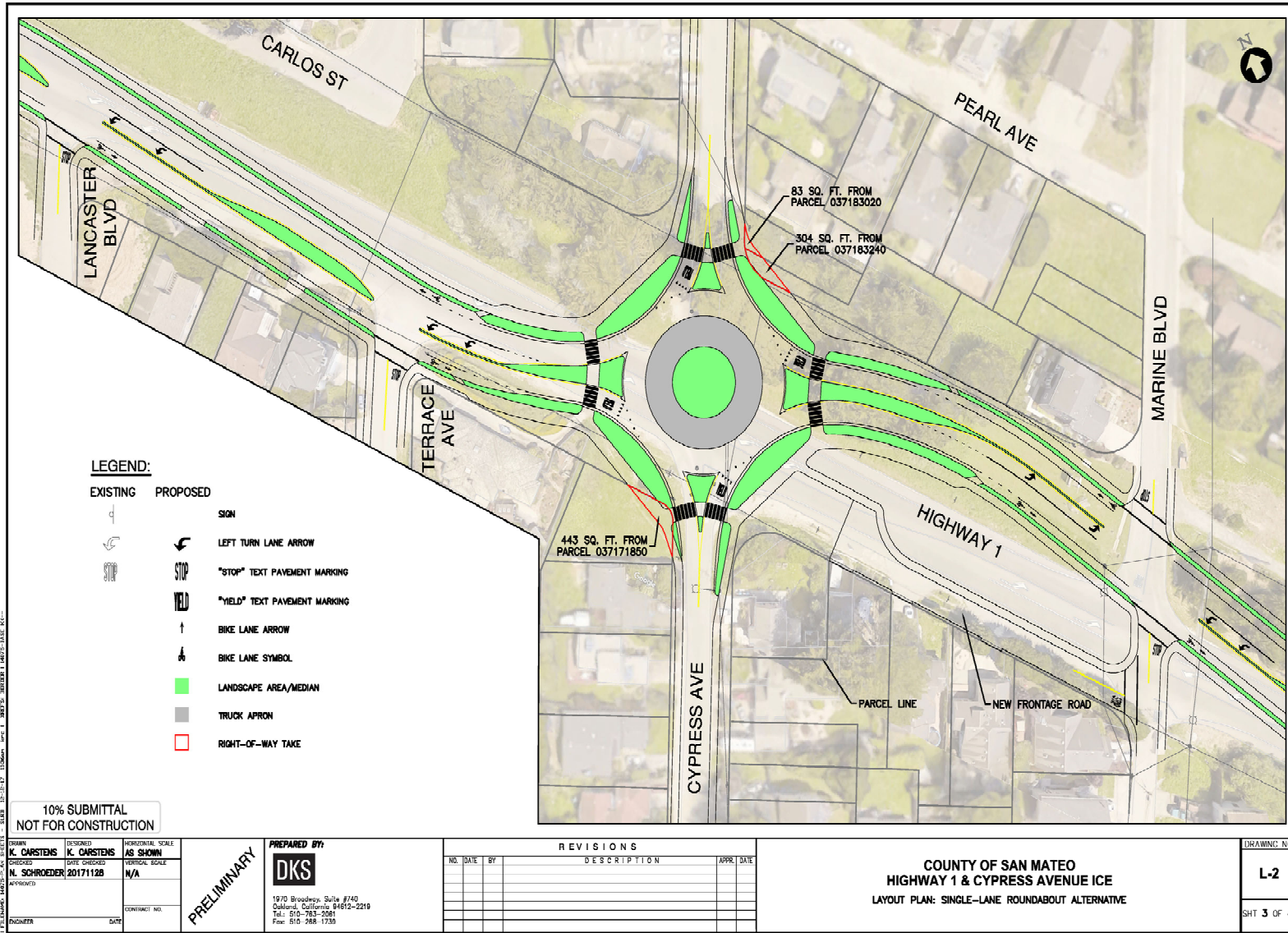
 1970 Broadway, Suite #740
 Oakland, California 94612-2219
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 Fax: 510-268-1739

| REVISIONS | | | |
|-----------|------|----|-------------|
| NO. | DATE | BY | DESCRIPTION |
| | | | |
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COUNTY OF SAN MATEO
HIGHWAY 1 & CALIFORNIA AVENUE
ROUNDABOUT CONCEPT PLAN
LAYOUT PLAN: SINGLE-LANE ROUNDABOUT ALTERNATIVE

DRAWING NO.
L-2
SHT 3 OF 3

Project R5: Cypress Ave Single Lane Roundabout



LEGEND:

- | | | |
|-----------------|-----------------|-------------------------------|
| EXISTING | PROPOSED | SIGN |
| | | LEFT TURN LANE ARROW |
| | | "STOP" TEXT PAVEMENT MARKING |
| | | "YIELD" TEXT PAVEMENT MARKING |
| | | BIKE LANE ARROW |
| | | BIKE LANE SYMBOL |
| | | LANDSCAPE AREA/MEDIAN |
| | | TRUCK APRON |
| | | RIGHT-OF-WAY TAKE |

10% SUBMITTAL
NOT FOR CONSTRUCTION

| | | |
|-----------------------------------|--------------------------------|------------------------------|
| DESIGNED BY K. CARSTENS | DESIGNED K. CARSTENS | HORIZONTAL SCALE AS SHOWN |
| CHECKED BY N. SCHROEDER | DATE CHECKED 20171128 | VERTICAL SCALE N/A |
| APPROVED BY | | |
| ENGINEER | DATE | CONTRACT NO. |

PRELIMINARY

PREPARED BY:
DKS
1970 Broadway, Suite #740
Oakland, California 94612-2219
Tel: 510-783-2081
Fax: 510-788-1735

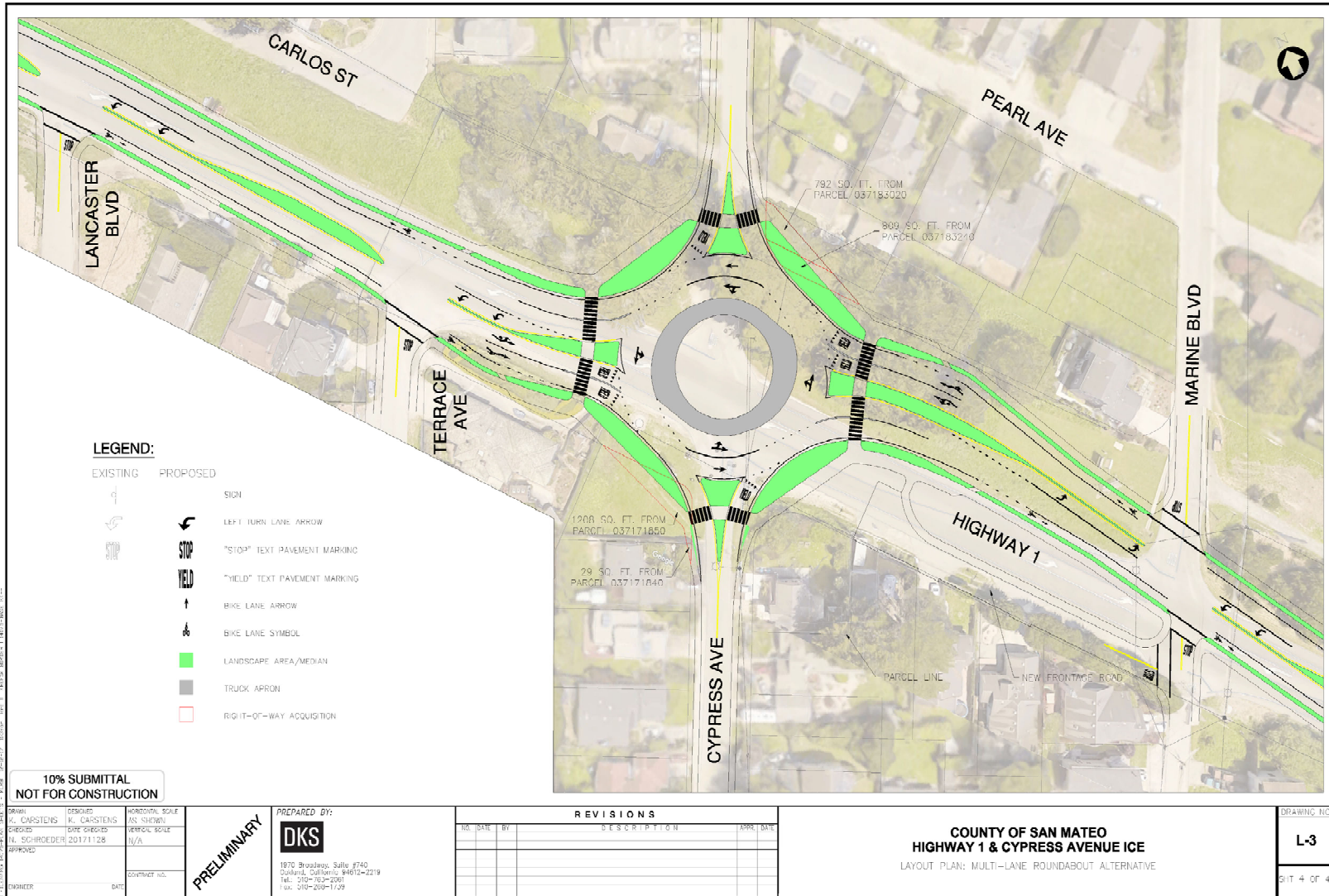
| REVISIONS | | | | | |
|-----------|------|----|-------------|-------|------|
| NO. | DATE | BY | DESCRIPTION | APPR. | DATE |
| | | | | | |
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**COUNTY OF SAN MATEO
HIGHWAY 1 & CYPRESS AVENUE ICE
LAYOUT PLAN: SINGLE-LANE ROUNDABOUT ALTERNATIVE**

DRAWING NO.
L-2
SHT 3 OF 4

FILED: 11/28/17 10:58 AM 2017 11/28/17 10:58 AM 2017 11/28/17 10:58 AM 2017 11/28/17 10:58 AM

Project R5: Cypress Ave Multi-Lane Roundabout









Project R6: 16th St Roundabout



LEGEND:

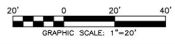
PROPOSED

-  SIGN
-  "YIELD" TEXT PAVEMENT MARKING
-  LANDSCAPE/HARDSCAPE AREA/MEDIAN
-  TRUCK APRON
-  10 FT WIDE CROSSWALK STRIPE
-  CURB RAMP

NOTES:

CONVERT SIDE-STREET STOP CONTROLLED INTERSECTION TO SINGLE LANE ROUNDABOUT

INSTALL YIELD SIGNS (R1-2), PEDESTRIAN CROSSING SIGNS (W11-2), AND ADVANCE ROUNDABOUT SIGNS (W2-6)



10% SUBMITTAL
NOT FOR CONSTRUCTION

| | | |
|--------------------------------|----------------------------------|-------------------------------------|
| DESIGNED BY B. SIDHU | DESIGNED DATE 20190928 | HORIZONTAL SCALE AS SHOWN |
| CHECKED BY C. SHEW | CHECKED DATE 20190928 | VERTICAL SCALE N/A |
| APPROVED BY | DATE | CONTRACT NO. |
| ENGINEER | | |

PRELIMINARY

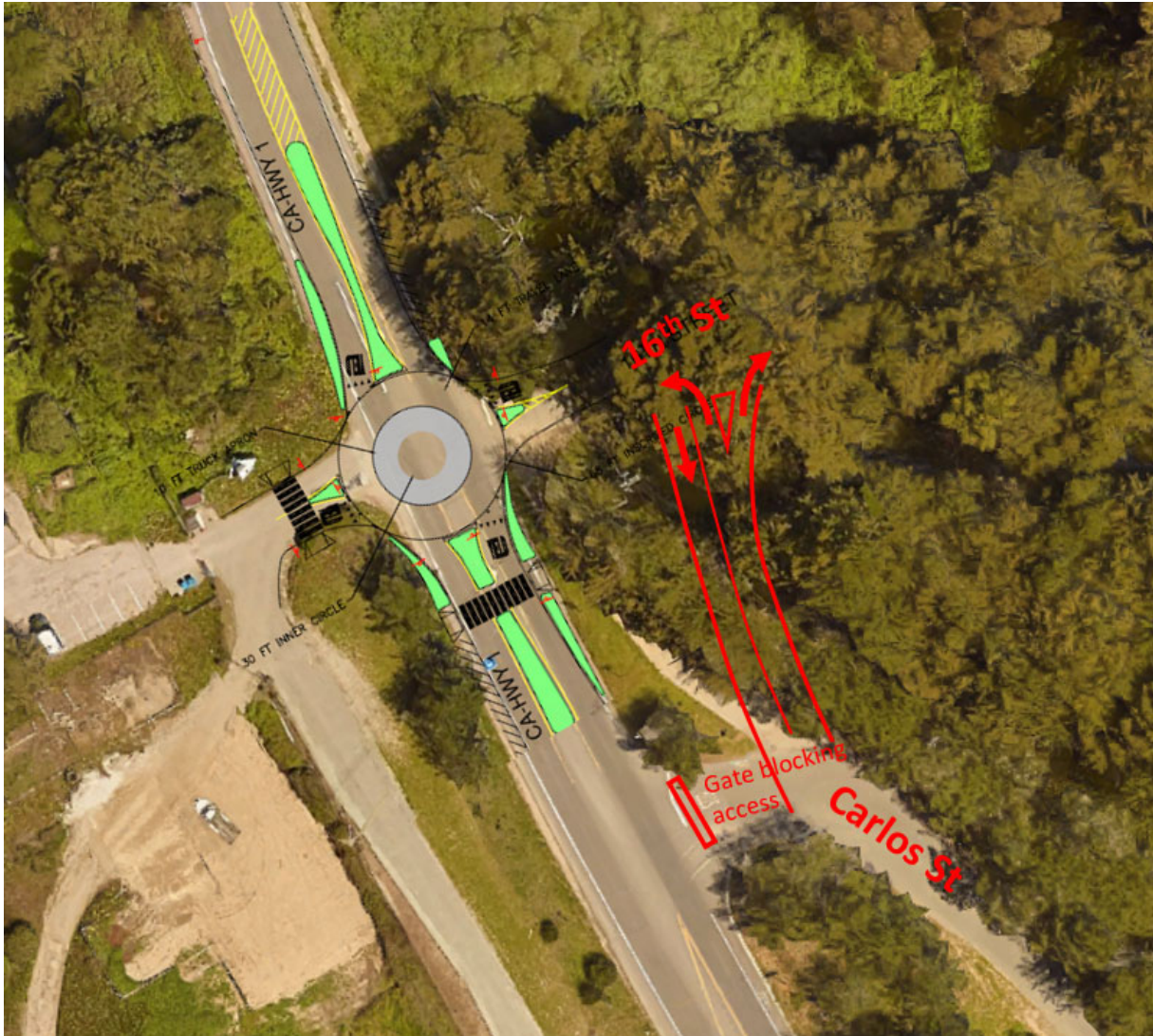
PREPARED BY:
DKS
1070 Broadway, Suite #740
Oakland, California 94612-2219
Tel: 510-763-2061
Fax: 510-268-1739

| REVISIONS | | | | |
|-----------|------|----|-------------|------------|
| NO. | DATE | BY | DESCRIPTION | APPR. DATE |
| | | | | |
| | | | | |
| | | | | |
| | | | | |

COUNTY OF SAN MATEO
HIGHWAY 1 & 16TH STREET
ROUNDBOUT CONCEPT PLAN
LAYOUT PLAN: SINGLE-LANE ROUNDABOUT ALTERNATIVE

| |
|---------------------------|
| DRAWING NO. L-1 |
| SHT 2 OF 3 |

Project R7: Carlos St Terminus Realignment



R8: SR-92/SR-35 (Lower) Roundabout

Concept Diagram under development

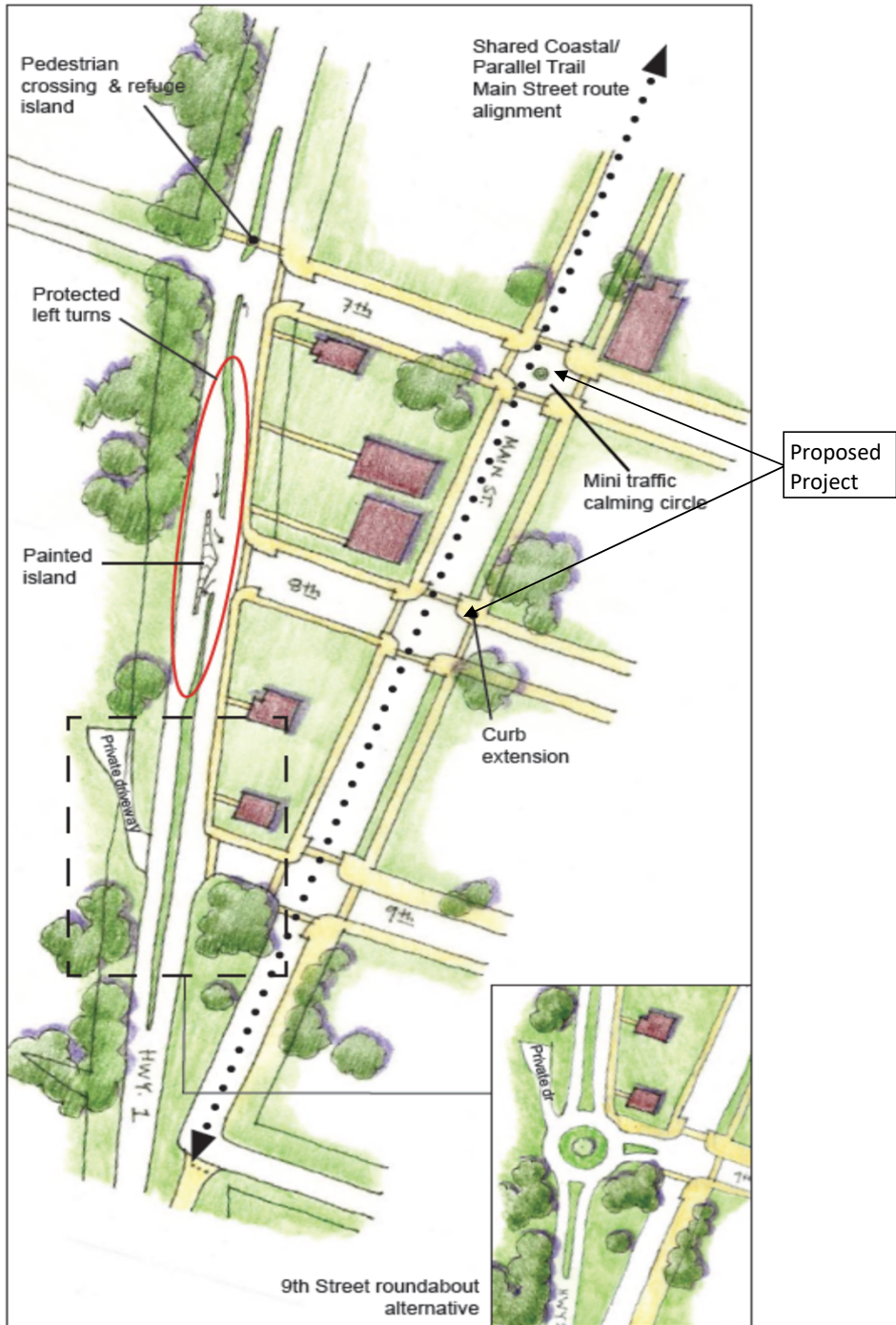


R9: Carlos Street Traffic Calming

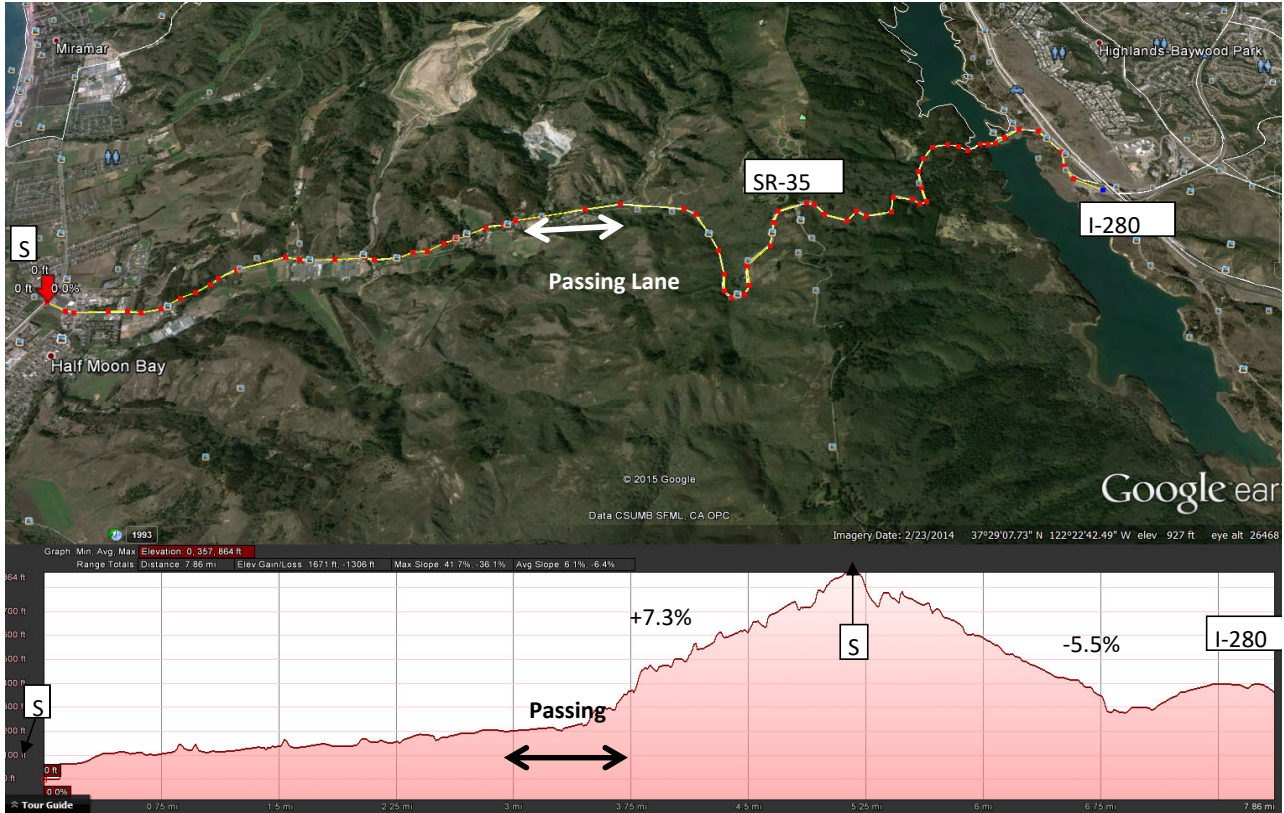
Concept under Development



R10: Main Street Traffic Calming



R11: SR-92 Passing/Climbing Lanes



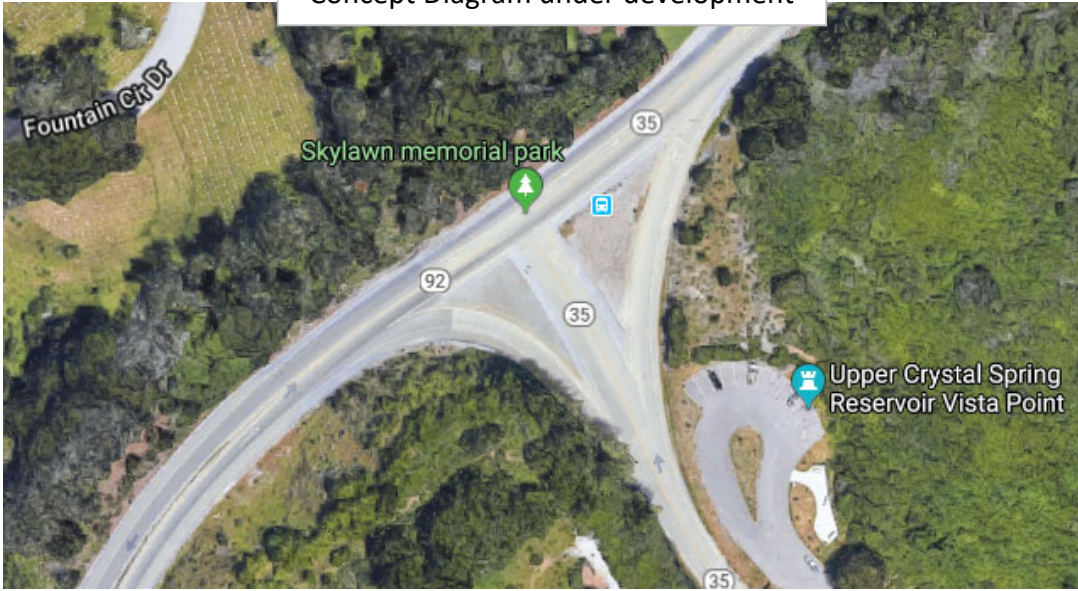
R12: SR-92 Truck Signs



R4-5

Project R14: SR-92/SR-35 (Upper) Roundabout

Concept Diagram under development



Project Pe1: Striped Pedestrian Crossing with Beacons

Crossing Locations:

- Gray Whale Cove
- Montara State Beach
- 2nd Street (median refuge, no flashing beacon)
- 7th Street
- Moss Beach Lighthouse (16th Street)
- Half Moon Bay Airport
- North Capistrano Road
- Surfer's Beach Parking Area, north of Coronado Street
- Between Magellan Avenue and Medio Avenue
- Mirada Road
- Purisima Way
- Redondo Beach Road
- Quarry Road (along SR-92)
- Pilarcitos Creek Road (along SR-92)
- SR-35 (along SR-92)












Parallel Trail Proposed Alignment - Sheet 1b



Coastal Trail follows Parallel Trail alignment

Legend 

-  Class I Trail
-  Sidewalk + Class III Sharrows
-  Coastal Trail
-  Simple Crossing
-  Striped Crossing
-  Beaconed Crossing
-  Roundabout



Parallel Trail Proposed Alignment - Sheet 1c





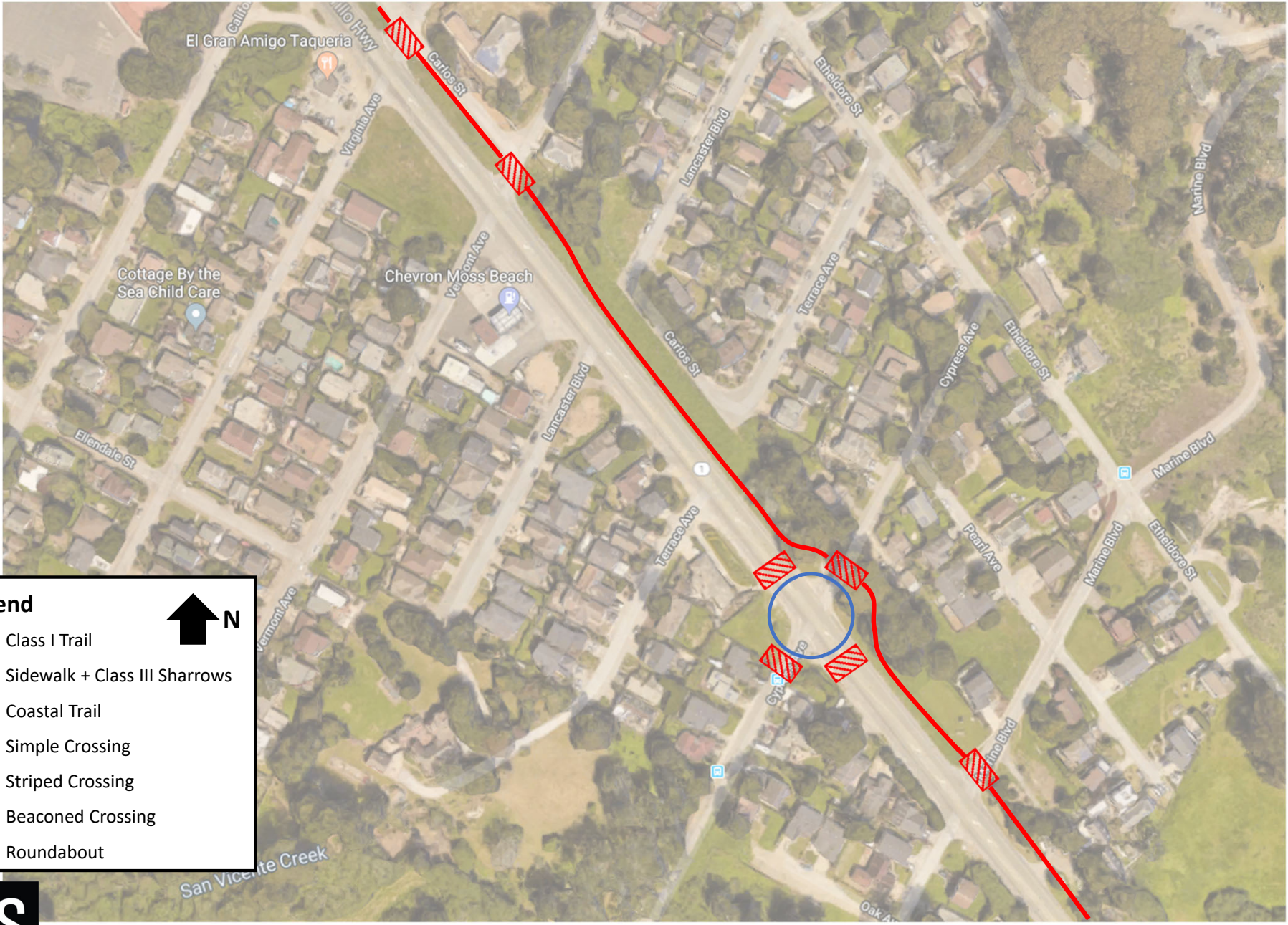
Legend

 Class I Trail
 Sidewalk + Class III Sharrows
 Coastal Trail
 Simple Crossing
 Striped Crossing
 Beaconned Crossing
 Roundabout








 N


DKS

Parallel Trail Proposed Alignment - Sheet 1e



Legend

-  Class I Trail
-  Sidewalk + Class III Sharrows
-  Coastal Trail
-  Simple Crossing
-  Striped Crossing
-  Beaconed Crossing
-  Roundabout








 N




Parallel Trail Proposed Alignment - Sheet 1f



Legend


-  Class I Trail
-  Sidewalk + Class III Sharrows
-  Coastal Trail
-  Simple Crossing
-  Striped Crossing
-  Beaconned Crossing
-  Roundabout








 N



Parallel Trail Proposed Alignment - Sheet 1g



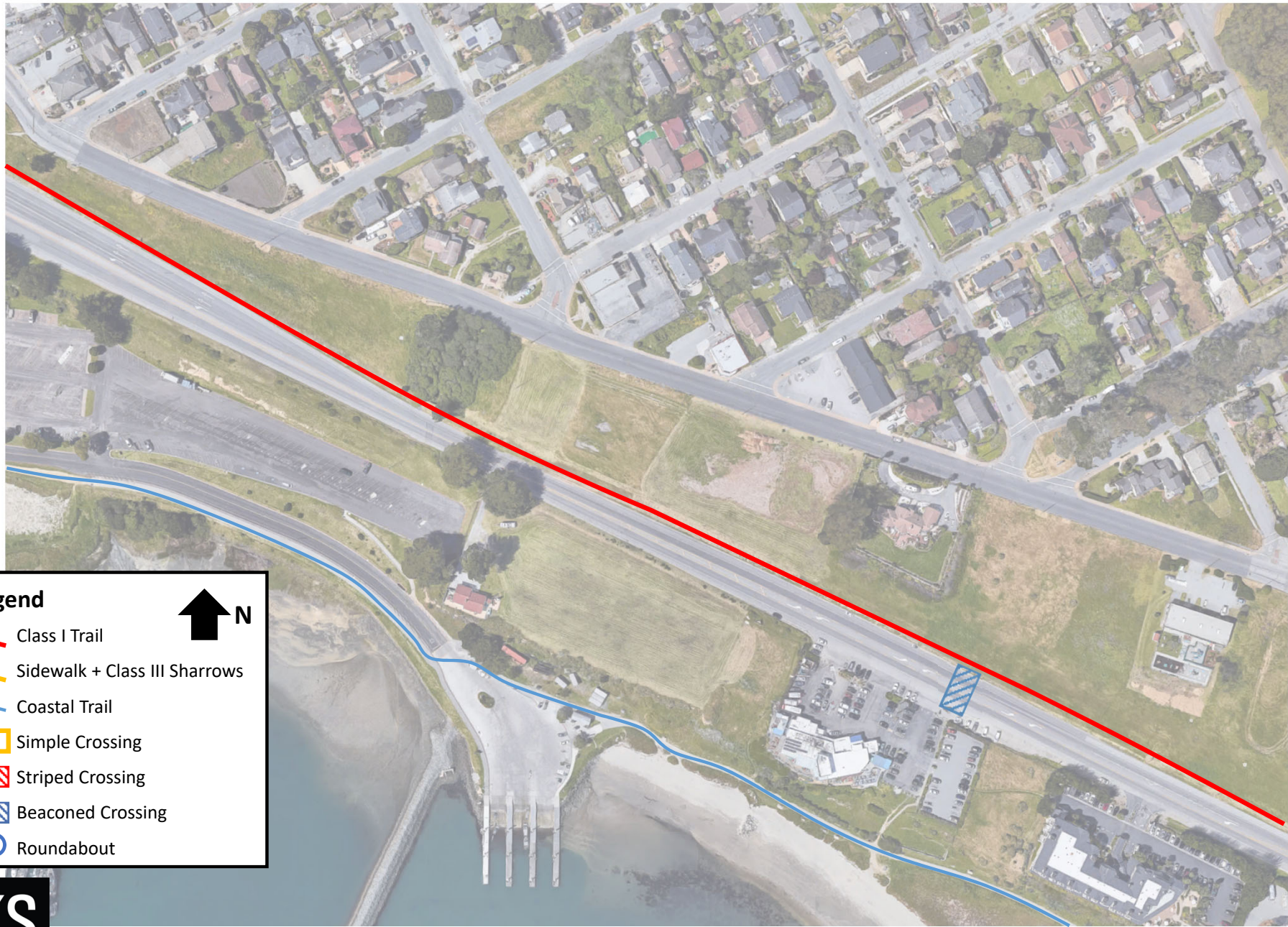
Legend 

-  Class I Trail
-  Sidewalk + Class III Sharrows
-  Coastal Trail
-  Simple Crossing
-  Striped Crossing
-  Beaconed Crossing
-  Roundabout



Parallel Trail Proposed Alignment - Sheet 2a





Legend

- Class I Trail
- Sidewalk + Class III Sharrows
- Coastal Trail
- Simple Crossing
- Striped Crossing
- Beaconed Crossing
- Roundabout


N










Parallel Trail Proposed Alignment - Sheet 2c





Legend 

-  Class I Trail
-  Sidewalk + Class III Sharrows
-  Coastal Trail
-  Simple Crossing
-  Striped Crossing
-  Beaconned Crossing
-  Roundabout

DKS

Parallel Trail Proposed Alignment - Sheet 2e

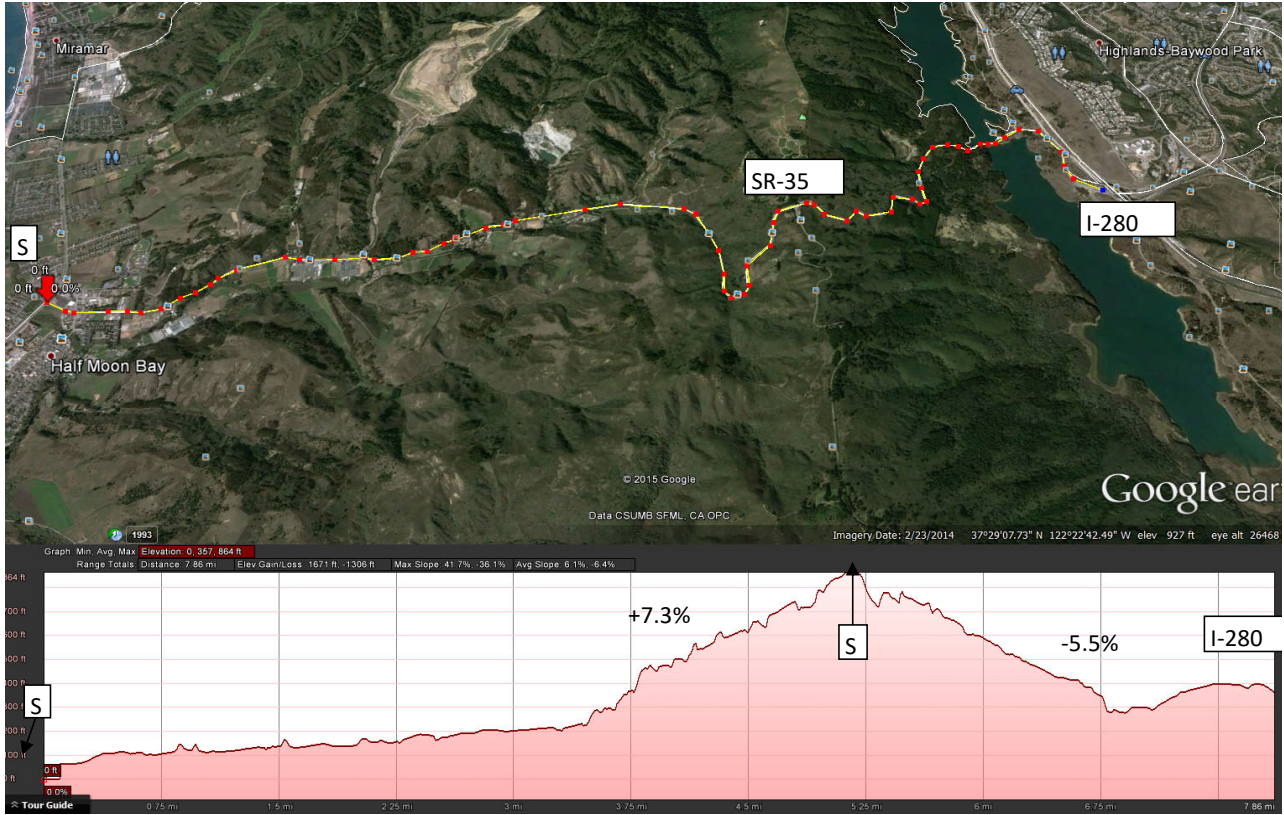
Project Pe3: Coronado Street and Ave Alhambra Sidewalk



Project B1: Capistrano Road Bicycle Facilities



Project B2: SR-92 Bike Lanes

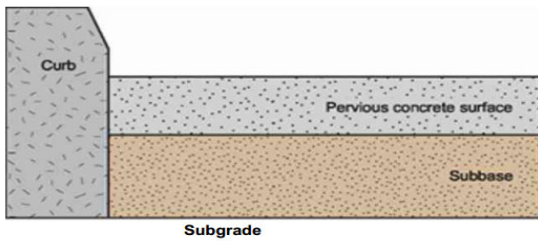
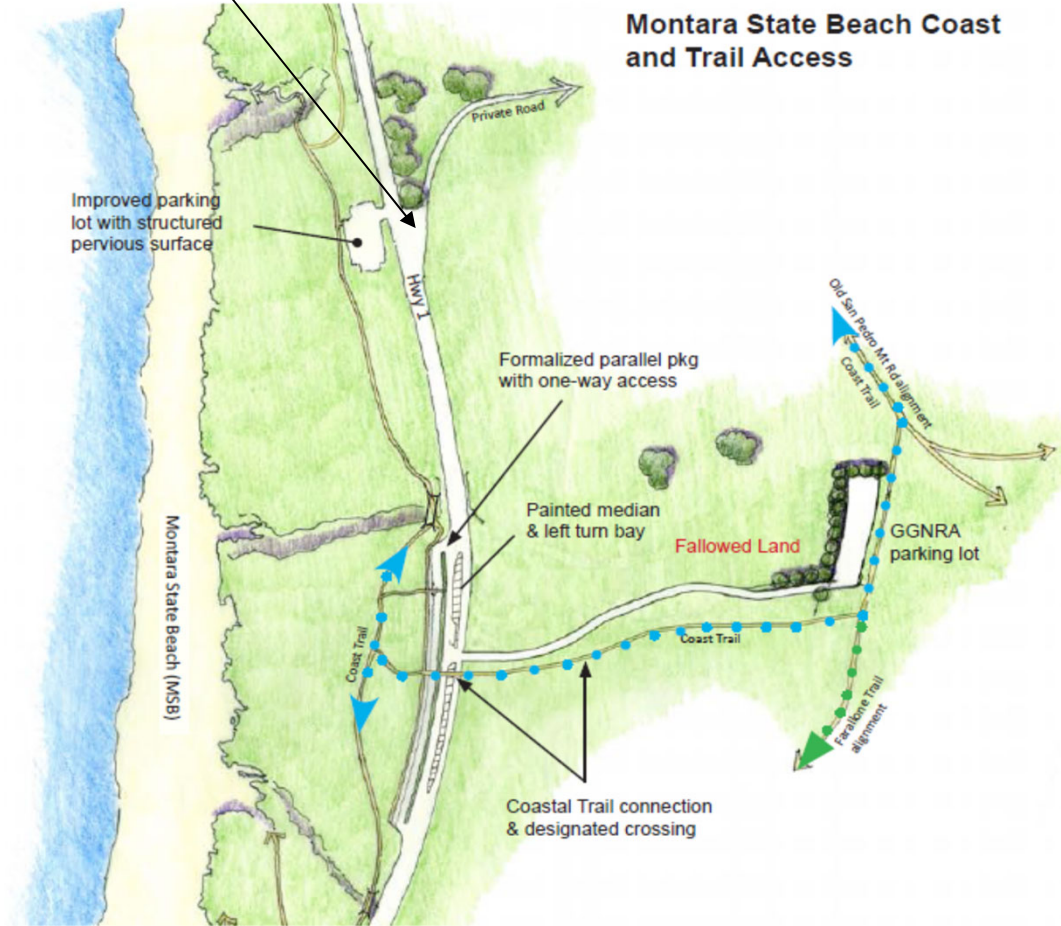


Project B4: Airport Class II Bike Lanes



Project Pa1: Montara State Beach Parking

Project

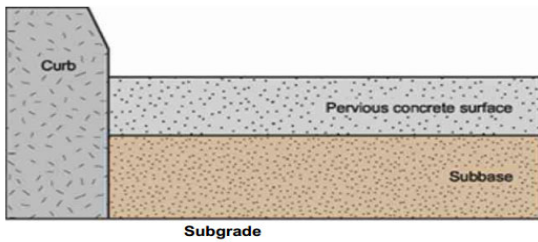


Project Pa2: Gray Whale Cove Parking Lot Improvements

Gray Whale Cove Parking Lot



Project Limits



Project Pa3: Wayfinding



Pa4: Carlos Street On-Street Parking

Concept under Development



Pa5: El Granada Diagonal Parking

Concept under Development



Appendix B Cost Estimates

(Under Construction)

Appendix C




















Technical Analysis Worksheets and Reports

SR-1 Existing Conditions Report

HCM Unsignalized Intersection Capacity Analysis

1: SR-1 & 2nd St


















9/18/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | |  | |  |  | |  |  |  |
| Volume (veh/h) | 0 | 0 | 0 | 12 | 0 | 121 | 0 | 553 | 10 | 27 | 242 | 0 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 13 | 0 | 127 | 0 | 582 | 11 | 28 | 255 | 0 |
| Pedestrians | | | | | | | | | | | | 3 |
| Lane Width (ft) | | | | | | | | | | | | 12.0 |
| Walking Speed (ft/s) | | | | | | | | | | | | 4.0 |
| Percent Blockage | | | | | | | | | | | | 0 |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | | None |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1024 | 904 | 255 | 899 | 899 | 590 | 255 | | | 593 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1024 | 904 | 255 | 899 | 899 | 590 | 255 | | | 593 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 100 | 95 | 100 | 75 | 100 | | | 97 | | |
| cM capacity (veh/h) | 156 | 269 | 784 | 254 | 271 | 506 | 1310 | | | 983 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 0 | 140 | 0 | 593 | 28 | 255 | | | | | | |
| Volume Left | 0 | 13 | 0 | 0 | 28 | 0 | | | | | | |
| Volume Right | 0 | 127 | 0 | 11 | 0 | 0 | | | | | | |
| cSH | 1700 | 464 | 1700 | 1700 | 983 | 1700 | | | | | | |
| Volume to Capacity | 0.00 | 0.30 | 0.00 | 0.35 | 0.03 | 0.15 | | | | | | |
| Queue Length 95th (ft) | 0 | 31 | 0 | 0 | 2 | 0 | | | | | | |
| Control Delay (s) | 0.0 | 16.1 | 0.0 | 0.0 | 8.8 | 0.0 | | | | | | |
| Lane LOS | A | C | | | A | | | | | | | |
| Approach Delay (s) | 0.0 | 16.1 | 0.0 | | 0.9 | | | | | | | |
| Approach LOS | A | C | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | | 2.5 | | | | | | | | |
| Intersection Capacity Utilization | | | 45.3% | | ICU Level of Service | | A | | | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

2: SR-1 & 7th St

9/18/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | | |  |  |  | | |  | |
| Volume (veh/h) | 1 | 0 | 2 | 0 | 0 | 20 | 0 | 548 | 1 | 0 | 251 | 1 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Hourly flow rate (vph) | 1 | 0 | 2 | 0 | 0 | 21 | 0 | 571 | 1 | 0 | 261 | 1 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 854 | 834 | 262 | 835 | 834 | 571 | 262 | | | 572 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 854 | 834 | 262 | 835 | 834 | 571 | 262 | | | 572 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 100 | 100 | 100 | 96 | 100 | | | 100 | | |
| cM capacity (veh/h) | 268 | 304 | 777 | 286 | 304 | 520 | 1302 | | | 1001 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | | | | | | | |
| Volume Total | 3 | 21 | 0 | 572 | 262 | | | | | | | |
| Volume Left | 1 | 0 | 0 | 0 | 0 | | | | | | | |
| Volume Right | 2 | 21 | 0 | 1 | 1 | | | | | | | |
| cSH | 475 | 520 | 1700 | 1700 | 1700 | | | | | | | |
| Volume to Capacity | 0.01 | 0.04 | 0.00 | 0.34 | 0.15 | | | | | | | |
| Queue Length 95th (ft) | 0 | 3 | 0 | 0 | 0 | | | | | | | |
| Control Delay (s) | 12.6 | 12.2 | 0.0 | 0.0 | 0.0 | | | | | | | |
| Lane LOS | B | B | | | | | | | | | | |
| Approach Delay (s) | 12.6 | 12.2 | 0.0 | | 0.0 | | | | | | | |
| Approach LOS | B | B | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 0.3 | | | | | | | | | |
| Intersection Capacity Utilization | | | 45.6% | | ICU Level of Service | | | | A | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

3: SR-1 & 8th St

9/18/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 96 | 21 | 524 | 14 | 3 | 248 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 |
| Hourly flow rate (vph) | 97 | 21 | 529 | 14 | 3 | 251 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 793 | 536 | | | 543 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 793 | 536 | | | 543 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 73 | 96 | | | 100 | |
| cM capacity (veh/h) | 356 | 544 | | | 1025 | |

| Direction, Lane # | WB 1 | NB 1 | SB 1 |
|------------------------|------|------|------|
| Volume Total | 118 | 543 | 254 |
| Volume Left | 97 | 0 | 3 |
| Volume Right | 21 | 14 | 0 |
| cSH | 380 | 1700 | 1025 |
| Volume to Capacity | 0.31 | 0.32 | 0.00 |
| Queue Length 95th (ft) | 33 | 0 | 0 |
| Control Delay (s) | 18.7 | 0.0 | 0.1 |
| Lane LOS | C | | A |
| Approach Delay (s) | 18.7 | 0.0 | 0.1 |
| Approach LOS | C | | |

| Intersection Summary | | | |
|-----------------------------------|-------|-----|------------------------|
| Average Delay | | 2.5 | |
| Intersection Capacity Utilization | 41.7% | | ICU Level of Service A |
| Analysis Period (min) | 15 | | |

HCM Unsignalized Intersection Capacity Analysis

4: SR-1 & Carlos St

9/18/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 0 | 15 | 551 | 0 | 4 | 465 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Hourly flow rate (vph) | 0 | 16 | 586 | 0 | 4 | 495 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1089 | 586 | | | 586 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1089 | 586 | | | 586 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 100 | 97 | | | 100 | |
| cM capacity (veh/h) | 237 | 510 | | | 989 | |




















| Direction, Lane # | WB 1 | NB 1 | SB 1 | SB 2 |
|------------------------|------|------|------|------|
| Volume Total | 16 | 586 | 4 | 495 |
| Volume Left | 0 | 0 | 4 | 0 |
| Volume Right | 16 | 0 | 0 | 0 |
| cSH | 510 | 1700 | 989 | 1700 |
| Volume to Capacity | 0.03 | 0.34 | 0.00 | 0.29 |
| Queue Length 95th (ft) | 2 | 0 | 0 | 0 |
| Control Delay (s) | 12.3 | 0.0 | 8.7 | 0.0 |
| Lane LOS | B | | A | |
| Approach Delay (s) | 12.3 | 0.0 | 0.1 | |
| Approach LOS | B | | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|------------------------|
| Average Delay | | 0.2 | |
| Intersection Capacity Utilization | | 39.0% | ICU Level of Service A |
| Analysis Period (min) | | 15 | |

HCM Unsignalized Intersection Capacity Analysis



















5: SR-1 & Vallemar St

9/18/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | |  | |  |  | |  |  |  |
| Volume (veh/h) | 3 | 0 | 3 | 8 | 0 | 27 | 0 | 513 | 2 | 11 | 446 | 1 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Hourly flow rate (vph) | 3 | 0 | 3 | 8 | 0 | 28 | 0 | 534 | 2 | 11 | 465 | 1 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1051 | 1024 | 465 | 1026 | 1024 | 535 | 466 | | | 536 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1051 | 1024 | 465 | 1026 | 1024 | 535 | 466 | | | 536 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 98 | 100 | 99 | 96 | 100 | 95 | 100 | | | 99 | | |
| cM capacity (veh/h) | 193 | 233 | 597 | 210 | 233 | 545 | 1096 | | | 1032 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 6 | 36 | 0 | 536 | 11 | 466 | | | | | | |
| Volume Left | 3 | 8 | 0 | 0 | 11 | 0 | | | | | | |
| Volume Right | 3 | 28 | 0 | 2 | 0 | 1 | | | | | | |
| cSH | 291 | 399 | 1700 | 1700 | 1032 | 1700 | | | | | | |
| Volume to Capacity | 0.02 | 0.09 | 0.00 | 0.32 | 0.01 | 0.27 | | | | | | |
| Queue Length 95th (ft) | 2 | 7 | 0 | 0 | 1 | 0 | | | | | | |
| Control Delay (s) | 17.6 | 14.9 | 0.0 | 0.0 | 8.5 | 0.0 | | | | | | |
| Lane LOS | C | B | | | A | | | | | | | |
| Approach Delay (s) | 17.6 | 14.9 | 0.0 | | 0.2 | | | | | | | |
| Approach LOS | C | B | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 0.7 | | | | | | | | | |
| Intersection Capacity Utilization | | | 37.1% | ICU Level of Service | A | | | | | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis
6: SR-1 & California Ave



















9/18/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | |  | |  |  | |  |  | |
| Volume (veh/h) | 6 | 1 | 10 | 33 | 0 | 12 | 4 | 506 | 34 | 9 | 460 | 0 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 6 | 1 | 11 | 35 | 0 | 13 | 4 | 544 | 37 | 10 | 495 | 0 |
| Pedestrians | | | | | | | | | | | | 2 |
| Lane Width (ft) | | | | | | | | | | | | 12.0 |
| Walking Speed (ft/s) | | | | | | | | | | | | 4.0 |
| Percent Blockage | | | | | | | | | | | | 0 |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | | None |
| Median storage veh | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1082 | 1103 | 495 | 1096 | 1085 | 564 | 495 | | | 581 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1082 | 1103 | 495 | 1096 | 1085 | 564 | 495 | | | 581 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 97 | 99 | 98 | 81 | 100 | 98 | 100 | | | 99 | | |
| cM capacity (veh/h) | 188 | 208 | 575 | 184 | 214 | 524 | 1069 | | | 993 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 18 | 48 | 4 | 581 | 10 | 495 | | | | | | |
| Volume Left | 6 | 35 | 4 | 0 | 10 | 0 | | | | | | |
| Volume Right | 11 | 13 | 0 | 37 | 0 | 0 | | | | | | |
| cSH | 314 | 223 | 1069 | 1700 | 993 | 1700 | | | | | | |
| Volume to Capacity | 0.06 | 0.22 | 0.00 | 0.34 | 0.01 | 0.29 | | | | | | |
| Queue Length 95th (ft) | 5 | 20 | 0 | 0 | 1 | 0 | | | | | | |
| Control Delay (s) | 17.2 | 25.6 | 8.4 | 0.0 | 8.7 | 0.0 | | | | | | |
| Lane LOS | C | D | A | | A | | | | | | | |
| Approach Delay (s) | 17.2 | 25.6 | 0.1 | | 0.2 | | | | | | | |
| Approach LOS | C | D | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 1.4 | | | | | | | | | |
| Intersection Capacity Utilization | | | 40.9% | ICU Level of Service | A | | | | | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

7: SR-1 & Virginia Ave

9/18/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | |  | |  |  | |  |  | |
| Volume (veh/h) | 2 | 0 | 9 | 8 | 1 | 3 | 9 | 542 | 3 | 1 | 488 | 3 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 2 | 0 | 9 | 8 | 1 | 3 | 9 | 571 | 3 | 1 | 514 | 3 |
| Pedestrians | | 1 | | | | | | 1 | | | | |
| Lane Width (ft) | | 12.0 | | | | | | 12.0 | | | | |
| Walking Speed (ft/s) | | 4.0 | | | | | | 4.0 | | | | |
| Percent Blockage | | 0 | | | | | | 0 | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1112 | 1111 | 517 | 1117 | 1111 | 572 | 518 | | | 574 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1112 | 1111 | 517 | 1117 | 1111 | 572 | 518 | | | 574 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 99 | 100 | 98 | 95 | 99 | 99 | 99 | | | 100 | | |
| cM capacity (veh/h) | 183 | 207 | 557 | 180 | 207 | 520 | 1047 | | | 999 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 12 | 13 | 9 | 574 | 1 | 517 | | | | | | |
| Volume Left | 2 | 8 | 9 | 0 | 1 | 0 | | | | | | |
| Volume Right | 9 | 3 | 0 | 3 | 0 | 3 | | | | | | |
| cSH | 406 | 218 | 1047 | 1700 | 999 | 1700 | | | | | | |
| Volume to Capacity | 0.03 | 0.06 | 0.01 | 0.34 | 0.00 | 0.30 | | | | | | |
| Queue Length 95th (ft) | 2 | 5 | 1 | 0 | 0 | 0 | | | | | | |
| Control Delay (s) | 14.1 | 22.6 | 8.5 | 0.0 | 8.6 | 0.0 | | | | | | |
| Lane LOS | B | C | A | | A | | | | | | | |
| Approach Delay (s) | 14.1 | 22.6 | 0.1 | | 0.0 | | | | | | | |
| Approach LOS | B | C | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 0.5 | | | | | | | | | |
| Intersection Capacity Utilization | | | 39.0% | ICU Level of Service | A | | | | | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

8: SR-1 & Vermont Ave

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 13 | 3 | 13 | 29 | 1 | 5 | 1 | 534 | 20 | 2 | 495 | 8 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 14 | 3 | 14 | 31 | 1 | 5 | 1 | 562 | 21 | 2 | 521 | 8 |
| Pedestrians | | | | | 1 | | | 1 | | | | |
| Lane Width (ft) | | | | | 12.0 | | | 12.0 | | | | |
| Walking Speed (ft/s) | | | | | 4.0 | | | 4.0 | | | | |
| Percent Blockage | | | | | 0 | | | 0 | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage veh | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1099 | 1116 | 526 | 1117 | 1109 | 574 | 529 | | | 584 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1099 | 1116 | 526 | 1117 | 1109 | 574 | 529 | | | 584 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 93 | 98 | 98 | 83 | 99 | 99 | 100 | | | 100 | | |
| cM capacity (veh/h) | 187 | 207 | 551 | 177 | 209 | 518 | 1038 | | | 990 | | |




















| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 31 | 37 | 1 | 583 | 2 | 529 |
| Volume Left | 14 | 31 | 1 | 0 | 2 | 0 |
| Volume Right | 14 | 5 | 0 | 21 | 0 | 8 |
| cSH | 269 | 196 | 1038 | 1700 | 990 | 1700 |
| Volume to Capacity | 0.11 | 0.19 | 0.00 | 0.34 | 0.00 | 0.31 |
| Queue Length 95th (ft) | 9 | 17 | 0 | 0 | 0 | 0 |
| Control Delay (s) | 20.1 | 27.5 | 8.5 | 0.0 | 8.6 | 0.0 |
| Lane LOS | C | D | A | | A | |
| Approach Delay (s) | 20.1 | 27.5 | 0.0 | | 0.0 | |
| Approach LOS | C | D | | | | |

| Intersection Summary | | |
|-----------------------------------|-------|----------------------|
| Average Delay | | 1.4 |
| Intersection Capacity Utilization | 39.6% | ICU Level of Service |
| Analysis Period (min) | | 15 |
| | | A |

HCM Unsignalized Intersection Capacity Analysis

9: SR-1 & Cypress Ave

9/18/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | |  | |  |  | |  |  |  |
| Volume (veh/h) | 61 | 1 | 15 | 13 | 3 | 6 | 22 | 511 | 5 | 5 | 530 | 46 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 66 | 1 | 16 | 14 | 3 | 7 | 24 | 555 | 5 | 5 | 576 | 50 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1223 | 1221 | 601 | 1210 | 1243 | 558 | 626 | | | 561 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1223 | 1221 | 601 | 1210 | 1243 | 558 | 626 | | | 561 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 55 | 99 | 97 | 91 | 98 | 99 | 97 | | | 99 | | |
| cM capacity (veh/h) | 148 | 174 | 500 | 150 | 169 | 529 | 956 | | | 1010 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 84 | 24 | 24 | 561 | 5 | 626 | | | | | | |
| Volume Left | 66 | 14 | 24 | 0 | 5 | 0 | | | | | | |
| Volume Right | 16 | 7 | 0 | 5 | 0 | 50 | | | | | | |
| cSH | 172 | 190 | 956 | 1700 | 1010 | 1700 | | | | | | |
| Volume to Capacity | 0.49 | 0.13 | 0.03 | 0.33 | 0.01 | 0.37 | | | | | | |
| Queue Length 95th (ft) | 58 | 11 | 2 | 0 | 0 | 0 | | | | | | |
| Control Delay (s) | 44.2 | 26.7 | 8.9 | 0.0 | 8.6 | 0.0 | | | | | | |
| Lane LOS | E | D | A | | A | | | | | | | |
| Approach Delay (s) | 44.2 | 26.7 | 0.4 | | 0.1 | | | | | | | |
| Approach LOS | E | D | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 3.5 | | | | | | | | | |
| Intersection Capacity Utilization | | | 43.3% | ICU Level of Service | A | | | | | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

10: SR-1 & St Etheldore St

9/18/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 11 | 0 | 521 | 14 | 1 | 544 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Hourly flow rate (vph) | 12 | 0 | 573 | 15 | 1 | 598 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1180 | 580 | | | 588 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1180 | 580 | | | 588 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 94 | 100 | | | 100 | |
| cM capacity (veh/h) | 210 | 514 | | | 987 | |

| Direction, Lane # | WB 1 | NB 1 | SB 1 |
|------------------------|------|------|------|
| Volume Total | 12 | 588 | 599 |
| Volume Left | 12 | 0 | 1 |
| Volume Right | 0 | 15 | 0 |
| cSH | 210 | 1700 | 987 |
| Volume to Capacity | 0.06 | 0.35 | 0.00 |
| Queue Length 95th (ft) | 5 | 0 | 0 |
| Control Delay (s) | 23.2 | 0.0 | 0.0 |
| Lane LOS | C | | A |
| Approach Delay (s) | 23.2 | 0.0 | 0.0 |
| Approach LOS | C | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|----------------------|
| Average Delay | | 0.2 | |
| Intersection Capacity Utilization | | 39.4% | ICU Level of Service |
| Analysis Period (min) | | 15 | A |

HCM Unsignalized Intersection Capacity Analysis

11: SR-1 & Capistrano Rd

9/18/2014



| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 16 | 2 | 6 | 518 | 519 | 32 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 17 | 2 | 7 | 563 | 564 | 35 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 859 | 564 | 564 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 859 | 564 | 564 | | | |
| tC, single (s) | 6.8 | 6.9 | 4.1 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | |
| p0 queue free % | 94 | 100 | 99 | | | |
| cM capacity (veh/h) | 294 | 469 | 1004 | | | |

| Direction, Lane # | EB 1 | EB 2 | NB 1 | NB 2 | NB 3 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|------|
| Volume Total | 17 | 2 | 7 | 282 | 282 | 564 | 35 |
| Volume Left | 17 | 0 | 7 | 0 | 0 | 0 | 0 |
| Volume Right | 0 | 2 | 0 | 0 | 0 | 0 | 35 |
| cSH | 294 | 469 | 1004 | 1700 | 1700 | 1700 | 1700 |
| Volume to Capacity | 0.06 | 0.00 | 0.01 | 0.17 | 0.17 | 0.33 | 0.02 |
| Queue Length 95th (ft) | 5 | 0 | 0 | 0 | 0 | 0 | 0 |
| Control Delay (s) | 18.0 | 12.7 | 8.6 | 0.0 | 0.0 | 0.0 | 0.0 |
| Lane LOS | C | B | A | | | | |
| Approach Delay (s) | 17.4 | | 0.1 | | | 0.0 | |
| Approach LOS | C | | | | | | |

| Intersection Summary | | | |
|-----------------------------------|-------|----------------------|---|
| Average Delay | | 0.3 | |
| Intersection Capacity Utilization | 37.3% | ICU Level of Service | A |
| Analysis Period (min) | 15 | | |

HCM Unsignalized Intersection Capacity Analysis

12: Coral Reef Ave & SR-1

9/18/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 35 | 51 | 445 | 20 | 18 | 505 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 38 | 55 | 478 | 22 | 19 | 543 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1071 | 489 | | | 500 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1071 | 489 | | | 500 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 84 | 91 | | | 98 | |
| cM capacity (veh/h) | 240 | 579 | | | 1064 | |

| Direction, Lane # | WB 1 | WB 2 | NB 1 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|
| Volume Total | 38 | 55 | 500 | 19 | 543 |
| Volume Left | 38 | 0 | 0 | 19 | 0 |
| Volume Right | 0 | 55 | 22 | 0 | 0 |
| cSH | 240 | 579 | 1700 | 1064 | 1700 |
| Volume to Capacity | 0.16 | 0.09 | 0.29 | 0.02 | 0.32 |
| Queue Length 95th (ft) | 14 | 8 | 0 | 1 | 0 |
| Control Delay (s) | 22.8 | 11.9 | 0.0 | 8.4 | 0.0 |
| Lane LOS | C | B | | A | |
| Approach Delay (s) | 16.3 | | 0.0 | 0.3 | |
| Approach LOS | C | | | | |

| Intersection Summary | | | | | |
|-----------------------------------|--|--|-------|----------------------|---|
| Average Delay | | | 1.4 | | |
| Intersection Capacity Utilization | | | 36.6% | ICU Level of Service | A |
| Analysis Period (min) | | | 15 | | |

HCM Signalized Intersection Capacity Analysis

13: Capistrano Rd

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|------|------|------|------|-------|------|------|------|------|-------|-------|------|
| Lane Configurations | | ↕ | ↗ | | ↕ | | ↗ | ↕ | | ↗ | ↕ | ↗ |
| Volume (vph) | 8 | 97 | 96 | 79 | 67 | 158 | 96 | 353 | 41 | 87 | 400 | 23 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 3.5 | 3.5 | | 3.5 | | 3.0 | 6.5 | | 3.0 | 6.5 | 6.5 |
| Lane Util. Factor | | 1.00 | 1.00 | | 1.00 | | 0.97 | 0.95 | | 1.00 | 0.95 | 1.00 |
| Fr _t | | 1.00 | 0.85 | | 0.93 | | 1.00 | 0.98 | | 1.00 | 1.00 | 0.85 |
| Fl _t Protected | | 1.00 | 1.00 | | 0.99 | | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | | 1855 | 1583 | | 1710 | | 3433 | 3484 | | 1770 | 3539 | 1583 |
| Fl _t Permitted | | 0.98 | 1.00 | | 0.90 | | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | | 1817 | 1583 | | 1560 | | 3433 | 3484 | | 1770 | 3539 | 1583 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 9 | 105 | 104 | 86 | 73 | 172 | 104 | 384 | 45 | 95 | 435 | 25 |
| RTOR Reduction (vph) | 0 | 0 | 55 | 0 | 21 | 0 | 0 | 11 | 0 | 0 | 0 | 19 |
| Lane Group Flow (vph) | 0 | 114 | 49 | 0 | 310 | 0 | 104 | 418 | 0 | 95 | 435 | 6 |
| Turn Type | Perm | NA | Perm | | NA | | Prot | NA | | Prot | NA | Perm |
| Protected Phases | | 8 | | | 4 | | 1 | 6 | | 5 | 2 | |
| Permitted Phases | 8 | | 8 | | | | | | | | | 2 |
| Actuated Green, G (s) | | 30.5 | 30.5 | | 30.5 | | 5.3 | 14.2 | | 6.7 | 15.6 | 15.6 |
| Effective Green, g (s) | | 30.5 | 30.5 | | 30.5 | | 5.3 | 14.2 | | 6.7 | 15.6 | 15.6 |
| Actuated g/C Ratio | | 0.47 | 0.47 | | 0.47 | | 0.08 | 0.22 | | 0.10 | 0.24 | 0.24 |
| Clearance Time (s) | | 3.5 | 3.5 | | 3.5 | | 3.0 | 6.5 | | 3.0 | 6.5 | 6.5 |
| Vehicle Extension (s) | | 2.0 | 2.0 | | 2.0 | | 2.0 | 4.0 | | 2.0 | 4.0 | 4.0 |
| Lane Grp Cap (vph) | | 860 | 749 | | 738 | | 282 | 768 | | 184 | 857 | 383 |
| v/s Ratio Prot | | | | | | | 0.03 | 0.12 | | c0.05 | c0.12 | |
| v/s Ratio Perm | | 0.06 | 0.03 | | c0.20 | | | | | | | 0.00 |
| v/c Ratio | | 0.13 | 0.07 | | 0.42 | | 0.37 | 0.54 | | 0.52 | 0.51 | 0.02 |
| Uniform Delay, d ₁ | | 9.5 | 9.2 | | 11.1 | | 28.0 | 22.2 | | 27.3 | 21.1 | 18.6 |
| Progression Factor | | 1.00 | 1.00 | | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d ₂ | | 0.0 | 0.0 | | 0.1 | | 0.3 | 1.0 | | 1.0 | 0.6 | 0.0 |
| Delay (s) | | 9.5 | 9.2 | | 11.3 | | 28.3 | 23.2 | | 28.3 | 21.7 | 18.6 |
| Level of Service | | A | A | | B | | C | C | | C | C | B |
| Approach Delay (s) | | 9.4 | | | 11.3 | | | 24.2 | | | 22.7 | |
| Approach LOS | | A | | | B | | | C | | | C | |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 19.1 | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | 0.46 | | |
| Actuated Cycle Length (s) | 64.4 | Sum of lost time (s) | 13.0 |
| Intersection Capacity Utilization | 52.4% | ICU Level of Service | A |
| Analysis Period (min) | 15 | | |
| c Critical Lane Group | | | |

HCM Signalized Intersection Capacity Analysis

14: SR-1 & Coronado St

9/18/2014



| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
|------------------------|------|-------|------|------|-------|------|
| Lane Configurations | | | | | | |
| Volume (vph) | 43 | 655 | 483 | 196 | 431 | 20 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 3.0 | 5.5 | 5.5 | 5.5 | 3.0 | 3.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.98 |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 1.00 | 1.00 | 0.85 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (prot) | 1770 | 1863 | 1863 | 1583 | 1770 | 1545 |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (perm) | 1770 | 1863 | 1863 | 1583 | 1770 | 1545 |
| Peak-hour factor, PHF | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Adj. Flow (vph) | 47 | 720 | 531 | 215 | 474 | 22 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 134 | 0 | 13 |
| Lane Group Flow (vph) | 47 | 720 | 531 | 81 | 474 | 9 |
| Confl. Peds. (#/hr) | | | | | | 3 |
| Turn Type | Prot | NA | NA | Perm | NA | Perm |
| Protected Phases | 5 | 2 | 6 | | 4 | |
| Permitted Phases | | | | 6 | | 4 |
| Actuated Green, G (s) | 4.6 | 35.7 | 28.1 | 28.1 | 30.6 | 30.6 |
| Effective Green, g (s) | 4.6 | 35.7 | 28.1 | 28.1 | 30.6 | 30.6 |
| Actuated g/C Ratio | 0.06 | 0.48 | 0.38 | 0.38 | 0.41 | 0.41 |
| Clearance Time (s) | 3.0 | 5.5 | 5.5 | 5.5 | 3.0 | 3.0 |
| Vehicle Extension (s) | 2.5 | 2.4 | 2.4 | 2.4 | 2.0 | 2.0 |
| Lane Grp Cap (vph) | 108 | 889 | 699 | 594 | 724 | 632 |
| v/s Ratio Prot | 0.03 | c0.39 | 0.29 | | c0.27 | |
| v/s Ratio Perm | | | | 0.05 | | 0.01 |
| v/c Ratio | 0.44 | 0.81 | 0.76 | 0.14 | 0.65 | 0.01 |
| Uniform Delay, d1 | 33.8 | 16.7 | 20.4 | 15.4 | 17.8 | 13.1 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 2.0 | 5.3 | 4.5 | 0.1 | 1.6 | 0.0 |
| Delay (s) | 35.9 | 22.0 | 24.9 | 15.4 | 19.5 | 13.1 |
| Level of Service | D | C | C | B | B | B |
| Approach Delay (s) | | 22.8 | 22.2 | | 19.2 | |
| Approach LOS | | C | C | | B | |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 21.7 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.77 | | |
| Actuated Cycle Length (s) | 74.8 | Sum of lost time (s) | 11.5 |
| Intersection Capacity Utilization | 66.3% | ICU Level of Service | C |
| Analysis Period (min) | 15 | | |

c Critical Lane Group

HCM Unsignalized Intersection Capacity Analysis

15: Coronado St & Obispo Rd

9/18/2014



| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | | ↔ | |
| Volume (veh/h) | 27 | 213 | 345 | 0 | 3 | 101 |
| Sign Control | | Free | Free | | Stop | |
| Grade | | 0% | 0% | | 0% | |
| Peak Hour Factor | 0.76 | 0.76 | 0.76 | 0.76 | 0.76 | 0.76 |
| Hourly flow rate (vph) | 36 | 280 | 454 | 0 | 4 | 133 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | None | None | | | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | 151 | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 454 | | | | 805 | 454 |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 454 | | | | 805 | 454 |
| tC, single (s) | 4.1 | | | | 6.4 | 6.2 |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 2.2 | | | | 3.5 | 3.3 |
| p0 queue free % | 97 | | | | 99 | 78 |
| cM capacity (veh/h) | 1107 | | | | 340 | 606 |

| Direction, Lane # | EB 1 | WB 1 | SB 1 |
|------------------------|------|------|------|
| Volume Total | 316 | 454 | 137 |
| Volume Left | 36 | 0 | 4 |
| Volume Right | 0 | 0 | 133 |
| cSH | 1107 | 1700 | 593 |
| Volume to Capacity | 0.03 | 0.27 | 0.23 |
| Queue Length 95th (ft) | 2 | 0 | 22 |
| Control Delay (s) | 1.2 | 0.0 | 12.9 |
| Lane LOS | A | | B |
| Approach Delay (s) | 1.2 | 0.0 | 12.9 |
| Approach LOS | | | B |

| Intersection Summary | | | |
|-----------------------------------|-------|-----|------------------------|
| Average Delay | | 2.4 | |
| Intersection Capacity Utilization | 46.9% | | ICU Level of Service A |
| Analysis Period (min) | 15 | | |

HCM Unsignalized Intersection Capacity Analysis

16: SR-1 & Magellan Ave

9/18/2014






















| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|-----------------------------------|------|------|-------|------|----------------------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 7 | 2 | 9 | 716 | 1000 | 20 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Hourly flow rate (vph) | 8 | 2 | 10 | 796 | 1111 | 22 |
| Pedestrians | 1 | | | | | |
| Lane Width (ft) | 12.0 | | | | | |
| Walking Speed (ft/s) | 4.0 | | | | | |
| Percent Blockage | 0 | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1939 | 1123 | 1134 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1939 | 1123 | 1134 | | | |
| tC, single (s) | 6.4 | 6.2 | 4.1 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | |
| p0 queue free % | 89 | 99 | 98 | | | |
| cM capacity (veh/h) | 71 | 250 | 615 | | | |
| Direction, Lane # | EB 1 | NB 1 | NB 2 | SB 1 | | |
| Volume Total | 10 | 10 | 796 | 1133 | | |
| Volume Left | 8 | 10 | 0 | 0 | | |
| Volume Right | 2 | 0 | 0 | 22 | | |
| cSH | 84 | 615 | 1700 | 1700 | | |
| Volume to Capacity | 0.12 | 0.02 | 0.47 | 0.67 | | |
| Queue Length 95th (ft) | 10 | 1 | 0 | 0 | | |
| Control Delay (s) | 53.5 | 10.9 | 0.0 | 0.0 | | |
| Lane LOS | F | B | | | | |
| Approach Delay (s) | 53.5 | 0.1 | | 0.0 | | |
| Approach LOS | F | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 0.3 | | | |
| Intersection Capacity Utilization | | | 63.8% | | ICU Level of Service | B |
| Analysis Period (min) | | | 15 | | | |

HCM Unsignalized Intersection Capacity Analysis

17: SR-1 & Medio Ave




















9/18/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | |  | |  |  | |  |  |  |
| Volume (veh/h) | 3 | 0 | 28 | 17 | 0 | 9 | 9 | 656 | 6 | 6 | 1039 | 4 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Hourly flow rate (vph) | 3 | 0 | 31 | 19 | 0 | 10 | 10 | 721 | 7 | 7 | 1142 | 4 |
| Pedestrians | | | | | | | | | | | | 1 |
| Lane Width (ft) | | | | | | | | | | | | 12.0 |
| Walking Speed (ft/s) | | | | | | | | | | | | 4.0 |
| Percent Blockage | | | | | | | | | | | | 0 |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | | None |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1909 | 1904 | 1144 | 1930 | 1903 | 725 | 1146 | | | 727 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1909 | 1904 | 1144 | 1930 | 1903 | 725 | 1146 | | | 727 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 93 | 100 | 87 | 56 | 100 | 98 | 98 | | | 99 | | |
| cM capacity (veh/h) | 50 | 67 | 243 | 43 | 67 | 425 | 610 | | | 876 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 34 | 29 | 10 | 727 | 7 | 1146 | | | | | | |
| Volume Left | 3 | 19 | 10 | 0 | 7 | 0 | | | | | | |
| Volume Right | 31 | 10 | 0 | 7 | 0 | 4 | | | | | | |
| cSH | 177 | 62 | 610 | 1700 | 876 | 1700 | | | | | | |
| Volume to Capacity | 0.19 | 0.46 | 0.02 | 0.43 | 0.01 | 0.67 | | | | | | |
| Queue Length 95th (ft) | 17 | 45 | 1 | 0 | 1 | 0 | | | | | | |
| Control Delay (s) | 30.2 | 104.5 | 11.0 | 0.0 | 9.1 | 0.0 | | | | | | |
| Lane LOS | D | F | B | | A | | | | | | | |
| Approach Delay (s) | 30.2 | 104.5 | 0.1 | | 0.1 | | | | | | | |
| Approach LOS | D | F | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 2.1 | | | | | | | | | |
| Intersection Capacity Utilization | | | 68.1% | ICU Level of Service | | C | | | | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

18: SR-1 & Miramar Dr



















9/18/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | |  | |  |  | |  |  |  |
| Volume (veh/h) | 0 | 0 | 5 | 0 | 0 | 9 | 1 | 677 | 0 | 2 | 1085 | 6 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Hourly flow rate (vph) | 0 | 0 | 5 | 0 | 0 | 10 | 1 | 744 | 0 | 2 | 1192 | 7 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1956 | 1946 | 1196 | 1948 | 1949 | 744 | 1199 | | | 744 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1956 | 1946 | 1196 | 1948 | 1949 | 744 | 1199 | | | 744 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 98 | 100 | 100 | 98 | 100 | | | 100 | | |
| cM capacity (veh/h) | 47 | 64 | 227 | 47 | 64 | 415 | 582 | | | 864 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 5 | 10 | 1 | 744 | 2 | 1199 | | | | | | |
| Volume Left | 0 | 0 | 1 | 0 | 2 | 0 | | | | | | |
| Volume Right | 5 | 10 | 0 | 0 | 0 | 7 | | | | | | |
| cSH | 227 | 415 | 582 | 1700 | 864 | 1700 | | | | | | |
| Volume to Capacity | 0.02 | 0.02 | 0.00 | 0.44 | 0.00 | 0.71 | | | | | | |
| Queue Length 95th (ft) | 2 | 2 | 0 | 0 | 0 | 0 | | | | | | |
| Control Delay (s) | 21.3 | 13.9 | 11.2 | 0.0 | 9.2 | 0.0 | | | | | | |
| Lane LOS | C | B | B | | A | | | | | | | |
| Approach Delay (s) | 21.3 | 13.9 | 0.0 | | 0.0 | | | | | | | |
| Approach LOS | C | B | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 0.1 | | | | | | | | | |
| Intersection Capacity Utilization | | | 67.5% | ICU Level of Service | | C | | | | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

1: SR-1 & 2nd St

9/18/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | |  | |  |  | |  |  | |
| Volume (veh/h) | 0 | 0 | 0 | 12 | 0 | 75 | 0 | 549 | 8 | 54 | 819 | 0 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 13 | 0 | 80 | 0 | 584 | 9 | 57 | 871 | 0 |
| Pedestrians | | | | | | | | | | | | 9 |
| Lane Width (ft) | | | | | | | | | | | | 12.0 |
| Walking Speed (ft/s) | | | | | | | | | | | | 4.0 |
| Percent Blockage | | | | | | | | | | | | 1 |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | | None |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1659 | 1579 | 871 | 1574 | 1574 | 597 | 871 | | | 593 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1659 | 1579 | 871 | 1574 | 1574 | 597 | 871 | | | 593 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 100 | 85 | 100 | 84 | 100 | | | 94 | | |
| cM capacity (veh/h) | 62 | 103 | 350 | 85 | 103 | 499 | 774 | | | 983 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 0 | 93 | 0 | 593 | 57 | 871 | | | | | | |
| Volume Left | 0 | 13 | 0 | 0 | 57 | 0 | | | | | | |
| Volume Right | 0 | 80 | 0 | 9 | 0 | 0 | | | | | | |
| cSH | 1700 | 299 | 1700 | 1700 | 983 | 1700 | | | | | | |
| Volume to Capacity | 0.00 | 0.31 | 0.00 | 0.35 | 0.06 | 0.51 | | | | | | |
| Queue Length 95th (ft) | 0 | 32 | 0 | 0 | 5 | 0 | | | | | | |
| Control Delay (s) | 0.0 | 22.4 | 0.0 | 0.0 | 8.9 | 0.0 | | | | | | |
| Lane LOS | A | C | | | A | | | | | | | |
| Approach Delay (s) | 0.0 | 22.4 | 0.0 | | 0.5 | | | | | | | |
| Approach LOS | A | C | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | | 1.6 | | | | | | | | |
| Intersection Capacity Utilization | | | 59.5% | | ICU Level of Service | | | | B | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

2: SR-1 & 7th St

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------------|-------------|-------------|----------------------|-------------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | | ↔ | ↔ | ↔ | | | ↔ | |
| Volume (veh/h) | 0 | 0 | 1 | 0 | 0 | 11 | 0 | 601 | 9 | 10 | 803 | 1 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Hourly flow rate (vph) | 0 | 0 | 1 | 0 | 0 | 11 | 0 | 620 | 9 | 10 | 828 | 1 |
| Pedestrians | | 1 | | | 4 | | | | | | | |
| Lane Width (ft) | | 12.0 | | | 12.0 | | | | | | | |
| Walking Speed (ft/s) | | 4.0 | | | 4.0 | | | | | | | |
| Percent Blockage | | 0 | | | 0 | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1481 | 1483 | 829 | 1478 | 1479 | 628 | 830 | | | 633 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1481 | 1483 | 829 | 1478 | 1479 | 628 | 830 | | | 633 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 100 | 100 | 100 | 98 | 100 | | | 99 | | |
| cM capacity (veh/h) | 100 | 123 | 370 | 102 | 124 | 481 | 801 | | | 947 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | | | | | | | |
| Volume Total | 1 | 11 | 0 | 629 | 839 | | | | | | | |
| Volume Left | 0 | 0 | 0 | 0 | 10 | | | | | | | |
| Volume Right | 1 | 11 | 0 | 9 | 1 | | | | | | | |
| cSH | 370 | 481 | 1700 | 1700 | 947 | | | | | | | |
| Volume to Capacity | 0.00 | 0.02 | 0.00 | 0.37 | 0.01 | | | | | | | |
| Queue Length 95th (ft) | 0 | 2 | 0 | 0 | 1 | | | | | | | |
| Control Delay (s) | 14.8 | 12.7 | 0.0 | 0.0 | 0.3 | | | | | | | |
| Lane LOS | B | B | | | A | | | | | | | |
| Approach Delay (s) | 14.8 | 12.7 | 0.0 | | 0.3 | | | | | | | |
| Approach LOS | B | B | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 0.3 | | | | | | | | | |
| Intersection Capacity Utilization | | | 60.3% | ICU Level of Service | | B | | | | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

3: SR-1 & 8th St

9/18/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 59 | 16 | 588 | 13 | 16 | 789 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Hourly flow rate (vph) | 61 | 16 | 606 | 13 | 16 | 813 |
| Pedestrians | 4 | | | | | 1 |
| Lane Width (ft) | 12.0 | | | | | 12.0 |
| Walking Speed (ft/s) | 4.0 | | | | | 4.0 |
| Percent Blockage | 0 | | | | | 0 |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage veh | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1463 | 618 | | | 624 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1463 | 618 | | | 624 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 56 | 97 | | | 98 | |
| cM capacity (veh/h) | 139 | 487 | | | 954 | |

| Direction, Lane # | WB 1 | NB 1 | SB 1 |
|------------------------|------|------|------|
| Volume Total | 77 | 620 | 830 |
| Volume Left | 61 | 0 | 16 |
| Volume Right | 16 | 13 | 0 |
| cSH | 164 | 1700 | 954 |
| Volume to Capacity | 0.47 | 0.36 | 0.02 |
| Queue Length 95th (ft) | 56 | 0 | 1 |
| Control Delay (s) | 45.3 | 0.0 | 0.5 |
| Lane LOS | E | | A |
| Approach Delay (s) | 45.3 | 0.0 | 0.5 |
| Approach LOS | E | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|------------------------|
| Average Delay | | 2.5 | |
| Intersection Capacity Utilization | | 65.6% | ICU Level of Service C |
| Analysis Period (min) | | 15 | |

HCM Unsignalized Intersection Capacity Analysis

4: SR-1 & Carlos St

9/18/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 0 | 8 | 613 | 0 | 7 | 918 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.98 | 0.98 | 0.96 | 0.96 | 0.98 | 0.98 |
| Hourly flow rate (vph) | 0 | 8 | 639 | 0 | 7 | 937 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1590 | 639 | | | 639 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1590 | 639 | | | 639 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 100 | 98 | | | 99 | |
| cM capacity (veh/h) | 118 | 476 | | | 945 | |

| Direction, Lane # | WB 1 | NB 1 | SB 1 | SB 2 |
|------------------------|------|------|------|------|
| Volume Total | 8 | 639 | 7 | 937 |
| Volume Left | 0 | 0 | 7 | 0 |
| Volume Right | 8 | 0 | 0 | 0 |
| cSH | 476 | 1700 | 945 | 1700 |
| Volume to Capacity | 0.02 | 0.38 | 0.01 | 0.55 |
| Queue Length 95th (ft) | 1 | 0 | 1 | 0 |
| Control Delay (s) | 12.7 | 0.0 | 8.8 | 0.0 |
| Lane LOS | B | | A | |
| Approach Delay (s) | 12.7 | 0.0 | 0.1 | |
| Approach LOS | B | | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|------------------------|
| Average Delay | | 0.1 | |
| Intersection Capacity Utilization | | 58.3% | ICU Level of Service B |
| Analysis Period (min) | | 15 | |

HCM Unsignalized Intersection Capacity Analysis

5: SR-1 & Vallemar St

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 0 | 0 | 4 | 7 | 0 | 26 | 6 | 591 | 5 | 24 | 881 | 5 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 |
| Hourly flow rate (vph) | 0 | 0 | 4 | 7 | 0 | 27 | 6 | 603 | 5 | 24 | 899 | 5 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1592 | 1571 | 902 | 1570 | 1571 | 606 | 904 | | | 608 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1592 | 1571 | 902 | 1570 | 1571 | 606 | 904 | | | 608 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 99 | 92 | 100 | 95 | 99 | | | 97 | | |
| cM capacity (veh/h) | 80 | 107 | 336 | 86 | 107 | 497 | 752 | | | 970 | | |



















| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 4 | 34 | 6 | 608 | 24 | 904 |
| Volume Left | 0 | 7 | 6 | 0 | 24 | 0 |
| Volume Right | 4 | 27 | 0 | 5 | 0 | 5 |
| cSH | 336 | 248 | 752 | 1700 | 970 | 1700 |
| Volume to Capacity | 0.01 | 0.14 | 0.01 | 0.36 | 0.03 | 0.53 |
| Queue Length 95th (ft) | 1 | 12 | 1 | 0 | 2 | 0 |
| Control Delay (s) | 15.8 | 21.8 | 9.8 | 0.0 | 8.8 | 0.0 |
| Lane LOS | C | C | A | | A | |
| Approach Delay (s) | 15.8 | 21.8 | 0.1 | | 0.2 | |
| Approach LOS | C | C | | | | |

Intersection Summary

| | |
|-----------------------------------|-------|
| Average Delay | 0.7 |
| Intersection Capacity Utilization | 61.8% |
| ICU Level of Service | B |
| Analysis Period (min) | 15 |

HCM Unsignalized Intersection Capacity Analysis
6: SR-1 & California Ave

9/18/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | |  | |  |  | |  |  | |
| Volume (veh/h) | 5 | 0 | 22 | 26 | 1 | 20 | 17 | 582 | 39 | 18 | 878 | 13 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 |
| Hourly flow rate (vph) | 5 | 0 | 22 | 27 | 1 | 20 | 17 | 594 | 40 | 18 | 896 | 13 |
| Pedestrians | | 1 | | | 2 | | | | | | | |
| Lane Width (ft) | | 12.0 | | | 12.0 | | | | | | | |
| Walking Speed (ft/s) | | 4.0 | | | 4.0 | | | | | | | |
| Percent Blockage | | 0 | | | 0 | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage veh | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1590 | 1611 | 904 | 1606 | 1597 | 616 | 910 | | | 636 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1590 | 1611 | 904 | 1606 | 1597 | 616 | 910 | | | 636 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 94 | 100 | 93 | 65 | 99 | 96 | 98 | | | 98 | | |
| cM capacity (veh/h) | 80 | 100 | 335 | 76 | 102 | 490 | 748 | | | 946 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 28 | 48 | 17 | 634 | 18 | 909 | | | | | | |
| Volume Left | 5 | 27 | 17 | 0 | 18 | 0 | | | | | | |
| Volume Right | 22 | 20 | 0 | 40 | 0 | 13 | | | | | | |
| cSH | 210 | 120 | 748 | 1700 | 946 | 1700 | | | | | | |
| Volume to Capacity | 0.13 | 0.40 | 0.02 | 0.37 | 0.02 | 0.53 | | | | | | |
| Queue Length 95th (ft) | 11 | 42 | 2 | 0 | 1 | 0 | | | | | | |
| Control Delay (s) | 24.7 | 53.7 | 9.9 | 0.0 | 8.9 | 0.0 | | | | | | |
| Lane LOS | C | F | A | | A | | | | | | | |
| Approach Delay (s) | 24.7 | 53.7 | 0.3 | | 0.2 | | | | | | | |
| Approach LOS | C | F | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 2.2 | | | | | | | | | |
| Intersection Capacity Utilization | | | 60.3% | ICU Level of Service | | B | | | | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

7: SR-1 & Virginia Ave

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 8 | 0 | 29 | 16 | 2 | 9 | 13 | 590 | 17 | 11 | 947 | 8 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Hourly flow rate (vph) | 9 | 0 | 31 | 16 | 2 | 9 | 13 | 608 | 18 | 11 | 976 | 8 |
| Pedestrians | | 3 | | | | | | | | | | 1 |
| Lane Width (ft) | | 12.0 | | | | | | | | | | 12.0 |
| Walking Speed (ft/s) | | 4.0 | | | | | | | | | | 4.0 |
| Percent Blockage | | 0 | | | | | | | | | | 0 |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | | None |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1652 | 1659 | 983 | 1674 | 1654 | 618 | 988 | | | 626 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1652 | 1659 | 983 | 1674 | 1654 | 618 | 988 | | | 626 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 88 | 100 | 90 | 75 | 98 | 98 | 98 | | | 99 | | |
| cM capacity (veh/h) | 74 | 94 | 301 | 66 | 95 | 489 | 698 | | | 956 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 40 | 28 | 13 | 626 | 11 | 985 |
| Volume Left | 9 | 16 | 13 | 0 | 11 | 0 |
| Volume Right | 31 | 9 | 0 | 18 | 0 | 8 |
| cSH | 180 | 96 | 698 | 1700 | 956 | 1700 |
| Volume to Capacity | 0.22 | 0.29 | 0.02 | 0.37 | 0.01 | 0.58 |
| Queue Length 95th (ft) | 20 | 27 | 1 | 0 | 1 | 0 |
| Control Delay (s) | 30.5 | 57.1 | 10.3 | 0.0 | 8.8 | 0.0 |
| Lane LOS | D | F | B | | A | |
| Approach Delay (s) | 30.5 | 57.1 | 0.2 | | 0.1 | |
| Approach LOS | D | F | | | | |

Intersection Summary

| | | | | | | |
|-----------------------------------|--|-------|--|----------------------|--|---|
| Average Delay | | 1.8 | | | | |
| Intersection Capacity Utilization | | 60.9% | | ICU Level of Service | | B |
| Analysis Period (min) | | 15 | | | | |

HCM Unsignalized Intersection Capacity Analysis

8: SR-1 & Vermont Ave

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 10 | 3 | 5 | 10 | 3 | 5 | 9 | 613 | 30 | 11 | 952 | 12 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 |
| Hourly flow rate (vph) | 10 | 3 | 5 | 10 | 3 | 5 | 9 | 626 | 31 | 11 | 971 | 12 |
| Pedestrians | | | | | 4 | | | 4 | | | 1 | |
| Lane Width (ft) | | | | | 12.0 | | | 12.0 | | | 12.0 | |
| Walking Speed (ft/s) | | | | | 4.0 | | | 4.0 | | | 4.0 | |
| Percent Blockage | | | | | 0 | | | 0 | | | 0 | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage veh | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1652 | 1678 | 982 | 1668 | 1669 | 646 | 984 | | | 660 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1652 | 1678 | 982 | 1668 | 1669 | 646 | 984 | | | 660 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 86 | 97 | 98 | 86 | 97 | 99 | 99 | | | 99 | | |
| cM capacity (veh/h) | 74 | 92 | 301 | 71 | 93 | 470 | 702 | | | 925 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 18 | 18 | 9 | 656 | 11 | 984 |
| Volume Left | 10 | 10 | 9 | 0 | 11 | 0 |
| Volume Right | 5 | 5 | 0 | 31 | 0 | 12 |
| cSH | 98 | 98 | 702 | 1700 | 925 | 1700 |
| Volume to Capacity | 0.19 | 0.19 | 0.01 | 0.39 | 0.01 | 0.58 |
| Queue Length 95th (ft) | 16 | 16 | 1 | 0 | 1 | 0 |
| Control Delay (s) | 50.1 | 49.8 | 10.2 | 0.0 | 8.9 | 0.0 |
| Lane LOS | F | E | B | | A | |
| Approach Delay (s) | 50.1 | 49.8 | 0.1 | | 0.1 | |
| Approach LOS | F | E | | | | |

Intersection Summary

| | |
|-----------------------------------|-------|
| Average Delay | 1.2 |
| Intersection Capacity Utilization | 62.1% |
| ICU Level of Service | B |
| Analysis Period (min) | 15 |

HCM Unsignalized Intersection Capacity Analysis

9: SR-1 & Cypress Ave

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 43 | 1 | 28 | 6 | 3 | 9 | 37 | 616 | 9 | 4 | 869 | 50 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 47 | 1 | 30 | 7 | 3 | 10 | 40 | 670 | 10 | 4 | 945 | 54 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1742 | 1740 | 972 | 1739 | 1762 | 674 | 999 | | | 679 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1742 | 1740 | 972 | 1739 | 1762 | 674 | 999 | | | 679 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 24 | 99 | 90 | 89 | 96 | 98 | 94 | | | 100 | | |
| cM capacity (veh/h) | 61 | 81 | 306 | 58 | 79 | 454 | 693 | | | 913 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|-------|------|------|------|------|------|
| Volume Total | 78 | 20 | 40 | 679 | 4 | 999 |
| Volume Left | 47 | 7 | 40 | 0 | 4 | 0 |
| Volume Right | 30 | 10 | 0 | 10 | 0 | 54 |
| cSH | 89 | 112 | 693 | 1700 | 913 | 1700 |
| Volume to Capacity | 0.87 | 0.18 | 0.06 | 0.40 | 0.00 | 0.59 |
| Queue Length 95th (ft) | 119 | 15 | 5 | 0 | 0 | 0 |
| Control Delay (s) | 146.0 | 44.0 | 10.5 | 0.0 | 9.0 | 0.0 |
| Lane LOS | F | E | B | | A | |
| Approach Delay (s) | 146.0 | 44.0 | 0.6 | | 0.0 | |
| Approach LOS | F | E | | | | |

Intersection Summary

| | |
|-----------------------------------|-------|
| Average Delay | 7.0 |
| Intersection Capacity Utilization | 62.7% |
| ICU Level of Service | B |
| Analysis Period (min) | 15 |

HCM Unsignalized Intersection Capacity Analysis

10: SR-1 & St Etheldore St

9/18/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 11 | 6 | 681 | 16 | 0 | 936 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 12 | 6 | 732 | 17 | 0 | 1006 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1747 | 741 | | | 749 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1747 | 741 | | | 749 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 87 | 98 | | | 100 | |
| cM capacity (veh/h) | 95 | 416 | | | 860 | |

| Direction, Lane # | WB 1 | NB 1 | SB 1 |
|------------------------|------|------|------|
| Volume Total | 18 | 749 | 1006 |
| Volume Left | 12 | 0 | 0 |
| Volume Right | 6 | 17 | 0 |
| cSH | 130 | 1700 | 860 |
| Volume to Capacity | 0.14 | 0.44 | 0.00 |
| Queue Length 95th (ft) | 12 | 0 | 0 |
| Control Delay (s) | 37.1 | 0.0 | 0.0 |
| Lane LOS | E | | |
| Approach Delay (s) | 37.1 | 0.0 | 0.0 |
| Approach LOS | E | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|----------------------|
| Average Delay | | 0.4 | |
| Intersection Capacity Utilization | | 59.3% | ICU Level of Service |
| Analysis Period (min) | | 15 | B |

HCM Unsignalized Intersection Capacity Analysis

11: SR-1 & Capistrano Rd

9/18/2014



| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 28 | 24 | 16 | 686 | 909 | 63 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Hourly flow rate (vph) | 30 | 26 | 17 | 730 | 967 | 67 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1366 | 967 | 967 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1366 | 967 | 967 | | | |
| tC, single (s) | 6.8 | 6.9 | 4.1 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | |
| p0 queue free % | 78 | 90 | 98 | | | |
| cM capacity (veh/h) | 135 | 254 | 708 | | | |

| Direction, Lane # | EB 1 | EB 2 | NB 1 | NB 2 | NB 3 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|------|
| Volume Total | 30 | 26 | 17 | 365 | 365 | 967 | 67 |
| Volume Left | 30 | 0 | 17 | 0 | 0 | 0 | 0 |
| Volume Right | 0 | 26 | 0 | 0 | 0 | 0 | 67 |
| cSH | 135 | 254 | 708 | 1700 | 1700 | 1700 | 1700 |
| Volume to Capacity | 0.22 | 0.10 | 0.02 | 0.21 | 0.21 | 0.57 | 0.04 |
| Queue Length 95th (ft) | 20 | 8 | 2 | 0 | 0 | 0 | 0 |
| Control Delay (s) | 39.1 | 20.7 | 10.2 | 0.0 | 0.0 | 0.0 | 0.0 |
| Lane LOS | E | C | B | | | | |
| Approach Delay (s) | 30.6 | | 0.2 | 0.0 | | | |
| Approach LOS | D | | | | | | |

| Intersection Summary | | | |
|-----------------------------------|-------|--|------------------------|
| Average Delay | | | 1.0 |
| Intersection Capacity Utilization | 57.8% | | ICU Level of Service B |
| Analysis Period (min) | 15 | | |

HCM Unsignalized Intersection Capacity Analysis
 12: Coral Reef Ave & SR-1

9/18/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 19 | 45 | 661 | 30 | 57 | 874 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 20 | 48 | 711 | 32 | 61 | 940 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage veh | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1789 | 727 | | | 743 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1789 | 727 | | | 743 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 75 | 89 | | | 93 | |
| cM capacity (veh/h) | 83 | 424 | | | 864 | |

| Direction, Lane # | WB 1 | WB 2 | NB 1 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|
| Volume Total | 20 | 48 | 743 | 61 | 940 |
| Volume Left | 20 | 0 | 0 | 61 | 0 |
| Volume Right | 0 | 48 | 32 | 0 | 0 |
| cSH | 83 | 424 | 1700 | 864 | 1700 |
| Volume to Capacity | 0.25 | 0.11 | 0.44 | 0.07 | 0.55 |
| Queue Length 95th (ft) | 22 | 10 | 0 | 6 | 0 |
| Control Delay (s) | 62.2 | 14.6 | 0.0 | 9.5 | 0.0 |
| Lane LOS | F | B | | A | |
| Approach Delay (s) | 28.7 | | 0.0 | 0.6 | |
| Approach LOS | D | | | | |

| Intersection Summary | | | | | |
|-----------------------------------|--|--|-------|----------------------|---|
| Average Delay | | | 1.4 | | |
| Intersection Capacity Utilization | | | 56.0% | ICU Level of Service | B |
| Analysis Period (min) | | | 15 | | |

HCM Signalized Intersection Capacity Analysis

13: Capistrano Rd

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|------|-------|------|------|------|------|------|-------|------|-------|------|------|
| Lane Configurations | | ↖ | ↗ | | ↔ | | ↖ | ↗ | | ↖ | ↗ | ↗ |
| Volume (vph) | 93 | 145 | 163 | 13 | 91 | 119 | 295 | 433 | 47 | 278 | 470 | 98 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 3.5 | 3.5 | | 3.5 | | 3.0 | 6.5 | | 3.0 | 6.5 | 6.5 |
| Lane Util. Factor | | 1.00 | 1.00 | | 1.00 | | 0.97 | 0.95 | | 1.00 | 0.95 | 1.00 |
| Fr _t | | 1.00 | 0.85 | | 0.93 | | 1.00 | 0.99 | | 1.00 | 1.00 | 0.85 |
| Fl _t Protected | | 0.98 | 1.00 | | 1.00 | | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | | 1827 | 1583 | | 1724 | | 3433 | 3487 | | 1770 | 3539 | 1583 |
| Fl _t Permitted | | 0.74 | 1.00 | | 0.98 | | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | | 1386 | 1583 | | 1691 | | 3433 | 3487 | | 1770 | 3539 | 1583 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 101 | 158 | 177 | 14 | 99 | 129 | 321 | 471 | 51 | 302 | 511 | 107 |
| RTOR Reduction (vph) | 0 | 0 | 97 | 0 | 43 | 0 | 0 | 10 | 0 | 0 | 0 | 70 |
| Lane Group Flow (vph) | 0 | 259 | 80 | 0 | 199 | 0 | 321 | 512 | 0 | 302 | 511 | 37 |
| Turn Type | Perm | NA | Perm | Perm | NA | | Prot | NA | | Prot | NA | Perm |
| Protected Phases | | 8 | | | 4 | | 1 | 6 | | 5 | 2 | |
| Permitted Phases | 8 | | 8 | 4 | | | | | | | | 2 |
| Actuated Green, G (s) | | 18.1 | 18.1 | | 18.1 | | 11.2 | 17.1 | | 16.3 | 22.2 | 22.2 |
| Effective Green, g (s) | | 18.1 | 18.1 | | 18.1 | | 11.2 | 17.1 | | 16.3 | 22.2 | 22.2 |
| Actuated g/C Ratio | | 0.28 | 0.28 | | 0.28 | | 0.17 | 0.27 | | 0.25 | 0.34 | 0.34 |
| Clearance Time (s) | | 3.5 | 3.5 | | 3.5 | | 3.0 | 6.5 | | 3.0 | 6.5 | 6.5 |
| Vehicle Extension (s) | | 2.0 | 2.0 | | 2.0 | | 2.0 | 4.0 | | 2.0 | 4.0 | 4.0 |
| Lane Grp Cap (vph) | | 388 | 444 | | 474 | | 596 | 924 | | 447 | 1218 | 544 |
| v/s Ratio Prot | | | | | | | 0.09 | c0.15 | | c0.17 | 0.14 | |
| v/s Ratio Perm | | c0.19 | 0.05 | | 0.12 | | | | | | | 0.02 |
| v/c Ratio | | 0.67 | 0.18 | | 0.42 | | 0.54 | 0.55 | | 0.68 | 0.42 | 0.07 |
| Uniform Delay, d ₁ | | 20.5 | 17.6 | | 18.9 | | 24.3 | 20.4 | | 21.7 | 16.2 | 14.2 |
| Progression Factor | | 1.00 | 1.00 | | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d ₂ | | 3.4 | 0.1 | | 0.2 | | 0.5 | 0.9 | | 3.2 | 0.3 | 0.1 |
| Delay (s) | | 23.9 | 17.6 | | 19.1 | | 24.8 | 21.3 | | 24.9 | 16.5 | 14.3 |
| Level of Service | | C | B | | B | | C | C | | C | B | B |
| Approach Delay (s) | | 21.4 | | | 19.1 | | | 22.6 | | | 19.0 | |
| Approach LOS | | C | | | B | | | C | | | B | |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 20.7 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.63 | | |
| Actuated Cycle Length (s) | 64.5 | Sum of lost time (s) | 13.0 |
| Intersection Capacity Utilization | 69.9% | ICU Level of Service | C |
| Analysis Period (min) | 15 | | |
| c Critical Lane Group | | | |

HCM Signalized Intersection Capacity Analysis

14: Coronado St & SR-1

9/18/2014



| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
|------------------------|-------|------|------|--------|------|--------|
| Lane Configurations | | | | | | |
| Volume (vph) | 794 | 15 | 25 | 336 | 235 | 760 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.5 | 3.0 | 3.0 | 3.0 | 5.5 | 5.5 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 1.00 | 1.00 | 0.85 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (prot) | 1770 | 1863 | 1863 | 1583 | 1770 | 1583 |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (perm) | 1770 | 1863 | 1863 | 1583 | 1770 | 1583 |
| Peak-hour factor, PHF | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adj. Flow (vph) | 854 | 16 | 27 | 361 | 253 | 817 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 164 | 0 | 366 |
| Lane Group Flow (vph) | 854 | 16 | 27 | 197 | 253 | 451 |
| Turn Type | Prot | NA | NA | custom | NA | custom |
| Protected Phases | 2! | 5 | | 4 | | 6 |
| Permitted Phases | | | 4 | | 6! | |
| Actuated Green, G (s) | 35.0 | 0.9 | 12.9 | 12.9 | 31.1 | 31.1 |
| Effective Green, g (s) | 35.0 | 0.9 | 12.9 | 12.9 | 31.1 | 31.1 |
| Actuated g/C Ratio | 0.62 | 0.02 | 0.23 | 0.23 | 0.55 | 0.55 |
| Clearance Time (s) | 5.5 | 3.0 | 3.0 | 3.0 | 5.5 | 5.5 |
| Vehicle Extension (s) | 2.4 | 2.5 | 2.0 | 2.0 | 2.4 | 2.4 |
| Lane Grp Cap (vph) | 1098 | 29 | 426 | 362 | 976 | 872 |
| v/s Ratio Prot | c0.48 | 0.01 | | c0.12 | | 0.28 |
| v/s Ratio Perm | | | 0.01 | | 0.14 | |
| v/c Ratio | 0.78 | 0.55 | 0.06 | 0.54 | 0.26 | 0.52 |
| Uniform Delay, d1 | 7.8 | 27.5 | 17.0 | 19.2 | 6.6 | 7.9 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 3.4 | 16.9 | 0.0 | 0.9 | 0.1 | 0.4 |
| Delay (s) | 11.2 | 44.5 | 17.0 | 20.1 | 6.7 | 8.3 |
| Level of Service | B | D | B | C | A | A |
| Approach Delay (s) | | 11.8 | 19.8 | | 7.9 | |
| Approach LOS | | B | B | | A | |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 11.4 | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | 0.76 | | |
| Actuated Cycle Length (s) | 56.4 | Sum of lost time (s) | 11.5 |
| Intersection Capacity Utilization | 71.6% | ICU Level of Service | C |
| Analysis Period (min) | 15 | | |

! Phase conflict between lane groups.

c Critical Lane Group

HCM Unsignalized Intersection Capacity Analysis

15: Coronado St & Obispo Rd

9/18/2014



| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 115 | 178 | 305 | 4 | 4 | 186 |
| Sign Control | | Free | Free | | Stop | |
| Grade | | 0% | 0% | | 0% | |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Hourly flow rate (vph) | 120 | 185 | 318 | 4 | 4 | 194 |
| Pedestrians | | | | | 11 | |
| Lane Width (ft) | | | | | 12.0 | |
| Walking Speed (ft/s) | | | | | 4.0 | |
| Percent Blockage | | | | | 1 | |
| Right turn flare (veh) | | | | | | |
| Median type | | None | None | | | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | 151 | | | | |
| pX, platoon unblocked | | | | | 0.93 | |
| vC, conflicting volume | 333 | | | | 756 | 331 |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 333 | | | | 701 | 331 |
| tC, single (s) | 4.1 | | | | 6.4 | 6.2 |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 2.2 | | | | 3.5 | 3.3 |
| p0 queue free % | 90 | | | | 99 | 72 |
| cM capacity (veh/h) | 1215 | | | | 337 | 704 |

| Direction, Lane # | EB 1 | WB 1 | SB 1 |
|------------------------|------|------|------|
| Volume Total | 305 | 322 | 198 |
| Volume Left | 120 | 0 | 4 |
| Volume Right | 0 | 4 | 194 |
| cSH | 1215 | 1700 | 688 |
| Volume to Capacity | 0.10 | 0.19 | 0.29 |
| Queue Length 95th (ft) | 8 | 0 | 30 |
| Control Delay (s) | 3.8 | 0.0 | 12.3 |
| Lane LOS | A | | B |
| Approach Delay (s) | 3.8 | 0.0 | 12.3 |
| Approach LOS | | | B |

| Intersection Summary | | | |
|-----------------------------------|--|-------|----------------------|
| Average Delay | | 4.4 | |
| Intersection Capacity Utilization | | 53.8% | ICU Level of Service |
| Analysis Period (min) | | 15 | A |

HCM Unsignalized Intersection Capacity Analysis
 16: SR-1 & Magellan Ave

9/18/2014



| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 14 | 20 | 28 | 986 | 1106 | 23 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Hourly flow rate (vph) | 15 | 22 | 31 | 1084 | 1215 | 25 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 2373 | 1228 | 1241 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 2373 | 1228 | 1241 | | | |
| tC, single (s) | 6.4 | 6.2 | 4.1 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | |
| p0 queue free % | 57 | 90 | 95 | | | |
| cM capacity (veh/h) | 36 | 217 | 561 | | | |



















| Direction, Lane # | EB 1 | NB 1 | NB 2 | SB 1 |
|------------------------|-------|------|------|------|
| Volume Total | 37 | 31 | 1084 | 1241 |
| Volume Left | 15 | 31 | 0 | 0 |
| Volume Right | 22 | 0 | 0 | 25 |
| cSH | 71 | 561 | 1700 | 1700 |
| Volume to Capacity | 0.53 | 0.05 | 0.64 | 0.73 |
| Queue Length 95th (ft) | 55 | 4 | 0 | 0 |
| Control Delay (s) | 102.2 | 11.8 | 0.0 | 0.0 |
| Lane LOS | F | B | | |
| Approach Delay (s) | 102.2 | 0.3 | | 0.0 |
| Approach LOS | F | | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|------------------------|
| Average Delay | | 1.7 | |
| Intersection Capacity Utilization | | 69.6% | ICU Level of Service C |
| Analysis Period (min) | | 15 | |

HCM Unsignalized Intersection Capacity Analysis

17: SR-1 & Medio Ave

9/18/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | |  | |  |  | |  |  | |
| Volume (veh/h) | 7 | 0 | 38 | 17 | 0 | 28 | 32 | 981 | 19 | 22 | 1096 | 13 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Hourly flow rate (vph) | 7 | 0 | 40 | 18 | 0 | 29 | 33 | 1022 | 20 | 23 | 1142 | 14 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2312 | 2303 | 1148 | 2326 | 2299 | 1032 | 1155 | | | 1042 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2312 | 2303 | 1148 | 2326 | 2299 | 1032 | 1155 | | | 1042 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 67 | 100 | 84 | 13 | 100 | 90 | 94 | | | 97 | | |
| cM capacity (veh/h) | 22 | 35 | 242 | 20 | 35 | 283 | 605 | | | 668 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 47 | 47 | 33 | 1042 | 23 | 1155 | | | | | | |
| Volume Left | 7 | 18 | 33 | 0 | 23 | 0 | | | | | | |
| Volume Right | 40 | 29 | 0 | 20 | 0 | 14 | | | | | | |
| cSH | 96 | 48 | 605 | 1700 | 668 | 1700 | | | | | | |
| Volume to Capacity | 0.49 | 0.97 | 0.06 | 0.61 | 0.03 | 0.68 | | | | | | |
| Queue Length 95th (ft) | 53 | 103 | 4 | 0 | 3 | 0 | | | | | | |
| Control Delay (s) | 74.5 | 254.8 | 11.3 | 0.0 | 10.6 | 0.0 | | | | | | |
| Lane LOS | F | F | B | | B | | | | | | | |
| Approach Delay (s) | 74.5 | 254.8 | 0.4 | | 0.2 | | | | | | | |
| Approach LOS | F | F | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 6.8 | | | | | | | | | |
| Intersection Capacity Utilization | | | 70.6% | | ICU Level of Service | | | | | C | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

18: SR-1 & Miramar Dr

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↕ | | | ↕ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 2 | 1 | 14 | 1 | 0 | 5 | 4 | 1050 | 12 | 8 | 1108 | 9 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 2 | 1 | 15 | 1 | 0 | 5 | 4 | 1105 | 13 | 8 | 1166 | 9 |
| Pedestrians | | | | | 1 | | | | | | 1 | |
| Lane Width (ft) | | | | | 12.0 | | | | | | 12.0 | |
| Walking Speed (ft/s) | | | | | 4.0 | | | | | | 4.0 | |
| Percent Blockage | | | | | 0 | | | | | | 0 | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2308 | 2315 | 1171 | 2319 | 2314 | 1114 | 1176 | | | 1119 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2308 | 2315 | 1171 | 2319 | 2314 | 1114 | 1176 | | | 1119 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 92 | 97 | 94 | 96 | 100 | 98 | 99 | | | 99 | | |
| cM capacity (veh/h) | 26 | 37 | 235 | 24 | 37 | 253 | 594 | | | 624 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 18 | 6 | 4 | 1118 | 8 | 1176 |
| Volume Left | 2 | 1 | 4 | 0 | 8 | 0 |
| Volume Right | 15 | 5 | 0 | 13 | 0 | 9 |
| cSH | 104 | 97 | 594 | 1700 | 624 | 1700 |
| Volume to Capacity | 0.17 | 0.07 | 0.01 | 0.66 | 0.01 | 0.69 |
| Queue Length 95th (ft) | 15 | 5 | 1 | 0 | 1 | 0 |
| Control Delay (s) | 46.9 | 44.7 | 11.1 | 0.0 | 10.9 | 0.0 |
| Lane LOS | E | E | B | | B | |
| Approach Delay (s) | 46.9 | 44.7 | 0.0 | | 0.1 | |
| Approach LOS | E | E | | | | |

| Intersection Summary | | |
|-----------------------------------|-------|----------------------|
| Average Delay | | 0.5 |
| Intersection Capacity Utilization | 69.2% | ICU Level of Service |
| Analysis Period (min) | | 15 |
| | | C |

HCM Unsignalized Intersection Capacity Analysis

1: SR-1 & 2nd St

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↕ | | | ↕ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 0 | 0 | 0 | 7 | 0 | 50 | 0 | 451 | 8 | 128 | 609 | 0 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 7 | 0 | 51 | 0 | 460 | 8 | 131 | 621 | 0 |
| Pedestrians | | | | | | | | | | | | 4 |
| Lane Width (ft) | | | | | | | | | | | | 12.0 |
| Walking Speed (ft/s) | | | | | | | | | | | | 4.0 |
| Percent Blockage | | | | | | | | | | | | 0 |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | | None |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1398 | 1351 | 621 | 1347 | 1347 | 468 | 621 | | | 468 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1398 | 1351 | 621 | 1347 | 1347 | 468 | 621 | | | 468 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 100 | 94 | 100 | 91 | 100 | | | 88 | | |
| cM capacity (veh/h) | 98 | 132 | 487 | 116 | 133 | 593 | 959 | | | 1093 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 0 | 58 | 0 | 468 | 131 | 621 |
| Volume Left | 0 | 7 | 0 | 0 | 131 | 0 |
| Volume Right | 0 | 51 | 0 | 8 | 0 | 0 |
| cSH | 1700 | 395 | 1700 | 1700 | 1093 | 1700 |
| Volume to Capacity | 0.00 | 0.15 | 0.00 | 0.28 | 0.12 | 0.37 |
| Queue Length 95th (ft) | 0 | 13 | 0 | 0 | 10 | 0 |
| Control Delay (s) | 0.0 | 15.7 | 0.0 | 0.0 | 8.7 | 0.0 |
| Lane LOS | A | C | | | A | |
| Approach Delay (s) | 0.0 | 15.7 | 0.0 | | 1.5 | |
| Approach LOS | A | C | | | | |

Intersection Summary

| | | | | | | |
|-----------------------------------|--|-------|--|----------------------|--|---|
| Average Delay | | 1.6 | | | | |
| Intersection Capacity Utilization | | 50.4% | | ICU Level of Service | | A |
| Analysis Period (min) | | 15 | | | | |

HCM Unsignalized Intersection Capacity Analysis

2: SR-1 & 7th St

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | | ↗ | ↖ | ↖ | | | ↗ | |
| Volume (veh/h) | 0 | 0 | 1 | 0 | 0 | 10 | 0 | 447 | 16 | 0 | 643 | 0 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Hourly flow rate (vph) | 0 | 0 | 1 | 0 | 0 | 11 | 0 | 476 | 17 | 0 | 684 | 0 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1170 | 1177 | 684 | 1169 | 1168 | 484 | 684 | | | 493 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1170 | 1177 | 684 | 1169 | 1168 | 484 | 684 | | | 493 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 100 | 100 | 100 | 98 | 100 | | | 100 | | |
| cM capacity (veh/h) | 167 | 191 | 449 | 170 | 193 | 583 | 909 | | | 1071 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 |
|------------------------|------|------|------|------|------|
| Volume Total | 1 | 11 | 0 | 493 | 684 |
| Volume Left | 0 | 0 | 0 | 0 | 0 |
| Volume Right | 1 | 11 | 0 | 17 | 0 |
| cSH | 449 | 583 | 1700 | 1700 | 1700 |
| Volume to Capacity | 0.00 | 0.02 | 0.00 | 0.29 | 0.40 |
| Queue Length 95th (ft) | 0 | 1 | 0 | 0 | 0 |
| Control Delay (s) | 13.0 | 11.3 | 0.0 | 0.0 | 0.0 |
| Lane LOS | B | B | | | |
| Approach Delay (s) | 13.0 | 11.3 | 0.0 | | 0.0 |
| Approach LOS | B | B | | | |

| Intersection Summary | | |
|-----------------------------------|-------|----------------------|
| Average Delay | | 0.1 |
| Intersection Capacity Utilization | 43.8% | ICU Level of Service |
| Analysis Period (min) | | 15 |
| | | A |

HCM Unsignalized Intersection Capacity Analysis

3: SR-1 & 8th St

9/18/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 88 | 14 | 439 | 12 | 12 | 630 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 93 | 15 | 462 | 13 | 13 | 663 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1157 | 468 | | | 475 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1157 | 468 | | | 475 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 57 | 98 | | | 99 | |
| cM capacity (veh/h) | 215 | 595 | | | 1087 | |

| Direction, Lane # | WB 1 | NB 1 | SB 1 |
|------------------------|------|------|------|
| Volume Total | 107 | 475 | 676 |
| Volume Left | 93 | 0 | 13 |
| Volume Right | 15 | 13 | 0 |
| cSH | 235 | 1700 | 1087 |
| Volume to Capacity | 0.46 | 0.28 | 0.01 |
| Queue Length 95th (ft) | 55 | 0 | 1 |
| Control Delay (s) | 32.5 | 0.0 | 0.3 |
| Lane LOS | D | | A |
| Approach Delay (s) | 32.5 | 0.0 | 0.3 |
| Approach LOS | D | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|----------------------|
| Average Delay | | 2.9 | |
| Intersection Capacity Utilization | | 55.2% | ICU Level of Service |
| Analysis Period (min) | | 15 | B |

HCM Unsignalized Intersection Capacity Analysis

4: SR-1 & Carlos St

9/18/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 0 | 4 | 566 | 0 | 12 | 721 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Hourly flow rate (vph) | 0 | 4 | 590 | 0 | 12 | 751 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1366 | 590 | | | 590 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1366 | 590 | | | 590 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 100 | 99 | | | 99 | |
| cM capacity (veh/h) | 160 | 508 | | | 986 | |

| Direction, Lane # | WB 1 | NB 1 | SB 1 | SB 2 |
|------------------------|------|------|------|------|
| Volume Total | 4 | 590 | 12 | 751 |
| Volume Left | 0 | 0 | 12 | 0 |
| Volume Right | 4 | 0 | 0 | 0 |
| cSH | 508 | 1700 | 986 | 1700 |
| Volume to Capacity | 0.01 | 0.35 | 0.01 | 0.44 |
| Queue Length 95th (ft) | 1 | 0 | 1 | 0 |
| Control Delay (s) | 12.1 | 0.0 | 8.7 | 0.0 |
| Lane LOS | B | | A | |
| Approach Delay (s) | 12.1 | 0.0 | 0.1 | |
| Approach LOS | B | | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|------------------------|
| Average Delay | | 0.1 | |
| Intersection Capacity Utilization | | 47.9% | ICU Level of Service A |
| Analysis Period (min) | | 15 | |

HCM Unsignalized Intersection Capacity Analysis

5: SR-1 & Vallemar St

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↕ | | | ↕ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 2 | 0 | 6 | 8 | 2 | 14 | 9 | 541 | 7 | 44 | 677 | 4 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 2 | 0 | 6 | 8 | 2 | 15 | 9 | 569 | 7 | 46 | 713 | 4 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1412 | 1403 | 715 | 1404 | 1402 | 573 | 717 | | | 577 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1412 | 1403 | 715 | 1404 | 1402 | 573 | 717 | | | 577 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 98 | 100 | 99 | 92 | 98 | 97 | 99 | | | 95 | | |
| cM capacity (veh/h) | 106 | 132 | 431 | 110 | 132 | 519 | 884 | | | 997 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 8 | 25 | 9 | 577 | 46 | 717 |
| Volume Left | 2 | 8 | 9 | 0 | 46 | 0 |
| Volume Right | 6 | 15 | 0 | 7 | 0 | 4 |
| cSH | 244 | 209 | 884 | 1700 | 997 | 1700 |
| Volume to Capacity | 0.03 | 0.12 | 0.01 | 0.34 | 0.05 | 0.42 |
| Queue Length 95th (ft) | 3 | 10 | 1 | 0 | 4 | 0 |
| Control Delay (s) | 20.3 | 24.5 | 9.1 | 0.0 | 8.8 | 0.0 |
| Lane LOS | C | C | A | | A | |
| Approach Delay (s) | 20.3 | 24.5 | 0.1 | | 0.5 | |
| Approach LOS | C | C | | | | |

Intersection Summary

| | |
|-----------------------------------|-------|
| Average Delay | 0.9 |
| Intersection Capacity Utilization | 46.6% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

HCM Unsignalized Intersection Capacity Analysis

6: SR-1 & California Ave

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | ↖ | ↗ | | ↖ | ↗ | |
| Volume (veh/h) | 3 | 0 | 12 | 37 | 2 | 16 | 14 | 561 | 45 | 14 | 660 | 7 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 3 | 0 | 13 | 39 | 2 | 17 | 15 | 591 | 47 | 15 | 695 | 7 |
| Pedestrians | | | | | 4 | | | | | | | 7 |
| Lane Width (ft) | | | | | 12.0 | | | | | | | 12.0 |
| Walking Speed (ft/s) | | | | | 4.0 | | | | | | | 4.0 |
| Percent Blockage | | | | | 0 | | | | | | | 1 |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | | None |
| Median storage veh | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1373 | 1399 | 698 | 1385 | 1379 | 625 | 702 | | | 642 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1373 | 1399 | 698 | 1385 | 1379 | 625 | 702 | | | 642 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 97 | 100 | 97 | 66 | 98 | 96 | 98 | | | 98 | | |
| cM capacity (veh/h) | 114 | 136 | 440 | 114 | 139 | 480 | 895 | | | 940 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 16 | 58 | 15 | 638 | 15 | 702 |
| Volume Left | 3 | 39 | 15 | 0 | 15 | 0 |
| Volume Right | 13 | 17 | 0 | 47 | 0 | 7 |
| cSH | 279 | 148 | 895 | 1700 | 940 | 1700 |
| Volume to Capacity | 0.06 | 0.39 | 0.02 | 0.38 | 0.02 | 0.41 |
| Queue Length 95th (ft) | 4 | 42 | 1 | 0 | 1 | 0 |
| Control Delay (s) | 18.7 | 44.4 | 9.1 | 0.0 | 8.9 | 0.0 |
| Lane LOS | C | E | A | | A | |
| Approach Delay (s) | 18.7 | 44.4 | 0.2 | | 0.2 | |
| Approach LOS | C | E | | | | |

| Intersection Summary | | |
|-----------------------------------|-------|----------------------|
| Average Delay | | 2.2 |
| Intersection Capacity Utilization | 51.5% | ICU Level of Service |
| Analysis Period (min) | | 15 |
| | | A |

HCM Unsignalized Intersection Capacity Analysis

7: SR-1 & Virginia Ave

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 8 | 2 | 15 | 19 | 1 | 11 | 15 | 599 | 26 | 9 | 676 | 9 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 9 | 2 | 16 | 20 | 1 | 12 | 16 | 644 | 28 | 10 | 727 | 10 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1440 | 1455 | 732 | 1454 | 1446 | 658 | 737 | | | 672 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1440 | 1455 | 732 | 1454 | 1446 | 658 | 737 | | | 672 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 92 | 98 | 96 | 80 | 99 | 97 | 98 | | | 99 | | |
| cM capacity (veh/h) | 105 | 126 | 421 | 100 | 128 | 464 | 869 | | | 919 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 27 | 33 | 16 | 672 | 10 | 737 |
| Volume Left | 9 | 20 | 16 | 0 | 10 | 0 |
| Volume Right | 16 | 12 | 0 | 28 | 0 | 10 |
| cSH | 196 | 140 | 869 | 1700 | 919 | 1700 |
| Volume to Capacity | 0.14 | 0.24 | 0.02 | 0.40 | 0.01 | 0.43 |
| Queue Length 95th (ft) | 12 | 22 | 1 | 0 | 1 | 0 |
| Control Delay (s) | 26.3 | 38.5 | 9.2 | 0.0 | 9.0 | 0.0 |
| Lane LOS | D | E | A | | A | |
| Approach Delay (s) | 26.3 | 38.5 | 0.2 | | 0.1 | |
| Approach LOS | D | E | | | | |

Intersection Summary

| | | | |
|-----------------------------------|-------|-----|------------------------|
| Average Delay | | 1.5 | |
| Intersection Capacity Utilization | 46.1% | | ICU Level of Service A |
| Analysis Period (min) | 15 | | |

HCM Unsignalized Intersection Capacity Analysis

8: SR-1 & Vermont Ave

9/18/2014






















| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------------|-------------|-------------|-------------|----------------------|-------------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | ↔ | ↔ | | ↔ | ↔ | |
| Volume (veh/h) | 8 | 2 | 15 | 21 | 1 | 5 | 7 | 635 | 32 | 7 | 695 | 6 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 8 | 2 | 16 | 22 | 1 | 5 | 7 | 668 | 34 | 7 | 732 | 6 |
| Pedestrians | | 1 | | | | | | | | | | |
| Lane Width (ft) | | 12.0 | | | | | | | | | | |
| Walking Speed (ft/s) | | 4.0 | | | | | | | | | | |
| Percent Blockage | | 0 | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1439 | 1467 | 736 | 1463 | 1454 | 685 | 739 | | | 702 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1439 | 1467 | 736 | 1463 | 1454 | 685 | 739 | | | 702 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 92 | 98 | 96 | 78 | 99 | 99 | 99 | | | 99 | | |
| cM capacity (veh/h) | 107 | 125 | 419 | 100 | 128 | 448 | 867 | | | 895 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 26 | 28 | 7 | 702 | 7 | 738 | | | | | | |
| Volume Left | 8 | 22 | 7 | 0 | 7 | 0 | | | | | | |
| Volume Right | 16 | 5 | 0 | 34 | 0 | 6 | | | | | | |
| cSH | 198 | 118 | 867 | 1700 | 895 | 1700 | | | | | | |
| Volume to Capacity | 0.13 | 0.24 | 0.01 | 0.41 | 0.01 | 0.43 | | | | | | |
| Queue Length 95th (ft) | 11 | 22 | 1 | 0 | 1 | 0 | | | | | | |
| Control Delay (s) | 26.0 | 45.0 | 9.2 | 0.0 | 9.1 | 0.0 | | | | | | |
| Lane LOS | D | E | A | | A | | | | | | | |
| Approach Delay (s) | 26.0 | 45.0 | 0.1 | | 0.1 | | | | | | | |
| Approach LOS | D | E | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 1.4 | | | | | | | | | |
| Intersection Capacity Utilization | | | 46.9% | | ICU Level of Service | | | | | A | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

9: SR-1 & Cypress Ave

9/18/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | |  | |  |  | |  |  |  |
| Volume (veh/h) | 11 | 1 | 1 | 55 | 7 | 26 | 7 | 602 | 68 | 35 | 696 | 11 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 12 | 1 | 1 | 60 | 8 | 28 | 8 | 654 | 74 | 38 | 757 | 12 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1540 | 1582 | 762 | 1541 | 1551 | 691 | 768 | | | 728 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1540 | 1582 | 762 | 1541 | 1551 | 691 | 768 | | | 728 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 85 | 99 | 100 | 33 | 93 | 94 | 99 | | | 96 | | |
| cM capacity (veh/h) | 80 | 103 | 405 | 89 | 108 | 444 | 846 | | | 875 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 14 | 96 | 8 | 728 | 38 | 768 | | | | | | |
| Volume Left | 12 | 60 | 8 | 0 | 38 | 0 | | | | | | |
| Volume Right | 1 | 28 | 0 | 74 | 0 | 12 | | | | | | |
| cSH | 87 | 119 | 846 | 1700 | 875 | 1700 | | | | | | |
| Volume to Capacity | 0.16 | 0.80 | 0.01 | 0.43 | 0.04 | 0.45 | | | | | | |
| Queue Length 95th (ft) | 14 | 118 | 1 | 0 | 3 | 0 | | | | | | |
| Control Delay (s) | 54.3 | 104.6 | 9.3 | 0.0 | 9.3 | 0.0 | | | | | | |
| Lane LOS | F | F | A | | A | | | | | | | |
| Approach Delay (s) | 54.3 | 104.6 | 0.1 | | 0.4 | | | | | | | |
| Approach LOS | F | F | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 6.8 | | | | | | | | | |
| Intersection Capacity Utilization | | 48.9% | | ICU Level of Service | A | | | | | | | |
| Analysis Period (min) | | 15 | | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

10: SR-1 & St Etheldore St

9/18/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 20 | 0 | 637 | 26 | 0 | 694 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 22 | 0 | 685 | 28 | 0 | 746 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1445 | 699 | | | 713 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1445 | 699 | | | 713 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 85 | 100 | | | 100 | |
| cM capacity (veh/h) | 145 | 440 | | | 887 | |

| Direction, Lane # | WB 1 | NB 1 | SB 1 |
|------------------------|------|------|------|
| Volume Total | 22 | 713 | 746 |
| Volume Left | 22 | 0 | 0 |
| Volume Right | 0 | 28 | 0 |
| cSH | 145 | 1700 | 887 |
| Volume to Capacity | 0.15 | 0.42 | 0.00 |
| Queue Length 95th (ft) | 13 | 0 | 0 |
| Control Delay (s) | 34.1 | 0.0 | 0.0 |
| Lane LOS | D | | |
| Approach Delay (s) | 34.1 | 0.0 | 0.0 |
| Approach LOS | D | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|----------------------|
| Average Delay | | 0.5 | |
| Intersection Capacity Utilization | | 46.5% | ICU Level of Service |
| Analysis Period (min) | | 15 | A |

HCM Unsignalized Intersection Capacity Analysis

11: SR-1 & Capistrano Rd

9/18/2014



| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 27 | 15 | 12 | 663 | 676 | 48 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 29 | 16 | 13 | 713 | 727 | 52 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1109 | 727 | 727 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1109 | 727 | 727 | | | |
| tC, single (s) | 6.8 | 6.9 | 4.1 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | |
| p0 queue free % | 86 | 96 | 99 | | | |
| cM capacity (veh/h) | 201 | 366 | 872 | | | |

| Direction, Lane # | EB 1 | EB 2 | NB 1 | NB 2 | NB 3 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|------|
| Volume Total | 29 | 16 | 13 | 356 | 356 | 727 | 52 |
| Volume Left | 29 | 0 | 13 | 0 | 0 | 0 | 0 |
| Volume Right | 0 | 16 | 0 | 0 | 0 | 0 | 52 |
| cSH | 201 | 366 | 872 | 1700 | 1700 | 1700 | 1700 |
| Volume to Capacity | 0.14 | 0.04 | 0.01 | 0.21 | 0.21 | 0.43 | 0.03 |
| Queue Length 95th (ft) | 12 | 3 | 1 | 0 | 0 | 0 | 0 |
| Control Delay (s) | 26.0 | 15.3 | 9.2 | 0.0 | 0.0 | 0.0 | 0.0 |
| Lane LOS | D | C | A | | | | |
| Approach Delay (s) | 22.1 | | 0.2 | | | 0.0 | |
| Approach LOS | C | | | | | | |

| Intersection Summary | | | |
|-----------------------------------|-------|-----|------------------------|
| Average Delay | | 0.7 | |
| Intersection Capacity Utilization | 45.6% | | ICU Level of Service A |
| Analysis Period (min) | 15 | | |

HCM Unsignalized Intersection Capacity Analysis

12: Coral Reef Ave & SR-1

9/18/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 18 | 30 | 619 | 56 | 66 | 620 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Hourly flow rate (vph) | 20 | 33 | 680 | 62 | 73 | 681 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1537 | 711 | | | 742 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1537 | 711 | | | 742 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 83 | 92 | | | 92 | |
| cM capacity (veh/h) | 117 | 433 | | | 865 | |

| Direction, Lane # | WB 1 | WB 2 | NB 1 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|
| Volume Total | 20 | 33 | 742 | 73 | 681 |
| Volume Left | 20 | 0 | 0 | 73 | 0 |
| Volume Right | 0 | 33 | 62 | 0 | 0 |
| cSH | 117 | 433 | 1700 | 865 | 1700 |
| Volume to Capacity | 0.17 | 0.08 | 0.44 | 0.08 | 0.40 |
| Queue Length 95th (ft) | 15 | 6 | 0 | 7 | 0 |
| Control Delay (s) | 42.0 | 14.0 | 0.0 | 9.5 | 0.0 |
| Lane LOS | E | B | | A | |
| Approach Delay (s) | 24.5 | | 0.0 | 0.9 | |
| Approach LOS | C | | | | |

| Intersection Summary | | | | | |
|-----------------------------------|--|--|-------|----------------------|---|
| Average Delay | | | 1.3 | | |
| Intersection Capacity Utilization | | | 53.0% | ICU Level of Service | A |
| Analysis Period (min) | | | 15 | | |

HCM Signalized Intersection Capacity Analysis

13: Capistrano Rd

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|------|------|------|------|-------|------|------|------|------|-------|-------|------|
| Lane Configurations | | ↖ | ↗ | | ↔ | | ↖ | ↗ | | ↖ | ↗ | ↗ |
| Volume (vph) | 66 | 74 | 78 | 46 | 78 | 188 | 137 | 467 | 26 | 198 | 617 | 65 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 3.5 | 3.5 | | 3.5 | | 3.0 | 6.5 | | 3.0 | 6.5 | 6.5 |
| Lane Util. Factor | | 1.00 | 1.00 | | 1.00 | | 0.97 | 0.95 | | 1.00 | 0.95 | 1.00 |
| Fr _t | | 1.00 | 0.85 | | 0.92 | | 1.00 | 0.99 | | 1.00 | 1.00 | 0.85 |
| Fl _t Protected | | 0.98 | 1.00 | | 0.99 | | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | | 1820 | 1583 | | 1699 | | 3433 | 3511 | | 1770 | 3539 | 1583 |
| Fl _t Permitted | | 0.65 | 1.00 | | 0.94 | | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | | 1219 | 1583 | | 1603 | | 3433 | 3511 | | 1770 | 3539 | 1583 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 72 | 80 | 85 | 50 | 85 | 204 | 149 | 508 | 28 | 215 | 671 | 71 |
| RTOR Reduction (vph) | 0 | 0 | 63 | 0 | 58 | 0 | 0 | 4 | 0 | 0 | 0 | 42 |
| Lane Group Flow (vph) | 0 | 152 | 22 | 0 | 281 | 0 | 149 | 532 | 0 | 215 | 671 | 29 |
| Turn Type | Perm | NA | Perm | Perm | NA | | Prot | NA | | Prot | NA | Perm |
| Protected Phases | | 8 | | | 4 | | 1 | 6 | | 5 | 2 | |
| Permitted Phases | 8 | | 8 | 4 | | | | | | | | 2 |
| Actuated Green, G (s) | | 15.2 | 15.2 | | 15.2 | | 6.0 | 17.8 | | 12.4 | 24.2 | 24.2 |
| Effective Green, g (s) | | 15.2 | 15.2 | | 15.2 | | 6.0 | 17.8 | | 12.4 | 24.2 | 24.2 |
| Actuated g/C Ratio | | 0.26 | 0.26 | | 0.26 | | 0.10 | 0.30 | | 0.21 | 0.41 | 0.41 |
| Clearance Time (s) | | 3.5 | 3.5 | | 3.5 | | 3.0 | 6.5 | | 3.0 | 6.5 | 6.5 |
| Vehicle Extension (s) | | 2.0 | 2.0 | | 2.0 | | 2.0 | 4.0 | | 2.0 | 4.0 | 4.0 |
| Lane Grp Cap (vph) | | 317 | 412 | | 417 | | 352 | 1070 | | 375 | 1466 | 655 |
| v/s Ratio Prot | | | | | | | 0.04 | 0.15 | | c0.12 | c0.19 | |
| v/s Ratio Perm | | 0.12 | 0.01 | | c0.18 | | | | | | | 0.02 |
| v/c Ratio | | 0.48 | 0.05 | | 0.67 | | 0.42 | 0.50 | | 0.57 | 0.46 | 0.04 |
| Uniform Delay, d ₁ | | 18.3 | 16.2 | | 19.4 | | 24.6 | 16.6 | | 20.6 | 12.4 | 10.2 |
| Progression Factor | | 1.00 | 1.00 | | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d ₂ | | 0.4 | 0.0 | | 3.4 | | 0.3 | 0.5 | | 1.3 | 0.3 | 0.0 |
| Delay (s) | | 18.7 | 16.2 | | 22.7 | | 24.9 | 17.1 | | 21.9 | 12.7 | 10.2 |
| Level of Service | | B | B | | C | | C | B | | C | B | B |
| Approach Delay (s) | | 17.8 | | | 22.7 | | | 18.8 | | | 14.6 | |
| Approach LOS | | B | | | C | | | B | | | B | |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 17.5 | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | 0.59 | | |
| Actuated Cycle Length (s) | 58.4 | Sum of lost time (s) | 13.0 |
| Intersection Capacity Utilization | 61.6% | ICU Level of Service | B |
| Analysis Period (min) | 15 | | |
| c Critical Lane Group | | | |

HCM Signalized Intersection Capacity Analysis

14: SR-1 & Coronado St

9/18/2014



| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
|------------------------|------|-------|-------|------|-------|------|
| Lane Configurations | | | | | | |
| Volume (vph) | 34 | 658 | 834 | 394 | 232 | 21 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 3.0 | 5.5 | 5.5 | 5.5 | 3.0 | 3.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frbp, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.94 |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 1.00 | 1.00 | 0.85 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (prot) | 1770 | 1863 | 1863 | 1583 | 1770 | 1496 |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (perm) | 1770 | 1863 | 1863 | 1583 | 1770 | 1496 |
| Peak-hour factor, PHF | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adj. Flow (vph) | 37 | 708 | 897 | 424 | 249 | 23 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 167 | 0 | 18 |
| Lane Group Flow (vph) | 37 | 708 | 897 | 257 | 249 | 5 |
| Confl. Peds. (#/hr) | | | | | | 23 |
| Turn Type | Prot | NA | NA | Perm | NA | Perm |
| Protected Phases | 5 | 2 | 6 | | 4 | |
| Permitted Phases | | | | 6 | | 4 |
| Actuated Green, G (s) | 4.1 | 58.1 | 51.0 | 51.0 | 17.5 | 17.5 |
| Effective Green, g (s) | 4.1 | 58.1 | 51.0 | 51.0 | 17.5 | 17.5 |
| Actuated g/C Ratio | 0.05 | 0.69 | 0.61 | 0.61 | 0.21 | 0.21 |
| Clearance Time (s) | 3.0 | 5.5 | 5.5 | 5.5 | 3.0 | 3.0 |
| Vehicle Extension (s) | 2.5 | 2.4 | 2.4 | 2.4 | 2.0 | 2.0 |
| Lane Grp Cap (vph) | 86 | 1287 | 1129 | 959 | 368 | 311 |
| v/s Ratio Prot | 0.02 | c0.38 | c0.48 | | c0.14 | |
| v/s Ratio Perm | | | | 0.16 | | 0.00 |
| v/c Ratio | 0.43 | 0.55 | 0.79 | 0.27 | 0.68 | 0.02 |
| Uniform Delay, d1 | 38.9 | 6.5 | 12.6 | 7.8 | 30.7 | 26.5 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 2.5 | 0.4 | 3.8 | 0.1 | 3.8 | 0.0 |
| Delay (s) | 41.4 | 6.9 | 16.4 | 7.9 | 34.5 | 26.5 |
| Level of Service | D | A | B | A | C | C |
| Approach Delay (s) | | 8.6 | 13.6 | | 33.9 | |
| Approach LOS | | A | B | | C | |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 14.4 | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | 0.76 | | |
| Actuated Cycle Length (s) | 84.1 | Sum of lost time (s) | 11.5 |
| Intersection Capacity Utilization | 69.8% | ICU Level of Service | C |
| Analysis Period (min) | 15 | | |

c Critical Lane Group

HCM Unsignalized Intersection Capacity Analysis

15: Coronado St & Obispo Rd

9/18/2014



| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | | ↔ | |
| Volume (veh/h) | 118 | 308 | 160 | 2 | 4 | 86 |
| Sign Control | | Free | Free | | Stop | |
| Grade | | 0% | 0% | | 0% | |
| Peak Hour Factor | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 |
| Hourly flow rate (vph) | 133 | 346 | 180 | 2 | 4 | 97 |
| Pedestrians | | | 1 | | 5 | |
| Lane Width (ft) | | | 12.0 | | 12.0 | |
| Walking Speed (ft/s) | | | 4.0 | | 4.0 | |
| Percent Blockage | | | 0 | | 0 | |
| Right turn flare (veh) | | | | | | |
| Median type | | None | None | | | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | 151 | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 187 | | | | 798 | 186 |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 187 | | | | 798 | 186 |
| tC, single (s) | 4.1 | | | | 6.4 | 6.2 |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 2.2 | | | | 3.5 | 3.3 |
| p0 queue free % | 90 | | | | 99 | 89 |
| cM capacity (veh/h) | 1382 | | | | 319 | 853 |

| Direction, Lane # | EB 1 | WB 1 | SB 1 |
|------------------------|------|------|------|
| Volume Total | 479 | 182 | 101 |
| Volume Left | 133 | 0 | 4 |
| Volume Right | 0 | 2 | 97 |
| cSH | 1382 | 1700 | 794 |
| Volume to Capacity | 0.10 | 0.11 | 0.13 |
| Queue Length 95th (ft) | 8 | 0 | 11 |
| Control Delay (s) | 2.9 | 0.0 | 10.2 |
| Lane LOS | A | | B |
| Approach Delay (s) | 2.9 | 0.0 | 10.2 |
| Approach LOS | | | B |

| Intersection Summary | | | |
|-----------------------------------|-------|-----|------------------------|
| Average Delay | | 3.2 | |
| Intersection Capacity Utilization | 47.6% | | ICU Level of Service A |
| Analysis Period (min) | 15 | | |

HCM Unsignalized Intersection Capacity Analysis

16: SR-1 & Magellan Ave

9/18/2014






















| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|-----------------------------------|------|------|-------|----------------------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 14 | 13 | 24 | 1211 | 863 | 29 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 15 | 14 | 25 | 1275 | 908 | 31 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 2249 | 924 | 939 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 2249 | 924 | 939 | | | |
| tC, single (s) | 6.4 | 6.2 | 4.1 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | |
| p0 queue free % | 67 | 96 | 97 | | | |
| cM capacity (veh/h) | 44 | 327 | 730 | | | |
| Direction, Lane # | EB 1 | NB 1 | NB 2 | SB 1 | | |
| Volume Total | 28 | 25 | 1275 | 939 | | |
| Volume Left | 15 | 25 | 0 | 0 | | |
| Volume Right | 14 | 0 | 0 | 31 | | |
| cSH | 76 | 730 | 1700 | 1700 | | |
| Volume to Capacity | 0.37 | 0.03 | 0.75 | 0.55 | | |
| Queue Length 95th (ft) | 36 | 3 | 0 | 0 | | |
| Control Delay (s) | 78.5 | 10.1 | 0.0 | 0.0 | | |
| Lane LOS | F | B | | | | |
| Approach Delay (s) | 78.5 | 0.2 | | 0.0 | | |
| Approach LOS | F | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 1.1 | | | |
| Intersection Capacity Utilization | | | 73.7% | ICU Level of Service | D | |
| Analysis Period (min) | | | 15 | | | |

HCM Unsignalized Intersection Capacity Analysis

17: SR-1 & Medio Ave

9/18/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | |  | |  |  | |  |  |  |
| Volume (veh/h) | 3 | 0 | 24 | 6 | 0 | 18 | 20 | 1197 | 30 | 15 | 852 | 8 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Hourly flow rate (vph) | 3 | 0 | 25 | 6 | 0 | 19 | 21 | 1247 | 31 | 16 | 888 | 8 |
| Pedestrians | | 1 | | | | | | | | | | 2 |
| Lane Width (ft) | | 12.0 | | | | | | | | | | 12.0 |
| Walking Speed (ft/s) | | 4.0 | | | | | | | | | | 4.0 |
| Percent Blockage | | 0 | | | | | | | | | | 0 |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | | None |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2233 | 2244 | 893 | 2248 | 2232 | 1264 | 897 | | | 1278 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2233 | 2244 | 893 | 2248 | 2232 | 1264 | 897 | | | 1278 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 88 | 100 | 93 | 76 | 100 | 91 | 97 | | | 97 | | |
| cM capacity (veh/h) | 26 | 40 | 340 | 26 | 40 | 206 | 756 | | | 543 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 28 | 25 | 21 | 1278 | 16 | 896 | | | | | | |
| Volume Left | 3 | 6 | 21 | 0 | 16 | 0 | | | | | | |
| Volume Right | 25 | 19 | 0 | 31 | 0 | 8 | | | | | | |
| cSH | 146 | 76 | 756 | 1700 | 543 | 1700 | | | | | | |
| Volume to Capacity | 0.19 | 0.33 | 0.03 | 0.75 | 0.03 | 0.53 | | | | | | |
| Queue Length 95th (ft) | 17 | 31 | 2 | 0 | 2 | 0 | | | | | | |
| Control Delay (s) | 35.3 | 73.9 | 9.9 | 0.0 | 11.8 | 0.0 | | | | | | |
| Lane LOS | E | F | A | | B | | | | | | | |
| Approach Delay (s) | 35.3 | 73.9 | 0.2 | | 0.2 | | | | | | | |
| Approach LOS | E | F | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 1.4 | | | | | | | | | |
| Intersection Capacity Utilization | | | 75.5% | | ICU Level of Service | | | | | D | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

18: SR-1 & Miramar Dr

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 1 | 0 | 1 | 0 | 0 | 4 | 0 | 1293 | 2 | 5 | 869 | 3 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 1 | 0 | 1 | 0 | 0 | 4 | 0 | 1405 | 2 | 5 | 945 | 3 |
| Pedestrians | | | | | 1 | | | | | | 1 | |
| Lane Width (ft) | | | | | 12.0 | | | | | | 12.0 | |
| Walking Speed (ft/s) | | | | | 4.0 | | | | | | 4.0 | |
| Percent Blockage | | | | | 0 | | | | | | 0 | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2368 | 2366 | 946 | 2364 | 2366 | 1409 | 948 | | | 1409 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2368 | 2366 | 946 | 2364 | 2366 | 1409 | 948 | | | 1409 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 95 | 100 | 100 | 100 | 100 | 97 | 100 | | | 99 | | |
| cM capacity (veh/h) | 23 | 35 | 317 | 24 | 35 | 170 | 724 | | | 484 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 2 | 4 | 0 | 1408 | 5 | 948 |
| Volume Left | 1 | 0 | 0 | 0 | 5 | 0 |
| Volume Right | 1 | 4 | 0 | 2 | 0 | 3 |
| cSH | 44 | 170 | 1700 | 1700 | 484 | 1700 |
| Volume to Capacity | 0.05 | 0.03 | 0.00 | 0.83 | 0.01 | 0.56 |
| Queue Length 95th (ft) | 4 | 2 | 0 | 0 | 1 | 0 |
| Control Delay (s) | 91.7 | 26.8 | 0.0 | 0.0 | 12.5 | 0.0 |
| Lane LOS | F | D | | | B | |
| Approach Delay (s) | 91.7 | 26.8 | 0.0 | | 0.1 | |
| Approach LOS | F | D | | | | |

Intersection Summary

| | |
|-----------------------------------|-------|
| Average Delay | 0.2 |
| Intersection Capacity Utilization | 78.5% |
| ICU Level of Service | D |
| Analysis Period (min) | 15 |

SR-1 Buildout Conditions Report

HCM Unsignalized Intersection Capacity Analysis

1: SR-1 & 2nd St

9/23/2014




















| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------------|-------------|-------------|-------------|----------------------|-------------|------|------|------|------|------|------|
| Lane Configurations | | ↕ | | | ↕ | | ↕ | ↕ | | ↕ | ↕ | |
| Volume (veh/h) | 0 | 0 | 0 | 26 | 0 | 164 | 0 | 872 | 13 | 50 | 777 | 0 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 27 | 0 | 173 | 0 | 918 | 14 | 53 | 818 | 0 |
| Pedestrians | | | | | | | | | | | | 3 |
| Lane Width (ft) | | | | | | | | | | | | 12.0 |
| Walking Speed (ft/s) | | | | | | | | | | | | 4.0 |
| Percent Blockage | | | | | | | | | | | | 0 |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | | None |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2017 | 1855 | 818 | 1848 | 1848 | 928 | 818 | | | 932 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2017 | 1855 | 818 | 1848 | 1848 | 928 | 818 | | | 932 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 100 | 49 | 100 | 47 | 100 | | | 93 | | |
| cM capacity (veh/h) | 19 | 69 | 376 | 54 | 69 | 324 | 810 | | | 735 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 0 | 200 | 0 | 932 | 53 | 818 | | | | | | |
| Volume Left | 0 | 27 | 0 | 0 | 53 | 0 | | | | | | |
| Volume Right | 0 | 173 | 0 | 14 | 0 | 0 | | | | | | |
| cSH | 1700 | 193 | 1700 | 1700 | 735 | 1700 | | | | | | |
| Volume to Capacity | 0.00 | 1.04 | 0.00 | 0.55 | 0.07 | 0.48 | | | | | | |
| Queue Length 95th (ft) | 0 | 228 | 0 | 0 | 6 | 0 | | | | | | |
| Control Delay (s) | 0.0 | 126.3 | 0.0 | 0.0 | 10.3 | 0.0 | | | | | | |
| Lane LOS | A | F | | | B | | | | | | | |
| Approach Delay (s) | 0.0 | 126.3 | 0.0 | | 0.6 | | | | | | | |
| Approach LOS | A | F | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | | 12.9 | | | | | | | | |
| Intersection Capacity Utilization | | | 65.3% | | ICU Level of Service | | | | | C | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

2: SR-1 & 7th St

9/23/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|--|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | | |  |  |  | | |  | |
| Volume (veh/h) | 0 | 0 | 0 | 0 | 0 | 69 | 0 | 808 | 26 | 0 | 796 | 0 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 0 | 0 | 72 | 0 | 842 | 27 | 0 | 829 | 0 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1743 | 1698 | 829 | 1684 | 1684 | 855 | 829 | | | 869 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1743 | 1698 | 829 | 1684 | 1684 | 855 | 829 | | | 869 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 100 | 100 | 100 | 80 | 100 | | | 100 | | |
| cM capacity (veh/h) | 54 | 92 | 370 | 75 | 94 | 358 | 802 | | | 776 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | | | | | | | |
| Volume Total | 0 | 72 | 0 | 869 | 829 | | | | | | | |
| Volume Left | 0 | 0 | 0 | 0 | 0 | | | | | | | |
| Volume Right | 0 | 72 | 0 | 27 | 0 | | | | | | | |
| cSH | 1700 | 358 | 1700 | 1700 | 1700 | | | | | | | |
| Volume to Capacity | 0.00 | 0.20 | 0.00 | 0.51 | 0.49 | | | | | | | |
| Queue Length 95th (ft) | 0 | 18 | 0 | 0 | 0 | | | | | | | |
| Control Delay (s) | 0.0 | 17.6 | 0.0 | 0.0 | 0.0 | | | | | | | |
| Lane LOS | A | C | | | | | | | | | | |
| Approach Delay (s) | 0.0 | 17.6 | 0.0 | | 0.0 | | | | | | | |
| Approach LOS | A | C | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 0.7 | | | | | | | | | |
| Intersection Capacity Utilization | | | 55.0% | ICU Level of Service | | B | | | | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

3: SR-1 & 8th St

9/23/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 116 | 19 | 823 | 19 | 7 | 783 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 | 0.99 |
| Hourly flow rate (vph) | 117 | 19 | 831 | 19 | 7 | 791 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1646 | 841 | | | 851 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1646 | 841 | | | 851 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 0 | 95 | | | 99 | |
| cM capacity (veh/h) | 108 | 365 | | | 788 | |

| Direction, Lane # | WB 1 | NB 1 | SB 1 |
|------------------------|-------|------|------|
| Volume Total | 136 | 851 | 798 |
| Volume Left | 117 | 0 | 7 |
| Volume Right | 19 | 19 | 0 |
| cSH | 120 | 1700 | 788 |
| Volume to Capacity | 1.13 | 0.50 | 0.01 |
| Queue Length 95th (ft) | 206 | 0 | 1 |
| Control Delay (s) | 192.4 | 0.0 | 0.2 |
| Lane LOS | F | | A |
| Approach Delay (s) | 192.4 | 0.0 | 0.2 |
| Approach LOS | F | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|----------------------|
| Average Delay | | 14.8 | |
| Intersection Capacity Utilization | | 61.0% | ICU Level of Service |
| Analysis Period (min) | | 15 | B |

HCM Unsignalized Intersection Capacity Analysis

4: SR-1 & Carlos St

9/23/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 0 | 28 | 847 | 0 | 14 | 1009 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Hourly flow rate (vph) | 0 | 30 | 901 | 0 | 15 | 1073 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 2004 | 901 | | | 901 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 2004 | 901 | | | 901 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 100 | 91 | | | 98 | |
| cM capacity (veh/h) | 64 | 337 | | | 754 | |



















| Direction, Lane # | WB 1 | NB 1 | SB 1 | SB 2 |
|------------------------|------|------|------|------|
| Volume Total | 30 | 901 | 15 | 1073 |
| Volume Left | 0 | 0 | 15 | 0 |
| Volume Right | 30 | 0 | 0 | 0 |
| cSH | 337 | 1700 | 754 | 1700 |
| Volume to Capacity | 0.09 | 0.53 | 0.02 | 0.63 |
| Queue Length 95th (ft) | 7 | 0 | 2 | 0 |
| Control Delay (s) | 16.7 | 0.0 | 9.9 | 0.0 |
| Lane LOS | C | | A | |
| Approach Delay (s) | 16.7 | 0.0 | 0.1 | |
| Approach LOS | C | | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|----------------------|
| Average Delay | | 0.3 | |
| Intersection Capacity Utilization | | 63.1% | ICU Level of Service |
| Analysis Period (min) | | 15 | B |

HCM Unsignalized Intersection Capacity Analysis

5: SR-1 & Vallemar St

9/23/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | |  | |  |  | |  |  | |
| Volume (veh/h) | 0 | 0 | 0 | 5 | 0 | 35 | 0 | 803 | 1 | 15 | 1010 | 0 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 5 | 0 | 36 | 0 | 836 | 1 | 16 | 1052 | 0 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1956 | 1921 | 1052 | 1920 | 1920 | 837 | 1052 | | | 838 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1956 | 1921 | 1052 | 1920 | 1920 | 837 | 1052 | | | 838 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 100 | 90 | 100 | 90 | 100 | | | 98 | | |
| cM capacity (veh/h) | 43 | 66 | 275 | 50 | 66 | 367 | 662 | | | 797 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 0 | 42 | 0 | 838 | 16 | 1052 | | | | | | |
| Volume Left | 0 | 5 | 0 | 0 | 16 | 0 | | | | | | |
| Volume Right | 0 | 36 | 0 | 1 | 0 | 0 | | | | | | |
| cSH | 1700 | 205 | 1700 | 1700 | 797 | 1700 | | | | | | |
| Volume to Capacity | 0.00 | 0.20 | 0.00 | 0.49 | 0.02 | 0.62 | | | | | | |
| Queue Length 95th (ft) | 0 | 18 | 0 | 0 | 1 | 0 | | | | | | |
| Control Delay (s) | 0.0 | 27.0 | 0.0 | 0.0 | 9.6 | 0.0 | | | | | | |
| Lane LOS | A | D | | | A | | | | | | | |
| Approach Delay (s) | 0.0 | 27.0 | 0.0 | | 0.1 | | | | | | | |
| Approach LOS | A | D | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 0.7 | | | | | | | | | |
| Intersection Capacity Utilization | | | 63.2% | ICU Level of Service | | B | | | | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

6: SR-1 & California Ave

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 10 | 1 | 19 | 49 | 0 | 15 | 14 | 752 | 43 | 15 | 992 | 0 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 11 | 1 | 20 | 53 | 0 | 16 | 15 | 809 | 46 | 16 | 1067 | 0 |
| Pedestrians | | | | | | | | | | | | 2 |
| Lane Width (ft) | | | | | | | | | | | | 12.0 |
| Walking Speed (ft/s) | | | | | | | | | | | | 4.0 |
| Percent Blockage | | | | | | | | | | | | 0 |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | | None |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1956 | 1984 | 1067 | 1982 | 1961 | 834 | 1067 | | | 855 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1956 | 1984 | 1067 | 1982 | 1961 | 834 | 1067 | | | 855 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 76 | 98 | 92 | 0 | 100 | 96 | 98 | | | 98 | | |
| cM capacity (veh/h) | 44 | 59 | 270 | 41 | 61 | 368 | 653 | | | 785 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|-------|------|------|------|------|
| Volume Total | 32 | 69 | 15 | 855 | 16 | 1067 |
| Volume Left | 11 | 53 | 15 | 0 | 16 | 0 |
| Volume Right | 20 | 16 | 0 | 46 | 0 | 0 |
| cSH | 96 | 51 | 653 | 1700 | 785 | 1700 |
| Volume to Capacity | 0.34 | 1.34 | 0.02 | 0.50 | 0.02 | 0.63 |
| Queue Length 95th (ft) | 33 | 157 | 2 | 0 | 2 | 0 |
| Control Delay (s) | 60.6 | 373.0 | 10.6 | 0.0 | 9.7 | 0.0 |
| Lane LOS | F | F | B | | A | |
| Approach Delay (s) | 60.6 | 373.0 | 0.2 | | 0.1 | |
| Approach LOS | F | F | | | | |

Intersection Summary

| | |
|-----------------------------------|-------|
| Average Delay | 13.6 |
| Intersection Capacity Utilization | 66.8% |
| ICU Level of Service | C |
| Analysis Period (min) | 15 |

HCM Unsignalized Intersection Capacity Analysis

7: SR-1 & Virginia Ave

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↕ | | | ↕ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 3 | 0 | 15 | 10 | 2 | 3 | 21 | 798 | 5 | 2 | 1033 | 8 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 3 | 0 | 16 | 11 | 2 | 3 | 22 | 840 | 5 | 2 | 1087 | 8 |
| Pedestrians | | 1 | | | | | | 1 | | | | |
| Lane Width (ft) | | 12.0 | | | | | | 12.0 | | | | |
| Walking Speed (ft/s) | | 4.0 | | | | | | 4.0 | | | | |
| Percent Blockage | | 0 | | | | | | 0 | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1985 | 1986 | 1094 | 1995 | 1988 | 843 | 1097 | | | 845 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1985 | 1986 | 1094 | 1995 | 1988 | 843 | 1097 | | | 845 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 93 | 100 | 94 | 74 | 96 | 99 | 97 | | | 100 | | |
| cM capacity (veh/h) | 43 | 59 | 260 | 41 | 59 | 364 | 636 | | | 791 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|-------|------|------|------|------|
| Volume Total | 19 | 16 | 22 | 845 | 2 | 1096 |
| Volume Left | 3 | 11 | 22 | 0 | 2 | 0 |
| Volume Right | 16 | 3 | 0 | 5 | 0 | 8 |
| cSH | 141 | 52 | 636 | 1700 | 791 | 1700 |
| Volume to Capacity | 0.13 | 0.30 | 0.03 | 0.50 | 0.00 | 0.64 |
| Queue Length 95th (ft) | 11 | 26 | 3 | 0 | 0 | 0 |
| Control Delay (s) | 34.5 | 101.1 | 10.9 | 0.0 | 9.6 | 0.0 |
| Lane LOS | D | F | B | | A | |
| Approach Delay (s) | 34.5 | 101.1 | 0.3 | | 0.0 | |
| Approach LOS | D | F | | | | |

Intersection Summary

| | | | | | | |
|-----------------------------------|--|-------|--|----------------------|--|---|
| Average Delay | | 1.3 | | | | |
| Intersection Capacity Utilization | | 65.2% | | ICU Level of Service | | C |
| Analysis Period (min) | | 15 | | | | |

HCM Unsignalized Intersection Capacity Analysis

8: SR-1 & Vermont Ave

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↕ | | | ↕ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 15 | 3 | 18 | 35 | 2 | 5 | 3 | 793 | 24 | 3 | 1033 | 24 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 16 | 3 | 19 | 37 | 2 | 5 | 3 | 835 | 25 | 3 | 1087 | 25 |
| Pedestrians | | | | | 1 | | | 1 | | | | |
| Lane Width (ft) | | | | | 12.0 | | | 12.0 | | | | |
| Walking Speed (ft/s) | | | | | 4.0 | | | 4.0 | | | | |
| Percent Blockage | | | | | 0 | | | 0 | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1954 | 1974 | 1101 | 1970 | 1974 | 848 | 1113 | | | 861 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1954 | 1974 | 1101 | 1970 | 1974 | 848 | 1113 | | | 861 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 66 | 95 | 93 | 11 | 97 | 99 | 99 | | | 100 | | |
| cM capacity (veh/h) | 46 | 62 | 258 | 41 | 62 | 361 | 628 | | | 780 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|-------|------|------|------|------|
| Volume Total | 38 | 44 | 3 | 860 | 3 | 1113 |
| Volume Left | 16 | 37 | 3 | 0 | 3 | 0 |
| Volume Right | 19 | 5 | 0 | 25 | 0 | 25 |
| cSH | 81 | 47 | 628 | 1700 | 780 | 1700 |
| Volume to Capacity | 0.47 | 0.94 | 0.01 | 0.51 | 0.00 | 0.65 |
| Queue Length 95th (ft) | 49 | 97 | 0 | 0 | 0 | 0 |
| Control Delay (s) | 84.0 | 248.5 | 10.8 | 0.0 | 9.6 | 0.0 |
| Lane LOS | F | F | B | | A | |
| Approach Delay (s) | 84.0 | 248.5 | 0.0 | | 0.0 | |
| Approach LOS | F | F | | | | |

| Intersection Summary | | |
|-----------------------------------|-------|----------------------|
| Average Delay | | 6.9 |
| Intersection Capacity Utilization | 66.8% | ICU Level of Service |
| Analysis Period (min) | | 15 |
| | | C |

HCM Unsignalized Intersection Capacity Analysis

9: SR-1 & Cypress Ave

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↕ | | | ↕ | | ↕ | ↕ | | ↕ | ↕ | |
| Volume (veh/h) | 77 | 2 | 23 | 23 | 3 | 9 | 20 | 756 | 9 | 10 | 1058 | 49 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 84 | 2 | 25 | 25 | 3 | 10 | 22 | 822 | 10 | 11 | 1150 | 53 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage veh | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2075 | 2073 | 1177 | 2068 | 2095 | 827 | 1203 | | | 832 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2075 | 2073 | 1177 | 2068 | 2095 | 827 | 1203 | | | 832 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 0 | 96 | 89 | 25 | 93 | 97 | 96 | | | 99 | | |
| cM capacity (veh/h) | 35 | 51 | 233 | 33 | 50 | 372 | 580 | | | 801 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|-------|-------|------|------|------|------|
| Volume Total | 111 | 38 | 22 | 832 | 11 | 1203 |
| Volume Left | 84 | 25 | 22 | 0 | 11 | 0 |
| Volume Right | 25 | 10 | 0 | 10 | 0 | 53 |
| cSH | 44 | 45 | 580 | 1700 | 801 | 1700 |
| Volume to Capacity | 2.54 | 0.85 | 0.04 | 0.49 | 0.01 | 0.71 |
| Queue Length 95th (ft) | 297 | 84 | 3 | 0 | 1 | 0 |
| Control Delay (s) | 894.7 | 228.6 | 11.4 | 0.0 | 9.6 | 0.0 |
| Lane LOS | F | F | B | | A | |
| Approach Delay (s) | 894.7 | 228.6 | 0.3 | | 0.1 | |
| Approach LOS | F | F | | | | |

| Intersection Summary | | |
|-----------------------------------|-------|------------------------|
| Average Delay | | 48.8 |
| Intersection Capacity Utilization | 72.4% | ICU Level of Service C |
| Analysis Period (min) | | 15 |

HCM Unsignalized Intersection Capacity Analysis

10: SR-1 & St Etheldore St

9/23/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 13 | 0 | 727 | 0 | 18 | 1016 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Hourly flow rate (vph) | 14 | 0 | 799 | 0 | 20 | 1116 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1955 | 799 | | | 799 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1955 | 799 | | | 799 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 79 | 100 | | | 98 | |
| cM capacity (veh/h) | 69 | 386 | | | 824 | |

| Direction, Lane # | WB 1 | NB 1 | SB 1 |
|------------------------|------|------|------|
| Volume Total | 14 | 799 | 1136 |
| Volume Left | 14 | 0 | 20 |
| Volume Right | 0 | 0 | 0 |
| cSH | 69 | 1700 | 824 |
| Volume to Capacity | 0.21 | 0.47 | 0.02 |
| Queue Length 95th (ft) | 18 | 0 | 2 |
| Control Delay (s) | 70.8 | 0.0 | 0.8 |
| Lane LOS | F | | A |
| Approach Delay (s) | 70.8 | 0.0 | 0.8 |
| Approach LOS | F | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|----------------------|
| Average Delay | | 1.0 | |
| Intersection Capacity Utilization | | 77.9% | ICU Level of Service |
| Analysis Period (min) | | 15 | D |

HCM Unsignalized Intersection Capacity Analysis

11: SR-1 & Capistrano Rd

9/23/2014



| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 16 | 2 | 6 | 518 | 519 | 32 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 17 | 2 | 7 | 563 | 564 | 35 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 859 | 564 | 564 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 859 | 564 | 564 | | | |
| tC, single (s) | 6.8 | 6.9 | 4.1 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | |
| p0 queue free % | 94 | 100 | 99 | | | |
| cM capacity (veh/h) | 294 | 469 | 1004 | | | |

| Direction, Lane # | EB 1 | EB 2 | NB 1 | NB 2 | NB 3 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|------|
| Volume Total | 17 | 2 | 7 | 282 | 282 | 564 | 35 |
| Volume Left | 17 | 0 | 7 | 0 | 0 | 0 | 0 |
| Volume Right | 0 | 2 | 0 | 0 | 0 | 0 | 35 |
| cSH | 294 | 469 | 1004 | 1700 | 1700 | 1700 | 1700 |
| Volume to Capacity | 0.06 | 0.00 | 0.01 | 0.17 | 0.17 | 0.33 | 0.02 |
| Queue Length 95th (ft) | 5 | 0 | 0 | 0 | 0 | 0 | 0 |
| Control Delay (s) | 18.0 | 12.7 | 8.6 | 0.0 | 0.0 | 0.0 | 0.0 |
| Lane LOS | C | B | A | | | | |
| Approach Delay (s) | 17.4 | | 0.1 | | | 0.0 | |
| Approach LOS | C | | | | | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|------------------------|
| Average Delay | | 0.3 | |
| Intersection Capacity Utilization | | 37.3% | ICU Level of Service A |
| Analysis Period (min) | | 15 | |

HCM Unsignalized Intersection Capacity Analysis

12: Coral Reef Ave & SR-1

9/23/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|-----------------------------------|------|------|-------|------|----------------------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 51 | 55 | 595 | 25 | 25 | 962 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 55 | 59 | 640 | 27 | 27 | 1034 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1741 | 653 | | | 667 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1741 | 653 | | | 667 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 41 | 87 | | | 97 | |
| cM capacity (veh/h) | 93 | 467 | | | 923 | |
| Direction, Lane # | WB 1 | WB 2 | NB 1 | SB 1 | SB 2 | |
| Volume Total | 55 | 59 | 667 | 27 | 1034 | |
| Volume Left | 55 | 0 | 0 | 27 | 0 | |
| Volume Right | 0 | 59 | 27 | 0 | 0 | |
| cSH | 93 | 467 | 1700 | 923 | 1700 | |
| Volume to Capacity | 0.59 | 0.13 | 0.39 | 0.03 | 0.61 | |
| Queue Length 95th (ft) | 69 | 11 | 0 | 2 | 0 | |
| Control Delay (s) | 89.1 | 13.8 | 0.0 | 9.0 | 0.0 | |
| Lane LOS | F | B | | A | | |
| Approach Delay (s) | 50.0 | | 0.0 | 0.2 | | |
| Approach LOS | F | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 3.2 | | | |
| Intersection Capacity Utilization | | | 60.6% | | ICU Level of Service | B |
| Analysis Period (min) | | | 15 | | | |

HCM Signalized Intersection Capacity Analysis

13: Capistrano Rd

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|------|------|------|------|-------|------|------|------|------|-------|-------|------|
| Lane Configurations | | ↕ | ↗ | | ↕ | | ↗ | ↕ | | ↗ | ↕ | ↗ |
| Volume (vph) | 12 | 138 | 181 | 122 | 109 | 196 | 158 | 445 | 48 | 126 | 763 | 46 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 3.5 | 3.5 | | 3.5 | | 3.0 | 6.5 | | 3.0 | 6.5 | 6.5 |
| Lane Util. Factor | | 1.00 | 1.00 | | 1.00 | | 0.97 | 0.95 | | 1.00 | 0.95 | 1.00 |
| Fr _t | | 1.00 | 0.85 | | 0.94 | | 1.00 | 0.99 | | 1.00 | 1.00 | 0.85 |
| Fl _t Protected | | 1.00 | 1.00 | | 0.99 | | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | | 1855 | 1583 | | 1723 | | 3433 | 3488 | | 1770 | 3539 | 1583 |
| Fl _t Permitted | | 0.96 | 1.00 | | 0.86 | | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | | 1791 | 1583 | | 1499 | | 3433 | 3488 | | 1770 | 3539 | 1583 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 13 | 150 | 197 | 133 | 118 | 213 | 172 | 484 | 52 | 137 | 829 | 50 |
| RTOR Reduction (vph) | 0 | 0 | 119 | 0 | 19 | 0 | 0 | 9 | 0 | 0 | 0 | 34 |
| Lane Group Flow (vph) | 0 | 163 | 78 | 0 | 445 | 0 | 172 | 527 | 0 | 137 | 829 | 16 |
| Turn Type | Perm | NA | Perm | | NA | | Prot | NA | | Prot | NA | Perm |
| Protected Phases | | 8 | | | 4 | | 1 | 6 | | 5 | 2 | |
| Permitted Phases | 8 | | 8 | | | | | | | | | 2 |
| Actuated Green, G (s) | | 30.3 | 30.3 | | 30.3 | | 8.4 | 23.1 | | 10.5 | 25.2 | 25.2 |
| Effective Green, g (s) | | 30.3 | 30.3 | | 30.3 | | 8.4 | 23.1 | | 10.5 | 25.2 | 25.2 |
| Actuated g/C Ratio | | 0.39 | 0.39 | | 0.39 | | 0.11 | 0.30 | | 0.14 | 0.33 | 0.33 |
| Clearance Time (s) | | 3.5 | 3.5 | | 3.5 | | 3.0 | 6.5 | | 3.0 | 6.5 | 6.5 |
| Vehicle Extension (s) | | 2.0 | 2.0 | | 2.0 | | 2.0 | 4.0 | | 2.0 | 4.0 | 4.0 |
| Lane Grp Cap (vph) | | 705 | 623 | | 590 | | 374 | 1047 | | 241 | 1159 | 518 |
| v/s Ratio Prot | | | | | | | 0.05 | 0.15 | | c0.08 | c0.23 | |
| v/s Ratio Perm | | 0.09 | 0.05 | | c0.30 | | | | | | | 0.01 |
| v/c Ratio | | 0.23 | 0.12 | | 0.75 | | 0.46 | 0.50 | | 0.57 | 0.72 | 0.03 |
| Uniform Delay, d ₁ | | 15.5 | 14.8 | | 20.1 | | 32.1 | 22.2 | | 31.1 | 22.7 | 17.6 |
| Progression Factor | | 1.00 | 1.00 | | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d ₂ | | 0.1 | 0.0 | | 4.9 | | 0.3 | 0.5 | | 1.8 | 2.3 | 0.0 |
| Delay (s) | | 15.6 | 14.9 | | 24.9 | | 32.4 | 22.7 | | 32.9 | 25.0 | 17.6 |
| Level of Service | | B | B | | C | | C | C | | C | C | B |
| Approach Delay (s) | | 15.2 | | | 24.9 | | 25.1 | | | 25.7 | | |
| Approach LOS | | B | | | C | | C | | | C | | |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 23.9 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.72 | | |
| Actuated Cycle Length (s) | 76.9 | Sum of lost time (s) | 13.0 |
| Intersection Capacity Utilization | 73.9% | ICU Level of Service | D |
| Analysis Period (min) | 15 | | |
| c Critical Lane Group | | | |

HCM Signalized Intersection Capacity Analysis

14: SR-1 & Coronado St

9/23/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|-------|------|------|------|------|-------|
| Lane Configurations | | | | | | |
| Volume (vph) | 489 | 19 | 649 | 225 | 55 | 1138 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 3.0 | 3.0 | 5.5 | 5.5 | 3.0 | 5.5 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frbp, ped/bikes | 1.00 | 0.97 | 1.00 | 1.00 | 1.00 | 1.00 |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 0.85 | 1.00 | 0.85 | 1.00 | 1.00 |
| Flt Protected | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (prot) | 1770 | 1543 | 1863 | 1583 | 1770 | 1863 |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (perm) | 1770 | 1543 | 1863 | 1583 | 1770 | 1863 |
| Peak-hour factor, PHF | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Adj. Flow (vph) | 537 | 21 | 713 | 247 | 60 | 1251 |
| RTOR Reduction (vph) | 0 | 12 | 0 | 118 | 0 | 0 |
| Lane Group Flow (vph) | 537 | 9 | 713 | 129 | 60 | 1251 |
| Confl. Peds. (#/hr) | | 3 | | | | |
| Turn Type | Prot | Perm | NA | Perm | Prot | NA |
| Protected Phases | 4 | | 6 | | 5 | 2 |
| Permitted Phases | | 4 | | 6 | | |
| Actuated Green, G (s) | 30.1 | 30.1 | 53.1 | 53.1 | 7.3 | 63.4 |
| Effective Green, g (s) | 30.1 | 30.1 | 53.1 | 53.1 | 7.3 | 63.4 |
| Actuated g/C Ratio | 0.30 | 0.30 | 0.52 | 0.52 | 0.07 | 0.62 |
| Clearance Time (s) | 3.0 | 3.0 | 5.5 | 5.5 | 3.0 | 5.5 |
| Vehicle Extension (s) | 2.0 | 2.0 | 2.4 | 2.4 | 2.5 | 2.4 |
| Lane Grp Cap (vph) | 522 | 455 | 969 | 824 | 126 | 1157 |
| v/s Ratio Prot | c0.30 | | 0.38 | | 0.03 | c0.67 |
| v/s Ratio Perm | | 0.01 | | 0.08 | | |
| v/c Ratio | 1.03 | 0.02 | 0.74 | 0.16 | 0.48 | 1.08 |
| Uniform Delay, d1 | 36.0 | 25.5 | 19.0 | 12.8 | 45.5 | 19.3 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 46.9 | 0.0 | 2.7 | 0.1 | 2.1 | 51.3 |
| Delay (s) | 82.9 | 25.5 | 21.7 | 12.8 | 47.6 | 70.6 |
| Level of Service | F | C | C | B | D | E |
| Approach Delay (s) | 80.7 | | 19.4 | | | 69.6 |
| Approach LOS | F | | B | | | E |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 54.8 | HCM 2000 Level of Service | D |
| HCM 2000 Volume to Capacity ratio | 1.10 | | |
| Actuated Cycle Length (s) | 102.0 | Sum of lost time (s) | 11.5 |
| Intersection Capacity Utilization | 94.9% | ICU Level of Service | F |
| Analysis Period (min) | 15 | | |

c Critical Lane Group

HCM Unsignalized Intersection Capacity Analysis

15: Coronado St & Obispo Rd

9/23/2014



| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | | ↔ | |
| Volume (veh/h) | 38 | 255 | 401 | 0 | 0 | 123 |
| Sign Control | | Free | Free | | Stop | |
| Grade | | 0% | 0% | | 0% | |
| Peak Hour Factor | 0.76 | 0.76 | 0.76 | 0.76 | 0.76 | 0.76 |
| Hourly flow rate (vph) | 50 | 336 | 528 | 0 | 0 | 162 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | None | None | | | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | 151 | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 528 | | | | 963 | 528 |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 528 | | | | 963 | 528 |
| tC, single (s) | 4.1 | | | | 6.4 | 6.2 |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 2.2 | | | | 3.5 | 3.3 |
| p0 queue free % | 95 | | | | 100 | 71 |
| cM capacity (veh/h) | 1039 | | | | 270 | 551 |

| Direction, Lane # | EB 1 | WB 1 | SB 1 |
|------------------------|------|------|------|
| Volume Total | 386 | 528 | 162 |
| Volume Left | 50 | 0 | 0 |
| Volume Right | 0 | 0 | 162 |
| cSH | 1039 | 1700 | 551 |
| Volume to Capacity | 0.05 | 0.31 | 0.29 |
| Queue Length 95th (ft) | 4 | 0 | 30 |
| Control Delay (s) | 1.6 | 0.0 | 14.2 |
| Lane LOS | A | | B |
| Approach Delay (s) | 1.6 | 0.0 | 14.2 |
| Approach LOS | | | B |

| Intersection Summary | | | |
|-----------------------------------|--|-------|----------------------|
| Average Delay | | 2.7 | |
| Intersection Capacity Utilization | | 54.2% | ICU Level of Service |
| Analysis Period (min) | | 15 | A |

HCM Unsignalized Intersection Capacity Analysis
 16: SR-1 & Magellan Ave

9/23/2014



| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 8 | 3 | 9 | 934 | 1561 | 22 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Hourly flow rate (vph) | 9 | 3 | 10 | 1038 | 1734 | 24 |
| Pedestrians | 1 | | | | | |
| Lane Width (ft) | 12.0 | | | | | |
| Walking Speed (ft/s) | 4.0 | | | | | |
| Percent Blockage | 0 | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 2805 | 1748 | 1760 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 2805 | 1748 | 1760 | | | |
| tC, single (s) | 6.4 | 6.2 | 4.1 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | |
| p0 queue free % | 55 | 97 | 97 | | | |
| cM capacity (veh/h) | 20 | 107 | 355 | | | |

| Direction, Lane # | EB 1 | NB 1 | NB 2 | SB 1 |
|------------------------|-------|------|------|------|
| Volume Total | 12 | 10 | 1038 | 1759 |
| Volume Left | 9 | 10 | 0 | 0 |
| Volume Right | 3 | 0 | 0 | 24 |
| cSH | 25 | 355 | 1700 | 1700 |
| Volume to Capacity | 0.49 | 0.03 | 0.61 | 1.03 |
| Queue Length 95th (ft) | 37 | 2 | 0 | 0 |
| Control Delay (s) | 243.7 | 15.4 | 0.0 | 0.0 |
| Lane LOS | F | C | | |
| Approach Delay (s) | 243.7 | 0.1 | | 0.0 |
| Approach LOS | F | | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|------------------------|
| Average Delay | | 1.1 | |
| Intersection Capacity Utilization | | 93.5% | ICU Level of Service F |
| Analysis Period (min) | | 15 | |

HCM Unsignalized Intersection Capacity Analysis

17: SR-1 & Medio Ave

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↕ | | | ↕ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 3 | 0 | 34 | 28 | 0 | 14 | 12 | 867 | 11 | 12 | 1587 | 6 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Hourly flow rate (vph) | 3 | 0 | 37 | 31 | 0 | 15 | 13 | 953 | 12 | 13 | 1744 | 7 |
| Pedestrians | | | | | | | | | | | | 1 |
| Lane Width (ft) | | | | | | | | | | | | 12.0 |
| Walking Speed (ft/s) | | | | | | | | | | | | 4.0 |
| Percent Blockage | | | | | | | | | | | | 0 |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | | None |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2769 | 2765 | 1747 | 2793 | 2762 | 960 | 1751 | | | 965 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2769 | 2765 | 1747 | 2793 | 2762 | 960 | 1751 | | | 965 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 71 | 100 | 65 | 0 | 100 | 95 | 96 | | | 98 | | |
| cM capacity (veh/h) | 11 | 18 | 107 | 7 | 18 | 311 | 358 | | | 714 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|-------|------|------|------|------|------|
| Volume Total | 41 | 46 | 13 | 965 | 13 | 1751 |
| Volume Left | 3 | 31 | 13 | 0 | 13 | 0 |
| Volume Right | 37 | 15 | 0 | 12 | 0 | 7 |
| cSH | 63 | 11 | 358 | 1700 | 714 | 1700 |
| Volume to Capacity | 0.64 | 4.19 | 0.04 | 0.57 | 0.02 | 1.03 |
| Queue Length 95th (ft) | 68 | Err | 3 | 0 | 1 | 0 |
| Control Delay (s) | 132.7 | Err | 15.4 | 0.0 | 10.1 | 0.0 |
| Lane LOS | F | F | C | | B | |
| Approach Delay (s) | 132.7 | Err | 0.2 | | 0.1 | |
| Approach LOS | F | F | | | | |

| Intersection Summary | | |
|-----------------------------------|-------|----------------------|
| Average Delay | | 165.2 |
| Intersection Capacity Utilization | 99.8% | ICU Level of Service |
| Analysis Period (min) | | 15 |
| | | F |

HCM Unsignalized Intersection Capacity Analysis

18: SR-1 & Miramar Dr

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↕ | | | ↕ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 0 | 0 | 7 | 0 | 0 | 11 | 1 | 895 | 0 | 3 | 1645 | 7 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Hourly flow rate (vph) | 0 | 0 | 8 | 0 | 0 | 12 | 1 | 984 | 0 | 3 | 1808 | 8 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2816 | 2804 | 1812 | 2808 | 2808 | 984 | 1815 | | | 984 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2816 | 2804 | 1812 | 2808 | 2808 | 984 | 1815 | | | 984 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 92 | 100 | 100 | 96 | 100 | | | 100 | | |
| cM capacity (veh/h) | 11 | 18 | 98 | 11 | 18 | 302 | 338 | | | 702 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 8 | 12 | 1 | 984 | 3 | 1815 |
| Volume Left | 0 | 0 | 1 | 0 | 3 | 0 |
| Volume Right | 8 | 12 | 0 | 0 | 0 | 8 |
| cSH | 98 | 302 | 338 | 1700 | 702 | 1700 |
| Volume to Capacity | 0.08 | 0.04 | 0.00 | 0.58 | 0.00 | 1.07 |
| Queue Length 95th (ft) | 6 | 3 | 0 | 0 | 0 | 0 |
| Control Delay (s) | 45.0 | 17.4 | 15.7 | 0.0 | 10.2 | 0.0 |
| Lane LOS | E | C | C | | B | |
| Approach Delay (s) | 45.0 | 17.4 | 0.0 | | 0.0 | |
| Approach LOS | E | C | | | | |

| Intersection Summary | | |
|-----------------------------------|-------|----------------------|
| Average Delay | | 0.2 |
| Intersection Capacity Utilization | 97.0% | ICU Level of Service |
| Analysis Period (min) | | 15 |
| | | F |

HCM Unsignalized Intersection Capacity Analysis

1: SR-1 & 2nd St

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------------|-------------|-------------|-------------|----------------------|-------------|------|------|------|------|------|------|
| Lane Configurations | | ↕ | | | ↕ | | ↕ | ↕ | | ↕ | ↕ | |
| Volume (veh/h) | 0 | 0 | 0 | 20 | 0 | 121 | 0 | 865 | 11 | 78 | 1360 | 0 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 21 | 0 | 129 | 0 | 920 | 12 | 83 | 1447 | 0 |
| Pedestrians | | | | | | | | | | | | 9 |
| Lane Width (ft) | | | | | | | | | | | | 12.0 |
| Walking Speed (ft/s) | | | | | | | | | | | | 4.0 |
| Percent Blockage | | | | | | | | | | | | 1 |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | | None |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2671 | 2545 | 1447 | 2539 | 2539 | 935 | 1447 | | | 932 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2671 | 2545 | 1447 | 2539 | 2539 | 935 | 1447 | | | 932 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 100 | 0 | 100 | 60 | 100 | | | 89 | | |
| cM capacity (veh/h) | 8 | 24 | 161 | 17 | 24 | 319 | 468 | | | 734 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 0 | 150 | 0 | 932 | 83 | 1447 | | | | | | |
| Volume Left | 0 | 21 | 0 | 0 | 83 | 0 | | | | | | |
| Volume Right | 0 | 129 | 0 | 12 | 0 | 0 | | | | | | |
| cSH | 1700 | 89 | 1700 | 1700 | 734 | 1700 | | | | | | |
| Volume to Capacity | 0.00 | 1.68 | 0.00 | 0.55 | 0.11 | 0.85 | | | | | | |
| Queue Length 95th (ft) | 0 | 305 | 0 | 0 | 10 | 0 | | | | | | |
| Control Delay (s) | 0.0 | 429.3 | 0.0 | 0.0 | 10.5 | 0.0 | | | | | | |
| Lane LOS | A | F | | | B | | | | | | | |
| Approach Delay (s) | 0.0 | 429.3 | 0.0 | | 0.6 | | | | | | | |
| Approach LOS | A | F | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | | 25.0 | | | | | | | | |
| Intersection Capacity Utilization | | | 88.7% | | ICU Level of Service | | | | | E | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

2: SR-1 & 7th St

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | | ↔ | ↔ | ↔ | | | ↔ | |
| Volume (veh/h) | 0 | 0 | 0 | 0 | 0 | 61 | 0 | 863 | 31 | 13 | 1344 | 0 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 0 | 0 | 63 | 0 | 890 | 32 | 13 | 1386 | 0 |
| Pedestrians | | 1 | | | 4 | | | | | | | |
| Lane Width (ft) | | 12.0 | | | 12.0 | | | | | | | |
| Walking Speed (ft/s) | | 4.0 | | | 4.0 | | | | | | | |
| Percent Blockage | | 0 | | | 0 | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2366 | 2339 | 1387 | 2322 | 2323 | 910 | 1387 | | | 926 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2366 | 2339 | 1387 | 2322 | 2323 | 910 | 1387 | | | 926 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 100 | 100 | 100 | 81 | 100 | | | 98 | | |
| cM capacity (veh/h) | 19 | 36 | 175 | 26 | 37 | 332 | 493 | | | 736 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 |
|------------------------|------|------|------|------|------|
| Volume Total | 0 | 63 | 0 | 922 | 1399 |
| Volume Left | 0 | 0 | 0 | 0 | 13 |
| Volume Right | 0 | 63 | 0 | 32 | 0 |
| cSH | 1700 | 332 | 1700 | 1700 | 736 |
| Volume to Capacity | 0.00 | 0.19 | 0.00 | 0.54 | 0.02 |
| Queue Length 95th (ft) | 0 | 17 | 0 | 0 | 1 |
| Control Delay (s) | 0.0 | 18.4 | 0.0 | 0.0 | 1.1 |
| Lane LOS | A | C | | | A |
| Approach Delay (s) | 0.0 | 18.4 | 0.0 | | 1.1 |
| Approach LOS | A | C | | | |

| Intersection Summary | | |
|-----------------------------------|-------|----------------------|
| Average Delay | | 1.1 |
| Intersection Capacity Utilization | 84.4% | ICU Level of Service |
| Analysis Period (min) | | 15 |
| | | E |

HCM Unsignalized Intersection Capacity Analysis

3: SR-1 & 8th St

9/23/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 74 | 19 | 885 | 16 | 21 | 1325 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Hourly flow rate (vph) | 76 | 20 | 912 | 16 | 22 | 1366 |
| Pedestrians | 4 | | | | | 1 |
| Lane Width (ft) | 12.0 | | | | | 12.0 |
| Walking Speed (ft/s) | 4.0 | | | | | 4.0 |
| Percent Blockage | 0 | | | | | 0 |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 2334 | 926 | | | 933 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 2334 | 926 | | | 933 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 0 | 94 | | | 97 | |
| cM capacity (veh/h) | 39 | 325 | | | 731 | |

| Direction, Lane # | WB 1 | NB 1 | SB 1 |
|------------------------|-------|------|------|
| Volume Total | 96 | 929 | 1388 |
| Volume Left | 76 | 0 | 22 |
| Volume Right | 20 | 16 | 0 |
| cSH | 48 | 1700 | 731 |
| Volume to Capacity | 2.01 | 0.55 | 0.03 |
| Queue Length 95th (ft) | 243 | 0 | 2 |
| Control Delay (s) | 653.3 | 0.0 | 1.7 |
| Lane LOS | F | | A |
| Approach Delay (s) | 653.3 | 0.0 | 1.7 |
| Approach LOS | F | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|----------------------|
| Average Delay | | 26.9 | |
| Intersection Capacity Utilization | | 98.7% | ICU Level of Service |
| Analysis Period (min) | | 15 | F |

HCM Unsignalized Intersection Capacity Analysis

4: SR-1 & Carlos St

9/23/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 0 | 21 | 909 | 0 | 17 | 1462 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.98 | 0.98 | 0.96 | 0.96 | 0.98 | 0.98 |
| Hourly flow rate (vph) | 0 | 21 | 947 | 0 | 17 | 1492 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 2473 | 947 | | | 947 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 2473 | 947 | | | 947 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 100 | 93 | | | 98 | |
| cM capacity (veh/h) | 32 | 317 | | | 725 | |

| Direction, Lane # | WB 1 | NB 1 | SB 1 | SB 2 |
|------------------------|------|------|------|------|
| Volume Total | 21 | 947 | 17 | 1492 |
| Volume Left | 0 | 0 | 17 | 0 |
| Volume Right | 21 | 0 | 0 | 0 |
| cSH | 317 | 1700 | 725 | 1700 |
| Volume to Capacity | 0.07 | 0.56 | 0.02 | 0.88 |
| Queue Length 95th (ft) | 5 | 0 | 2 | 0 |
| Control Delay (s) | 17.2 | 0.0 | 10.1 | 0.0 |
| Lane LOS | C | | B | |
| Approach Delay (s) | 17.2 | 0.0 | 0.1 | |
| Approach LOS | C | | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|------------------------|
| Average Delay | | 0.2 | |
| Intersection Capacity Utilization | | 86.9% | ICU Level of Service E |
| Analysis Period (min) | | 15 | |

HCM Unsignalized Intersection Capacity Analysis

5: SR-1 & Vallemar St

9/23/2014






















| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 0 | 0 | 0 | 4 | 0 | 33 | 0 | 880 | 3 | 29 | 1444 | 0 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 4 | 0 | 34 | 0 | 898 | 3 | 30 | 1473 | 0 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2464 | 2434 | 1473 | 2432 | 2432 | 899 | 1473 | | | 901 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2464 | 2434 | 1473 | 2432 | 2432 | 899 | 1473 | | | 901 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 100 | 81 | 100 | 90 | 100 | | | 96 | | |
| cM capacity (veh/h) | 18 | 31 | 156 | 21 | 31 | 337 | 457 | | | 754 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 0 | 38 | 0 | 901 | 30 | 1473 |
| Volume Left | 0 | 4 | 0 | 0 | 30 | 0 |
| Volume Right | 0 | 34 | 0 | 3 | 0 | 0 |
| cSH | 1700 | 129 | 1700 | 1700 | 754 | 1700 |
| Volume to Capacity | 0.00 | 0.29 | 0.00 | 0.53 | 0.04 | 0.87 |
| Queue Length 95th (ft) | 0 | 28 | 0 | 0 | 3 | 0 |
| Control Delay (s) | 0.0 | 44.0 | 0.0 | 0.0 | 10.0 | 0.0 |
| Lane LOS | A | E | | | A | |
| Approach Delay (s) | 0.0 | 44.0 | 0.0 | | 0.2 | |
| Approach LOS | A | E | | | | |

| Intersection Summary | | |
|-----------------------------------|-------|------------------------|
| Average Delay | | 0.8 |
| Intersection Capacity Utilization | 86.0% | ICU Level of Service E |
| Analysis Period (min) | | 15 |

HCM Unsignalized Intersection Capacity Analysis
6: SR-1 & California Ave

9/23/2014

| |  |  |  |  |  |  |  |  |  |  |  |  | |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
| Lane Configurations | |  | | |  | |  |  | |  |  |  | |
| Volume (veh/h) | 7 | 0 | 33 | 38 | 1 | 28 | 22 | 829 | 49 | 24 | 1408 | 18 | |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | | |
| Peak Hour Factor | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | |
| Hourly flow rate (vph) | 7 | 0 | 34 | 39 | 1 | 29 | 22 | 846 | 50 | 24 | 1437 | 18 | |
| Pedestrians | | 1 | | | 2 | | | | | | | | |
| Lane Width (ft) | | 12.0 | | | 12.0 | | | | | | | | |
| Walking Speed (ft/s) | | 4.0 | | | 4.0 | | | | | | | | |
| Percent Blockage | | 0 | | | 0 | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | | |
| Median storage (veh) | | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | | |
| vC, conflicting volume | 2416 | 2439 | 1447 | 2437 | 2423 | 873 | 1456 | | | 898 | | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | | |
| vCu, unblocked vol | 2416 | 2439 | 1447 | 2437 | 2423 | 873 | 1456 | | | 898 | | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | | |
| tC, 2 stage (s) | | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | | |
| p0 queue free % | 62 | 100 | 79 | 0 | 97 | 92 | 95 | | | 97 | | | |
| cM capacity (veh/h) | 19 | 29 | 161 | 16 | 30 | 349 | 464 | | | 755 | | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | | |
| Volume Total | 41 | 68 | 22 | 896 | 24 | 1455 | | | | | | | |
| Volume Left | 7 | 39 | 22 | 0 | 24 | 0 | | | | | | | |
| Volume Right | 34 | 29 | 0 | 50 | 0 | 18 | | | | | | | |
| cSH | 69 | 27 | 464 | 1700 | 755 | 1700 | | | | | | | |
| Volume to Capacity | 0.59 | 2.53 | 0.05 | 0.53 | 0.03 | 0.86 | | | | | | | |
| Queue Length 95th (ft) | 63 | 207 | 4 | 0 | 3 | 0 | | | | | | | |
| Control Delay (s) | 113.8 | 1004.6 | 13.2 | 0.0 | 9.9 | 0.0 | | | | | | | |
| Lane LOS | F | F | B | | A | | | | | | | | |
| Approach Delay (s) | 113.8 | 1004.6 | 0.3 | | 0.2 | | | | | | | | |
| Approach LOS | F | F | | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | | |
| Average Delay | | | 29.5 | | | | | | | | | | |
| Intersection Capacity Utilization | | | 91.8% | | ICU Level of Service | | | | | | | | F |
| Analysis Period (min) | | | 15 | | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

7: SR-1 & Virginia Ave

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 9 | 0 | 35 | 18 | 3 | 10 | 23 | 846 | 18 | 13 | 1491 | 15 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Hourly flow rate (vph) | 10 | 0 | 38 | 19 | 3 | 10 | 24 | 872 | 19 | 13 | 1537 | 15 |
| Pedestrians | | 3 | | | | | | | | | | 1 |
| Lane Width (ft) | | 12.0 | | | | | | | | | | 12.0 |
| Walking Speed (ft/s) | | 4.0 | | | | | | | | | | 4.0 |
| Percent Blockage | | 0 | | | | | | | | | | 0 |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | | None |
| Median storage veh | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2507 | 2513 | 1548 | 2530 | 2511 | 882 | 1556 | | | 891 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2507 | 2513 | 1548 | 2530 | 2511 | 882 | 1556 | | | 891 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 40 | 100 | 73 | 0 | 88 | 97 | 94 | | | 98 | | |
| cM capacity (veh/h) | 16 | 26 | 140 | 13 | 26 | 345 | 424 | | | 761 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|-------|-------|------|------|------|------|
| Volume Total | 47 | 32 | 24 | 891 | 13 | 1553 |
| Volume Left | 10 | 19 | 24 | 0 | 13 | 0 |
| Volume Right | 38 | 10 | 0 | 19 | 0 | 15 |
| cSH | 54 | 20 | 424 | 1700 | 761 | 1700 |
| Volume to Capacity | 0.87 | 1.60 | 0.06 | 0.52 | 0.02 | 0.91 |
| Queue Length 95th (ft) | 95 | 107 | 4 | 0 | 1 | 0 |
| Control Delay (s) | 207.2 | 704.9 | 14.0 | 0.0 | 9.8 | 0.0 |
| Lane LOS | F | F | B | | A | |
| Approach Delay (s) | 207.2 | 704.9 | 0.4 | | 0.1 | |
| Approach LOS | F | F | | | | |

Intersection Summary

| | |
|-----------------------------------|-------|
| Average Delay | 12.8 |
| Intersection Capacity Utilization | 90.6% |
| ICU Level of Service | E |
| Analysis Period (min) | 15 |

HCM Unsignalized Intersection Capacity Analysis

8: SR-1 & Vermont Ave

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↕ | | | ↕ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 14 | 3 | 7 | 14 | 5 | 6 | 16 | 868 | 33 | 13 | 1496 | 22 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 |
| Hourly flow rate (vph) | 14 | 3 | 7 | 14 | 5 | 6 | 16 | 886 | 34 | 13 | 1527 | 22 |
| Pedestrians | | | | | 4 | | | 4 | | | 1 | |
| Lane Width (ft) | | | | | 12.0 | | | 12.0 | | | 12.0 | |
| Walking Speed (ft/s) | | | | | 4.0 | | | 4.0 | | | 4.0 | |
| Percent Blockage | | | | | 0 | | | 0 | | | 0 | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2492 | 2520 | 1542 | 2505 | 2515 | 908 | 1549 | | | 923 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2492 | 2520 | 1542 | 2505 | 2515 | 908 | 1549 | | | 923 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 10 | 88 | 95 | 10 | 81 | 98 | 96 | | | 98 | | |
| cM capacity (veh/h) | 16 | 26 | 141 | 16 | 27 | 332 | 428 | | | 737 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|-------|-------|------|------|------|------|
| Volume Total | 24 | 26 | 16 | 919 | 13 | 1549 |
| Volume Left | 14 | 14 | 16 | 0 | 13 | 0 |
| Volume Right | 7 | 6 | 0 | 34 | 0 | 22 |
| cSH | 23 | 23 | 428 | 1700 | 737 | 1700 |
| Volume to Capacity | 1.07 | 1.11 | 0.04 | 0.54 | 0.02 | 0.91 |
| Queue Length 95th (ft) | 78 | 81 | 3 | 0 | 1 | 0 |
| Control Delay (s) | 454.8 | 467.4 | 13.7 | 0.0 | 10.0 | 0.0 |
| Lane LOS | F | F | B | | A | |
| Approach Delay (s) | 454.8 | 467.4 | 0.2 | | 0.1 | |
| Approach LOS | F | F | | | | |

| Intersection Summary | | |
|-----------------------------------|-------|----------------------|
| Average Delay | | 9.2 |
| Intersection Capacity Utilization | 91.3% | ICU Level of Service |
| Analysis Period (min) | | 15 |
| | | F |

HCM Unsignalized Intersection Capacity Analysis

9: SR-1 & Cypress Ave

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↕ | | | ↕ | | ↕ | ↕ | | ↕ | ↕ | |
| Volume (veh/h) | 56 | 2 | 40 | 11 | 4 | 16 | 36 | 859 | 15 | 7 | 1399 | 52 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 61 | 2 | 43 | 12 | 4 | 17 | 39 | 934 | 16 | 8 | 1521 | 57 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2596 | 2592 | 1549 | 2601 | 2612 | 942 | 1577 | | | 950 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2596 | 2592 | 1549 | 2601 | 2612 | 942 | 1577 | | | 950 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 0 | 90 | 69 | 0 | 80 | 95 | 91 | | | 99 | | |
| cM capacity (veh/h) | 12 | 22 | 140 | 10 | 22 | 319 | 417 | | | 723 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|-------|------|------|------|------|
| Volume Total | 107 | 34 | 39 | 950 | 8 | 1577 |
| Volume Left | 61 | 12 | 39 | 0 | 8 | 0 |
| Volume Right | 43 | 17 | 0 | 16 | 0 | 57 |
| cSH | 20 | 23 | 417 | 1700 | 723 | 1700 |
| Volume to Capacity | 5.36 | 1.48 | 0.09 | 0.56 | 0.01 | 0.93 |
| Queue Length 95th (ft) | Err | 108 | 8 | 0 | 1 | 0 |
| Control Delay (s) | Err | 616.2 | 14.5 | 0.0 | 10.0 | 0.0 |
| Lane LOS | F | F | B | | B | |
| Approach Delay (s) | Err | 616.2 | 0.6 | | 0.0 | |
| Approach LOS | F | F | | | | |

Intersection Summary

| | |
|-----------------------------------|-------|
| Average Delay | 400.3 |
| Intersection Capacity Utilization | 92.7% |
| ICU Level of Service | F |
| Analysis Period (min) | 15 |

HCM Unsignalized Intersection Capacity Analysis

10: SR-1 & St Etheldore St

9/23/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 0 | 21 | 871 | 18 | 0 | 1423 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 0 | 23 | 937 | 19 | 0 | 1530 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 2476 | 946 | | | 956 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 2476 | 946 | | | 956 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 100 | 93 | | | 100 | |
| cM capacity (veh/h) | 33 | 317 | | | 719 | |

| Direction, Lane # | WB 1 | NB 1 | SB 1 |
|------------------------|------|------|------|
| Volume Total | 23 | 956 | 1530 |
| Volume Left | 0 | 0 | 0 |
| Volume Right | 23 | 19 | 0 |
| cSH | 317 | 1700 | 719 |
| Volume to Capacity | 0.07 | 0.56 | 0.00 |
| Queue Length 95th (ft) | 6 | 0 | 0 |
| Control Delay (s) | 17.2 | 0.0 | 0.0 |
| Lane LOS | C | | |
| Approach Delay (s) | 17.2 | 0.0 | 0.0 |
| Approach LOS | C | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|------------------------|
| Average Delay | | 0.2 | |
| Intersection Capacity Utilization | | 84.9% | ICU Level of Service E |
| Analysis Period (min) | | 15 | |

HCM Unsignalized Intersection Capacity Analysis

11: SR-1 & Capistrano Rd

9/23/2014



| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 28 | 24 | 16 | 686 | 909 | 63 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Hourly flow rate (vph) | 30 | 26 | 17 | 730 | 967 | 67 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1366 | 967 | 967 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1366 | 967 | 967 | | | |
| tC, single (s) | 6.8 | 6.9 | 4.1 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | |
| p0 queue free % | 78 | 90 | 98 | | | |
| cM capacity (veh/h) | 135 | 254 | 708 | | | |

| Direction, Lane # | EB 1 | EB 2 | NB 1 | NB 2 | NB 3 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|------|
| Volume Total | 30 | 26 | 17 | 365 | 365 | 967 | 67 |
| Volume Left | 30 | 0 | 17 | 0 | 0 | 0 | 0 |
| Volume Right | 0 | 26 | 0 | 0 | 0 | 0 | 67 |
| cSH | 135 | 254 | 708 | 1700 | 1700 | 1700 | 1700 |
| Volume to Capacity | 0.22 | 0.10 | 0.02 | 0.21 | 0.21 | 0.57 | 0.04 |
| Queue Length 95th (ft) | 20 | 8 | 2 | 0 | 0 | 0 | 0 |
| Control Delay (s) | 39.1 | 20.7 | 10.2 | 0.0 | 0.0 | 0.0 | 0.0 |
| Lane LOS | E | C | B | | | | |
| Approach Delay (s) | 30.6 | | 0.2 | 0.0 | | | |
| Approach LOS | D | | | | | | |

| Intersection Summary | | | |
|-----------------------------------|-------|--|----------------------|
| Average Delay | | | 1.0 |
| Intersection Capacity Utilization | 57.8% | | ICU Level of Service |
| Analysis Period (min) | 15 | | B |

HCM Unsignalized Intersection Capacity Analysis
 12: Coral Reef Ave & SR-1

9/23/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 28 | 54 | 809 | 33 | 66 | 1335 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 30 | 58 | 870 | 35 | 71 | 1435 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 2465 | 888 | | | 905 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 2465 | 888 | | | 905 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 0 | 83 | | | 91 | |
| cM capacity (veh/h) | 30 | 343 | | | 751 | |

| Direction, Lane # | WB 1 | WB 2 | NB 1 | SB 1 | SB 2 |
|------------------------|-------|------|------|------|------|
| Volume Total | 30 | 58 | 905 | 71 | 1435 |
| Volume Left | 30 | 0 | 0 | 71 | 0 |
| Volume Right | 0 | 58 | 35 | 0 | 0 |
| cSH | 30 | 343 | 1700 | 751 | 1700 |
| Volume to Capacity | 1.00 | 0.17 | 0.53 | 0.09 | 0.84 |
| Queue Length 95th (ft) | 84 | 15 | 0 | 8 | 0 |
| Control Delay (s) | 354.1 | 17.6 | 0.0 | 10.3 | 0.0 |
| Lane LOS | F | C | | B | |
| Approach Delay (s) | 132.5 | | 0.0 | 0.5 | |
| Approach LOS | F | | | | |

| Intersection Summary | | | | | |
|-----------------------------------|--|--|-------|----------------------|---|
| Average Delay | | | 5.0 | | |
| Intersection Capacity Utilization | | | 80.3% | ICU Level of Service | D |
| Analysis Period (min) | | | 15 | | |

HCM Signalized Intersection Capacity Analysis

13: Capistrano Rd

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|------|-------|------|------|------|------|------|------|------|-------|-------|------|
| Lane Configurations | | ↕ | ↗ | | ↕ | | ↖ | ↕ | | ↖ | ↕ | ↗ |
| Volume (vph) | 113 | 163 | 266 | 25 | 135 | 173 | 357 | 511 | 51 | 338 | 821 | 131 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 3.5 | 3.5 | | 3.5 | | 3.0 | 6.5 | | 3.0 | 6.5 | 6.5 |
| Lane Util. Factor | | 1.00 | 1.00 | | 1.00 | | 0.97 | 0.95 | | 1.00 | 0.95 | 1.00 |
| Fr _t | | 1.00 | 0.85 | | 0.93 | | 1.00 | 0.99 | | 1.00 | 1.00 | 0.85 |
| Fl _t Protected | | 0.98 | 1.00 | | 1.00 | | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | | 1825 | 1583 | | 1726 | | 3433 | 3491 | | 1770 | 3539 | 1583 |
| Fl _t Permitted | | 0.56 | 1.00 | | 0.96 | | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | | 1042 | 1583 | | 1667 | | 3433 | 3491 | | 1770 | 3539 | 1583 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 123 | 177 | 289 | 27 | 147 | 188 | 388 | 555 | 55 | 367 | 892 | 142 |
| RTOR Reduction (vph) | 0 | 0 | 134 | 0 | 39 | 0 | 0 | 8 | 0 | 0 | 0 | 90 |
| Lane Group Flow (vph) | 0 | 300 | 155 | 0 | 323 | 0 | 388 | 602 | 0 | 367 | 892 | 52 |
| Turn Type | Perm | NA | Perm | Perm | NA | | Prot | NA | | Prot | NA | Perm |
| Protected Phases | | 8 | | | 4 | | 1 | 6 | | 5 | 2 | |
| Permitted Phases | 8 | | 8 | 4 | | | | | | | | 2 |
| Actuated Green, G (s) | | 23.5 | 23.5 | | 23.5 | | 13.8 | 21.9 | | 20.8 | 28.9 | 28.9 |
| Effective Green, g (s) | | 23.5 | 23.5 | | 23.5 | | 13.8 | 21.9 | | 20.8 | 28.9 | 28.9 |
| Actuated g/C Ratio | | 0.30 | 0.30 | | 0.30 | | 0.17 | 0.28 | | 0.26 | 0.36 | 0.36 |
| Clearance Time (s) | | 3.5 | 3.5 | | 3.5 | | 3.0 | 6.5 | | 3.0 | 6.5 | 6.5 |
| Vehicle Extension (s) | | 2.0 | 2.0 | | 2.0 | | 2.0 | 4.0 | | 2.0 | 4.0 | 4.0 |
| Lane Grp Cap (vph) | | 309 | 469 | | 494 | | 598 | 965 | | 464 | 1291 | 577 |
| v/s Ratio Prot | | | | | | | 0.11 | 0.17 | | c0.21 | c0.25 | |
| v/s Ratio Perm | | c0.29 | 0.10 | | 0.19 | | | | | | | 0.03 |
| v/c Ratio | | 0.97 | 0.33 | | 0.65 | | 0.65 | 0.62 | | 0.79 | 0.69 | 0.09 |
| Uniform Delay, d ₁ | | 27.5 | 21.7 | | 24.3 | | 30.4 | 25.0 | | 27.2 | 21.4 | 16.5 |
| Progression Factor | | 1.00 | 1.00 | | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d ₂ | | 42.9 | 0.2 | | 2.4 | | 1.8 | 1.4 | | 8.4 | 1.7 | 0.1 |
| Delay (s) | | 70.5 | 21.9 | | 26.7 | | 32.3 | 26.5 | | 35.5 | 23.1 | 16.6 |
| Level of Service | | E | C | | C | | C | C | | D | C | B |
| Approach Delay (s) | | 46.6 | | | 26.7 | | | 28.7 | | | 25.7 | |
| Approach LOS | | D | | | C | | | C | | | C | |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 30.4 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.85 | | |
| Actuated Cycle Length (s) | 79.2 | Sum of lost time (s) | 13.0 |
| Intersection Capacity Utilization | 83.8% | ICU Level of Service | E |
| Analysis Period (min) | 15 | | |
| c Critical Lane Group | | | |

HCM Signalized Intersection Capacity Analysis

14: SR-1 & Coronado St

9/23/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|-------|------|------|------|------|-------|
| Lane Configurations | | | | | | |
| Volume (vph) | 389 | 26 | 945 | 266 | 21 | 1300 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 3.0 | 3.0 | 5.5 | 5.5 | 3.0 | 5.5 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frbp, ped/bikes | 1.00 | 0.94 | 1.00 | 1.00 | 1.00 | 1.00 |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 0.85 | 1.00 | 0.85 | 1.00 | 1.00 |
| Flt Protected | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (prot) | 1770 | 1484 | 1863 | 1583 | 1770 | 1863 |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (perm) | 1770 | 1484 | 1863 | 1583 | 1770 | 1863 |
| Peak-hour factor, PHF | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adj. Flow (vph) | 418 | 28 | 1016 | 286 | 23 | 1398 |
| RTOR Reduction (vph) | 0 | 20 | 0 | 119 | 0 | 0 |
| Lane Group Flow (vph) | 418 | 8 | 1016 | 167 | 23 | 1398 |
| Confl. Peds. (#/hr) | | 23 | | | | |
| Turn Type | Prot | Perm | NA | Perm | Prot | NA |
| Protected Phases | 4 | | 6 | | 5 | 2 |
| Permitted Phases | | 4 | | 6 | | |
| Actuated Green, G (s) | 28.4 | 28.4 | 60.3 | 60.3 | 2.9 | 66.2 |
| Effective Green, g (s) | 28.4 | 28.4 | 60.3 | 60.3 | 2.9 | 66.2 |
| Actuated g/C Ratio | 0.28 | 0.28 | 0.58 | 0.58 | 0.03 | 0.64 |
| Clearance Time (s) | 3.0 | 3.0 | 5.5 | 5.5 | 3.0 | 5.5 |
| Vehicle Extension (s) | 2.0 | 2.0 | 2.4 | 2.4 | 2.5 | 2.4 |
| Lane Grp Cap (vph) | 487 | 408 | 1089 | 925 | 49 | 1196 |
| v/s Ratio Prot | c0.24 | | 0.55 | | 0.01 | c0.75 |
| v/s Ratio Perm | | 0.01 | | 0.11 | | |
| v/c Ratio | 0.86 | 0.02 | 0.93 | 0.18 | 0.47 | 1.17 |
| Uniform Delay, d1 | 35.4 | 27.2 | 19.6 | 9.9 | 49.3 | 18.4 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 13.5 | 0.0 | 13.9 | 0.1 | 5.1 | 85.3 |
| Delay (s) | 48.9 | 27.2 | 33.5 | 10.0 | 54.4 | 103.7 |
| Level of Service | D | C | C | A | D | F |
| Approach Delay (s) | 47.6 | | 28.3 | | | 102.9 |
| Approach LOS | D | | C | | | F |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 64.5 | HCM 2000 Level of Service | E |
| HCM 2000 Volume to Capacity ratio | 1.11 | | |
| Actuated Cycle Length (s) | 103.1 | Sum of lost time (s) | 11.5 |
| Intersection Capacity Utilization | 98.4% | ICU Level of Service | F |
| Analysis Period (min) | 15 | | |

c Critical Lane Group

HCM Unsignalized Intersection Capacity Analysis

15: Coronado St & Obispo Rd

9/23/2014



| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | | ↔ | |
| Volume (veh/h) | 133 | 216 | 360 | 0 | 0 | 211 |
| Sign Control | | Free | Free | | Stop | |
| Grade | | 0% | 0% | | 0% | |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Hourly flow rate (vph) | 139 | 225 | 375 | 0 | 0 | 220 |
| Pedestrians | | | | | 11 | |
| Lane Width (ft) | | | | | 12.0 | |
| Walking Speed (ft/s) | | | | | 4.0 | |
| Percent Blockage | | | | | 1 | |
| Right turn flare (veh) | | | | | | |
| Median type | | None | None | | | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | 151 | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 386 | | | | 888 | 386 |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 386 | | | | 888 | 386 |
| tC, single (s) | 4.1 | | | | 6.4 | 6.2 |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 2.2 | | | | 3.5 | 3.3 |
| p0 queue free % | 88 | | | | 100 | 66 |
| cM capacity (veh/h) | 1162 | | | | 274 | 656 |

| Direction, Lane # | EB 1 | WB 1 | SB 1 |
|------------------------|------|------|------|
| Volume Total | 364 | 375 | 220 |
| Volume Left | 139 | 0 | 0 |
| Volume Right | 0 | 0 | 220 |
| cSH | 1162 | 1700 | 656 |
| Volume to Capacity | 0.12 | 0.22 | 0.34 |
| Queue Length 95th (ft) | 10 | 0 | 37 |
| Control Delay (s) | 4.0 | 0.0 | 13.2 |
| Lane LOS | A | | B |
| Approach Delay (s) | 4.0 | 0.0 | 13.2 |
| Approach LOS | | | B |

| Intersection Summary | | | |
|-----------------------------------|-------|-----|------------------------|
| Average Delay | | 4.5 | |
| Intersection Capacity Utilization | 60.7% | | ICU Level of Service B |
| Analysis Period (min) | 15 | | |

HCM Unsignalized Intersection Capacity Analysis
 16: SR-1 & Magellan Ave

9/23/2014



| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 14 | 22 | 27 | 1205 | 1665 | 25 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Hourly flow rate (vph) | 15 | 24 | 30 | 1324 | 1830 | 27 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 3227 | 1843 | 1857 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 3227 | 1843 | 1857 | | | |
| tC, single (s) | 6.4 | 6.2 | 4.1 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | |
| p0 queue free % | 0 | 74 | 91 | | | |
| cM capacity (veh/h) | 10 | 93 | 325 | | | |

| Direction, Lane # | EB 1 | NB 1 | NB 2 | SB 1 |
|------------------------|-------|------|------|------|
| Volume Total | 40 | 30 | 1324 | 1857 |
| Volume Left | 15 | 30 | 0 | 0 |
| Volume Right | 24 | 0 | 0 | 27 |
| cSH | 21 | 325 | 1700 | 1700 |
| Volume to Capacity | 1.84 | 0.09 | 0.78 | 1.09 |
| Queue Length 95th (ft) | 129 | 7 | 0 | 0 |
| Control Delay (s) | 781.6 | 17.2 | 0.0 | 0.0 |
| Lane LOS | F | C | | |
| Approach Delay (s) | 781.6 | 0.4 | | 0.0 |
| Approach LOS | F | | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|----------------------|
| Average Delay | | 9.7 | |
| Intersection Capacity Utilization | | 99.1% | ICU Level of Service |
| Analysis Period (min) | | 15 | F |

HCM Unsignalized Intersection Capacity Analysis

17: SR-1 & Medio Ave

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | ↗ | ↖ | | ↗ | ↖ | |
| Volume (veh/h) | 7 | 0 | 44 | 25 | 0 | 37 | 34 | 1191 | 23 | 30 | 1644 | 16 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Hourly flow rate (vph) | 7 | 0 | 46 | 26 | 0 | 39 | 35 | 1241 | 24 | 31 | 1712 | 17 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 3133 | 3119 | 1721 | 3144 | 3115 | 1253 | 1729 | | | 1265 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 3133 | 3119 | 1721 | 3144 | 3115 | 1253 | 1729 | | | 1265 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 0 | 100 | 59 | 0 | 100 | 82 | 90 | | | 94 | | |
| cM capacity (veh/h) | 5 | 10 | 111 | 3 | 10 | 210 | 365 | | | 550 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|-------|------|------|------|------|------|
| Volume Total | 53 | 65 | 35 | 1265 | 31 | 1729 |
| Volume Left | 7 | 26 | 35 | 0 | 31 | 0 |
| Volume Right | 46 | 39 | 0 | 24 | 0 | 17 |
| cSH | 28 | 8 | 365 | 1700 | 550 | 1700 |
| Volume to Capacity | 1.92 | 7.82 | 0.10 | 0.74 | 0.06 | 1.02 |
| Queue Length 95th (ft) | 158 | Err | 8 | 0 | 5 | 0 |
| Control Delay (s) | 736.4 | Err | 15.9 | 0.0 | 11.9 | 0.0 |
| Lane LOS | F | F | C | | B | |
| Approach Delay (s) | 736.4 | Err | 0.4 | | 0.2 | |
| Approach LOS | F | F | | | | |

Intersection Summary

| | | | | | | |
|-----------------------------------|--|--------|--|----------------------|--|---|
| Average Delay | | 215.8 | | | | |
| Intersection Capacity Utilization | | 102.5% | | ICU Level of Service | | G |
| Analysis Period (min) | | 15 | | | | |

HCM Unsignalized Intersection Capacity Analysis

18: SR-1 & Miramar Dr

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↕ | | | ↕ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 2 | 1 | 16 | 1 | 0 | 6 | 4 | 1268 | 12 | 9 | 1667 | 10 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 2 | 1 | 17 | 1 | 0 | 6 | 4 | 1335 | 13 | 9 | 1755 | 11 |
| Pedestrians | | | | | 1 | | | | | | 1 | |
| Lane Width (ft) | | | | | 12.0 | | | | | | 12.0 | |
| Walking Speed (ft/s) | | | | | 4.0 | | | | | | 4.0 | |
| Percent Blockage | | | | | 0 | | | | | | 0 | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 3129 | 3136 | 1760 | 3142 | 3135 | 1343 | 1765 | | | 1348 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 3129 | 3136 | 1760 | 3142 | 3135 | 1343 | 1765 | | | 1348 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 67 | 90 | 84 | 79 | 100 | 97 | 99 | | | 98 | | |
| cM capacity (veh/h) | 6 | 11 | 105 | 5 | 11 | 185 | 353 | | | 510 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|-------|-------|------|------|------|------|
| Volume Total | 20 | 7 | 4 | 1347 | 9 | 1765 |
| Volume Left | 2 | 1 | 4 | 0 | 9 | 0 |
| Volume Right | 17 | 6 | 0 | 13 | 0 | 11 |
| cSH | 34 | 30 | 353 | 1700 | 510 | 1700 |
| Volume to Capacity | 0.59 | 0.24 | 0.01 | 0.79 | 0.02 | 1.04 |
| Queue Length 95th (ft) | 50 | 19 | 1 | 0 | 1 | 0 |
| Control Delay (s) | 210.9 | 159.7 | 15.3 | 0.0 | 12.2 | 0.0 |
| Lane LOS | F | F | C | | B | |
| Approach Delay (s) | 210.9 | 159.7 | 0.0 | | 0.1 | |
| Approach LOS | F | F | | | | |



















Intersection Summary

| | |
|-----------------------------------|-------|
| Average Delay | 1.8 |
| Intersection Capacity Utilization | 98.7% |
| ICU Level of Service | F |
| Analysis Period (min) | 15 |

HCM Unsignalized Intersection Capacity Analysis

1: SR-1 & 2nd St

9/23/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | |  | |  |  | |  |  | |
| Volume (veh/h) | 0 | 0 | 0 | 14 | 0 | 96 | 0 | 768 | 9 | 154 | 1149 | 0 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 14 | 0 | 98 | 0 | 784 | 9 | 157 | 1172 | 0 |
| Pedestrians | | | | | | | | | | | | 4 |
| Lane Width (ft) | | | | | | | | | | | | 12.0 |
| Walking Speed (ft/s) | | | | | | | | | | | | 4.0 |
| Percent Blockage | | | | | | | | | | | | 0 |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | | None |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2372 | 2280 | 1172 | 2275 | 2275 | 792 | 1172 | | | 793 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2372 | 2280 | 1172 | 2275 | 2275 | 792 | 1172 | | | 793 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 100 | 41 | 100 | 75 | 100 | | | 81 | | |
| cM capacity (veh/h) | 15 | 32 | 234 | 24 | 32 | 388 | 596 | | | 828 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 0 | 112 | 0 | 793 | 157 | 1172 | | | | | | |
| Volume Left | 0 | 14 | 0 | 0 | 157 | 0 | | | | | | |
| Volume Right | 0 | 98 | 0 | 9 | 0 | 0 | | | | | | |
| cSH | 1700 | 133 | 1700 | 1700 | 828 | 1700 | | | | | | |
| Volume to Capacity | 0.00 | 0.84 | 0.00 | 0.47 | 0.19 | 0.69 | | | | | | |
| Queue Length 95th (ft) | 0 | 133 | 0 | 0 | 17 | 0 | | | | | | |
| Control Delay (s) | 0.0 | 103.8 | 0.0 | 0.0 | 10.4 | 0.0 | | | | | | |
| Lane LOS | A | F | | | B | | | | | | | |
| Approach Delay (s) | 0.0 | 103.8 | 0.0 | | 1.2 | | | | | | | |
| Approach LOS | A | F | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | | 5.9 | | | | | | | | |
| Intersection Capacity Utilization | | | 81.7% | | ICU Level of Service | | D | | | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

2: SR-1 & 7th St

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | | ↗ | ↖ | ↖ | | | ↗ | |
| Volume (veh/h) | 0 | 0 | 0 | 0 | 0 | 60 | 0 | 707 | 41 | 0 | 1186 | 0 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 0 | 0 | 64 | 0 | 752 | 44 | 0 | 1262 | 0 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2078 | 2057 | 1262 | 2036 | 2036 | 774 | 1262 | | | 796 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2078 | 2057 | 1262 | 2036 | 2036 | 774 | 1262 | | | 796 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 100 | 100 | 100 | 84 | 100 | | | 100 | | |
| cM capacity (veh/h) | 33 | 55 | 207 | 42 | 57 | 398 | 551 | | | 826 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 |
|------------------------|------|------|------|------|------|
| Volume Total | 0 | 64 | 0 | 796 | 1262 |
| Volume Left | 0 | 0 | 0 | 0 | 0 |
| Volume Right | 0 | 64 | 0 | 44 | 0 |
| cSH | 1700 | 398 | 1700 | 1700 | 1700 |
| Volume to Capacity | 0.00 | 0.16 | 0.00 | 0.47 | 0.74 |
| Queue Length 95th (ft) | 0 | 14 | 0 | 0 | 0 |
| Control Delay (s) | 0.0 | 15.7 | 0.0 | 0.0 | 0.0 |
| Lane LOS | A | C | | | |
| Approach Delay (s) | 0.0 | 15.7 | 0.0 | | 0.0 |
| Approach LOS | A | C | | | |

| Intersection Summary | | |
|-----------------------------------|-------|----------------------|
| Average Delay | | 0.5 |
| Intersection Capacity Utilization | 65.8% | ICU Level of Service |
| Analysis Period (min) | | 15 |
| | | C |

HCM Unsignalized Intersection Capacity Analysis

3: SR-1 & 8th St

9/23/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 104 | 16 | 737 | 16 | 17 | 1165 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 109 | 17 | 776 | 17 | 18 | 1226 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 2046 | 784 | | | 793 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 2046 | 784 | | | 793 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 0 | 96 | | | 98 | |
| cM capacity (veh/h) | 60 | 393 | | | 828 | |

| Direction, Lane # | WB 1 | NB 1 | SB 1 |
|------------------------|-------|------|------|
| Volume Total | 126 | 793 | 1244 |
| Volume Left | 109 | 0 | 18 |
| Volume Right | 17 | 17 | 0 |
| cSH | 68 | 1700 | 828 |
| Volume to Capacity | 1.86 | 0.47 | 0.02 |
| Queue Length 95th (ft) | 286 | 0 | 2 |
| Control Delay (s) | 537.8 | 0.0 | 0.9 |
| Lane LOS | F | | A |
| Approach Delay (s) | 537.8 | 0.0 | 0.9 |
| Approach LOS | F | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|----------------------|
| Average Delay | | 31.9 | |
| Intersection Capacity Utilization | | 88.3% | ICU Level of Service |
| Analysis Period (min) | | 15 | E |

HCM Unsignalized Intersection Capacity Analysis

4: SR-1 & Carlos St

9/23/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 0 | 17 | 862 | 0 | 22 | 1265 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Hourly flow rate (vph) | 0 | 18 | 898 | 0 | 23 | 1318 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 2261 | 898 | | | 898 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 2261 | 898 | | | 898 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 100 | 95 | | | 97 | |
| cM capacity (veh/h) | 44 | 338 | | | 756 | |




















| Direction, Lane # | WB 1 | NB 1 | SB 1 | SB 2 |
|------------------------|------|------|------|------|
| Volume Total | 18 | 898 | 23 | 1318 |
| Volume Left | 0 | 0 | 23 | 0 |
| Volume Right | 18 | 0 | 0 | 0 |
| cSH | 338 | 1700 | 756 | 1700 |
| Volume to Capacity | 0.05 | 0.53 | 0.03 | 0.78 |
| Queue Length 95th (ft) | 4 | 0 | 2 | 0 |
| Control Delay (s) | 16.2 | 0.0 | 9.9 | 0.0 |
| Lane LOS | C | | A | |
| Approach Delay (s) | 16.2 | 0.0 | 0.2 | |
| Approach LOS | C | | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|----------------------|
| Average Delay | | 0.2 | |
| Intersection Capacity Utilization | | 76.6% | ICU Level of Service |
| Analysis Period (min) | | 15 | D |

HCM Unsignalized Intersection Capacity Analysis

5: SR-1 & Vallemar St

9/23/2014

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | |  | |  |  | |  |  |  |
| Volume (veh/h) | 0 | 0 | 0 | 7 | 0 | 21 | 0 | 833 | 4 | 50 | 1239 | 0 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 7 | 0 | 22 | 0 | 877 | 4 | 53 | 1304 | 0 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2308 | 2291 | 1304 | 2288 | 2288 | 879 | 1304 | | | 881 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2308 | 2291 | 1304 | 2288 | 2288 | 879 | 1304 | | | 881 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 100 | 100 | 100 | 72 | 100 | 94 | 100 | | | 93 | | |
| cM capacity (veh/h) | 24 | 37 | 196 | 26 | 37 | 347 | 531 | | | 767 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | | | | | | |
| Volume Total | 0 | 29 | 0 | 881 | 53 | 1304 | | | | | | |
| Volume Left | 0 | 7 | 0 | 0 | 53 | 0 | | | | | | |
| Volume Right | 0 | 22 | 0 | 4 | 0 | 0 | | | | | | |
| cSH | 1700 | 86 | 1700 | 1700 | 767 | 1700 | | | | | | |
| Volume to Capacity | 0.00 | 0.34 | 0.00 | 0.52 | 0.07 | 0.77 | | | | | | |
| Queue Length 95th (ft) | 0 | 33 | 0 | 0 | 6 | 0 | | | | | | |
| Control Delay (s) | 0.0 | 67.7 | 0.0 | 0.0 | 10.0 | 0.0 | | | | | | |
| Lane LOS | A | F | | | B | | | | | | | |
| Approach Delay (s) | 0.0 | 67.7 | 0.0 | | 0.4 | | | | | | | |
| Approach LOS | A | F | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 1.1 | | | | | | | | | |
| Intersection Capacity Utilization | | 75.2% | | ICU Level of Service | | D | | | | | | |
| Analysis Period (min) | | 15 | | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

6: SR-1 & California Ave

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↕ | | | ↕ | | ↕ | ↕ | | ↕ | ↕ | |
| Volume (veh/h) | 5 | 0 | 22 | 52 | 2 | 21 | 20 | 809 | 55 | 20 | 1189 | 11 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 5 | 0 | 23 | 55 | 2 | 22 | 21 | 852 | 58 | 21 | 1252 | 12 |
| Pedestrians | | | | | 4 | | | | | | 7 | |
| Lane Width (ft) | | | | | 12.0 | | | | | | 12.0 | |
| Walking Speed (ft/s) | | | | | 4.0 | | | | | | 4.0 | |
| Percent Blockage | | | | | 0 | | | | | | 1 | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2223 | 2255 | 1257 | 2243 | 2232 | 892 | 1263 | | | 913 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2223 | 2255 | 1257 | 2243 | 2232 | 892 | 1263 | | | 913 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 80 | 100 | 89 | 0 | 95 | 93 | 96 | | | 97 | | |
| cM capacity (veh/h) | 26 | 38 | 209 | 25 | 40 | 338 | 550 | | | 744 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|------|-------|------|------|------|------|
| Volume Total | 28 | 79 | 21 | 909 | 21 | 1263 |
| Volume Left | 5 | 55 | 21 | 0 | 21 | 0 |
| Volume Right | 23 | 22 | 0 | 58 | 0 | 12 |
| cSH | 91 | 34 | 550 | 1700 | 744 | 1700 |
| Volume to Capacity | 0.31 | 2.30 | 0.04 | 0.53 | 0.03 | 0.74 |
| Queue Length 95th (ft) | 30 | 223 | 3 | 0 | 2 | 0 |
| Control Delay (s) | 61.6 | 843.2 | 11.8 | 0.0 | 10.0 | 0.0 |
| Lane LOS | F | F | B | | A | |
| Approach Delay (s) | 61.6 | 843.2 | 0.3 | | 0.2 | |
| Approach LOS | F | F | | | | |

| Intersection Summary | | |
|-----------------------------------|-------|----------------------|
| Average Delay | | 29.6 |
| Intersection Capacity Utilization | 81.5% | ICU Level of Service |
| Analysis Period (min) | | 15 |
| | | D |

HCM Unsignalized Intersection Capacity Analysis

7: SR-1 & Virginia Ave

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 10 | 2 | 20 | 22 | 1 | 11 | 25 | 856 | 27 | 11 | 1219 | 17 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 11 | 2 | 22 | 24 | 1 | 12 | 27 | 920 | 29 | 12 | 1311 | 18 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2330 | 2347 | 1320 | 2346 | 2341 | 935 | 1329 | | | 949 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2330 | 2347 | 1320 | 2346 | 2341 | 935 | 1329 | | | 949 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 53 | 94 | 89 | 0 | 97 | 96 | 95 | | | 98 | | |
| cM capacity (veh/h) | 23 | 34 | 192 | 20 | 34 | 322 | 519 | | | 723 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|-------|-------|------|------|------|------|
| Volume Total | 34 | 37 | 27 | 949 | 12 | 1329 |
| Volume Left | 11 | 24 | 27 | 0 | 12 | 0 |
| Volume Right | 22 | 12 | 0 | 29 | 0 | 18 |
| cSH | 54 | 30 | 519 | 1700 | 723 | 1700 |
| Volume to Capacity | 0.64 | 1.24 | 0.05 | 0.56 | 0.02 | 0.78 |
| Queue Length 95th (ft) | 65 | 104 | 4 | 0 | 1 | 0 |
| Control Delay (s) | 152.5 | 446.9 | 12.3 | 0.0 | 10.1 | 0.0 |
| Lane LOS | F | F | B | | B | |
| Approach Delay (s) | 152.5 | 446.9 | 0.3 | | 0.1 | |
| Approach LOS | F | F | | | | |

| Intersection Summary | | |
|-----------------------------------|-------|----------------------|
| Average Delay | | 9.2 |
| Intersection Capacity Utilization | 75.6% | ICU Level of Service |
| Analysis Period (min) | | 15 |
| | | D |

HCM Unsignalized Intersection Capacity Analysis

8: SR-1 & Vermont Ave

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↕ | | | ↕ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 10 | 2 | 20 | 27 | 2 | 6 | 16 | 891 | 35 | 9 | 1236 | 15 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 11 | 2 | 21 | 28 | 2 | 6 | 17 | 938 | 37 | 9 | 1301 | 16 |
| Pedestrians | | 1 | | | | | | | | | | |
| Lane Width (ft) | | 12.0 | | | | | | | | | | |
| Walking Speed (ft/s) | | 4.0 | | | | | | | | | | |
| Percent Blockage | | 0 | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2308 | 2337 | 1310 | 2332 | 2327 | 956 | 1318 | | | 975 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2308 | 2337 | 1310 | 2332 | 2327 | 956 | 1318 | | | 975 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 57 | 94 | 89 | 0 | 94 | 98 | 97 | | | 99 | | |
| cM capacity (veh/h) | 24 | 35 | 194 | 21 | 35 | 313 | 524 | | | 708 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|-------|-------|------|------|------|------|
| Volume Total | 34 | 37 | 17 | 975 | 9 | 1317 |
| Volume Left | 11 | 28 | 17 | 0 | 9 | 0 |
| Volume Right | 21 | 6 | 0 | 37 | 0 | 16 |
| cSH | 56 | 26 | 524 | 1700 | 708 | 1700 |
| Volume to Capacity | 0.60 | 1.42 | 0.03 | 0.57 | 0.01 | 0.77 |
| Queue Length 95th (ft) | 61 | 112 | 2 | 0 | 1 | 0 |
| Control Delay (s) | 139.8 | 551.6 | 12.1 | 0.0 | 10.2 | 0.0 |
| Lane LOS | F | F | B | | B | |
| Approach Delay (s) | 139.8 | 551.6 | 0.2 | | 0.1 | |
| Approach LOS | F | F | | | | |

| Intersection Summary | | |
|-----------------------------------|-------|----------------------|
| Average Delay | | 10.6 |
| Intersection Capacity Utilization | 76.9% | ICU Level of Service |
| Analysis Period (min) | | 15 |
| | | D |

HCM Unsignalized Intersection Capacity Analysis

9: SR-1 & Cypress Ave

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 32 | 2 | 3 | 67 | 5 | 28 | 7 | 844 | 71 | 42 | 1226 | 13 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 35 | 2 | 3 | 73 | 5 | 30 | 8 | 917 | 77 | 46 | 1333 | 14 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 2397 | 2441 | 1340 | 2399 | 2409 | 956 | 1347 | | | 995 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 2397 | 2441 | 1340 | 2399 | 2409 | 956 | 1347 | | | 995 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 0 | 92 | 98 | 0 | 82 | 90 | 99 | | | 93 | | |
| cM capacity (veh/h) | 17 | 29 | 187 | 20 | 30 | 313 | 511 | | | 696 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|-------|------|------|------|------|------|
| Volume Total | 40 | 109 | 8 | 995 | 46 | 1347 |
| Volume Left | 35 | 73 | 8 | 0 | 46 | 0 |
| Volume Right | 3 | 30 | 0 | 77 | 0 | 14 |
| cSH | 19 | 28 | 511 | 1700 | 696 | 1700 |
| Volume to Capacity | 2.14 | 3.90 | 0.01 | 0.59 | 0.07 | 0.79 |
| Queue Length 95th (ft) | 136 | Err | 1 | 0 | 5 | 0 |
| Control Delay (s) | 954.9 | Err | 12.1 | 0.0 | 10.5 | 0.0 |
| Lane LOS | F | F | B | | B | |
| Approach Delay (s) | 954.9 | Err | 0.1 | | 0.3 | |
| Approach LOS | F | F | | | | |

| Intersection Summary | | |
|-----------------------------------|-------|----------------------|
| Average Delay | | 442.6 |
| Intersection Capacity Utilization | 77.6% | ICU Level of Service |
| Analysis Period (min) | | 15 |
| | | D |

HCM Unsignalized Intersection Capacity Analysis

10: SR-1 & St Etheldore St

9/23/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 22 | 0 | 836 | 27 | 0 | 1176 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 24 | 0 | 899 | 29 | 0 | 1265 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 2178 | 913 | | | 928 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 2178 | 913 | | | 928 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 53 | 100 | | | 100 | |
| cM capacity (veh/h) | 51 | 331 | | | 737 | |

| Direction, Lane # | WB 1 | NB 1 | SB 1 |
|------------------------|-------|------|------|
| Volume Total | 24 | 928 | 1265 |
| Volume Left | 24 | 0 | 0 |
| Volume Right | 0 | 29 | 0 |
| cSH | 51 | 1700 | 737 |
| Volume to Capacity | 0.47 | 0.55 | 0.00 |
| Queue Length 95th (ft) | 43 | 0 | 0 |
| Control Delay (s) | 126.6 | 0.0 | 0.0 |
| Lane LOS | F | | |
| Approach Delay (s) | 126.6 | 0.0 | 0.0 |
| Approach LOS | F | | |

| Intersection Summary | | | |
|-----------------------------------|--|-------|----------------------|
| Average Delay | | 1.4 | |
| Intersection Capacity Utilization | | 71.9% | ICU Level of Service |
| Analysis Period (min) | | 15 | C |

HCM Unsignalized Intersection Capacity Analysis

11: SR-1 & Capistrano Rd

9/23/2014



| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 27 | 15 | 12 | 663 | 676 | 48 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 29 | 16 | 13 | 713 | 727 | 52 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1109 | 727 | 727 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1109 | 727 | 727 | | | |
| tC, single (s) | 6.8 | 6.9 | 4.1 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | |
| p0 queue free % | 86 | 96 | 99 | | | |
| cM capacity (veh/h) | 201 | 366 | 872 | | | |

| Direction, Lane # | EB 1 | EB 2 | NB 1 | NB 2 | NB 3 | SB 1 | SB 2 |
|------------------------|------|------|------|------|------|------|------|
| Volume Total | 29 | 16 | 13 | 356 | 356 | 727 | 52 |
| Volume Left | 29 | 0 | 13 | 0 | 0 | 0 | 0 |
| Volume Right | 0 | 16 | 0 | 0 | 0 | 0 | 52 |
| cSH | 201 | 366 | 872 | 1700 | 1700 | 1700 | 1700 |
| Volume to Capacity | 0.14 | 0.04 | 0.01 | 0.21 | 0.21 | 0.43 | 0.03 |
| Queue Length 95th (ft) | 12 | 3 | 1 | 0 | 0 | 0 | 0 |
| Control Delay (s) | 26.0 | 15.3 | 9.2 | 0.0 | 0.0 | 0.0 | 0.0 |
| Lane LOS | D | C | A | | | | |
| Approach Delay (s) | 22.1 | | 0.2 | | | 0.0 | |
| Approach LOS | C | | | | | | |

| Intersection Summary | | | |
|-----------------------------------|-------|----------------------|---|
| Average Delay | | 0.7 | |
| Intersection Capacity Utilization | 45.6% | ICU Level of Service | A |
| Analysis Period (min) | 15 | | |

HCM Unsignalized Intersection Capacity Analysis

12: Coral Reef Ave & SR-1

9/23/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 29 | 37 | 770 | 59 | 76 | 1078 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Hourly flow rate (vph) | 32 | 41 | 846 | 65 | 84 | 1185 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | None | | | None |
| Median storage veh | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 2230 | 879 | | | 911 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 2230 | 879 | | | 911 | |
| tC, single (s) | 6.4 | 6.2 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 24 | 88 | | | 89 | |
| cM capacity (veh/h) | 42 | 347 | | | 748 | |

| Direction, Lane # | WB 1 | WB 2 | NB 1 | SB 1 | SB 2 |
|------------------------|-------|------|------|------|------|
| Volume Total | 32 | 41 | 911 | 84 | 1185 |
| Volume Left | 32 | 0 | 0 | 84 | 0 |
| Volume Right | 0 | 41 | 65 | 0 | 0 |
| cSH | 42 | 347 | 1700 | 748 | 1700 |
| Volume to Capacity | 0.76 | 0.12 | 0.54 | 0.11 | 0.70 |
| Queue Length 95th (ft) | 72 | 10 | 0 | 9 | 0 |
| Control Delay (s) | 217.2 | 16.8 | 0.0 | 10.4 | 0.0 |
| Lane LOS | F | C | | B | |
| Approach Delay (s) | 104.8 | | 0.0 | 0.7 | |
| Approach LOS | F | | | | |

| Intersection Summary | | | | | |
|-----------------------------------|--|--|-------|----------------------|---|
| Average Delay | | | 3.8 | | |
| Intersection Capacity Utilization | | | 66.7% | ICU Level of Service | C |
| Analysis Period (min) | | | 15 | | |

HCM Signalized Intersection Capacity Analysis

13: Capistrano Rd

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|-------|------|------|------|------|-------|-------|------|
| Lane Configurations | | ↕ | ↗ | | ↕ | | ↗ | ↕ | | ↗ | ↕ | ↗ |
| Volume (vph) | 96 | 107 | 145 | 75 | 118 | 234 | 196 | 548 | 31 | 244 | 989 | 97 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 3.5 | 3.5 | | 3.5 | | 3.0 | 6.5 | | 3.0 | 6.5 | 6.5 |
| Lane Util. Factor | | 1.00 | 1.00 | | 1.00 | | 0.97 | 0.95 | | 1.00 | 0.95 | 1.00 |
| Frt | | 1.00 | 0.85 | | 0.93 | | 1.00 | 0.99 | | 1.00 | 1.00 | 0.85 |
| Flt Protected | | 0.98 | 1.00 | | 0.99 | | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | | 1820 | 1583 | | 1710 | | 3433 | 3511 | | 1770 | 3539 | 1583 |
| Flt Permitted | | 0.57 | 1.00 | | 0.90 | | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | | 1069 | 1583 | | 1558 | | 3433 | 3511 | | 1770 | 3539 | 1583 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 104 | 116 | 158 | 82 | 128 | 254 | 213 | 596 | 34 | 265 | 1075 | 105 |
| RTOR Reduction (vph) | 0 | 0 | 93 | 0 | 41 | 0 | 0 | 4 | 0 | 0 | 0 | 65 |
| Lane Group Flow (vph) | 0 | 220 | 65 | 0 | 423 | 0 | 213 | 626 | 0 | 265 | 1075 | 40 |
| Turn Type | Perm | NA | Perm | Perm | NA | | Prot | NA | | Prot | NA | Perm |
| Protected Phases | | 8 | | | 4 | | 1 | 6 | | 5 | 2 | |
| Permitted Phases | 8 | | 8 | 4 | | | | | | | | 2 |
| Actuated Green, G (s) | | 29.3 | 29.3 | | 29.3 | | 9.7 | 25.5 | | 16.5 | 32.3 | 32.3 |
| Effective Green, g (s) | | 29.3 | 29.3 | | 29.3 | | 9.7 | 25.5 | | 16.5 | 32.3 | 32.3 |
| Actuated g/C Ratio | | 0.35 | 0.35 | | 0.35 | | 0.12 | 0.30 | | 0.20 | 0.38 | 0.38 |
| Clearance Time (s) | | 3.5 | 3.5 | | 3.5 | | 3.0 | 6.5 | | 3.0 | 6.5 | 6.5 |
| Vehicle Extension (s) | | 2.0 | 2.0 | | 2.0 | | 2.0 | 4.0 | | 2.0 | 4.0 | 4.0 |
| Lane Grp Cap (vph) | | 371 | 550 | | 541 | | 395 | 1062 | | 346 | 1355 | 606 |
| v/s Ratio Prot | | | | | | | 0.06 | 0.18 | | c0.15 | c0.30 | |
| v/s Ratio Perm | | 0.21 | 0.04 | | c0.27 | | | | | | | 0.03 |
| v/c Ratio | | 0.59 | 0.12 | | 0.78 | | 0.54 | 0.59 | | 0.77 | 0.79 | 0.07 |
| Uniform Delay, d1 | | 22.6 | 18.7 | | 24.6 | | 35.2 | 25.0 | | 32.1 | 23.0 | 16.5 |
| Progression Factor | | 1.00 | 1.00 | | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | | 1.7 | 0.0 | | 6.7 | | 0.7 | 1.0 | | 8.8 | 3.5 | 0.1 |
| Delay (s) | | 24.3 | 18.8 | | 31.3 | | 35.9 | 26.0 | | 40.9 | 26.5 | 16.5 |
| Level of Service | | C | B | | C | | D | C | | D | C | B |
| Approach Delay (s) | | 22.0 | | | 31.3 | | | 28.5 | | | 28.4 | |
| Approach LOS | | C | | | C | | | C | | | C | |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 28.1 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.81 | | |
| Actuated Cycle Length (s) | 84.3 | Sum of lost time (s) | 13.0 |
| Intersection Capacity Utilization | 76.4% | ICU Level of Service | D |
| Analysis Period (min) | 15 | | |
| c Critical Lane Group | | | |

HCM Signalized Intersection Capacity Analysis

14: SR-1 & Coronado St

9/23/2014



| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
|------------------------|-------|------|------|------|------|-------|
| Lane Configurations | | | | | | |
| Volume (vph) | 285 | 22 | 1020 | 420 | 46 | 1161 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 3.0 | 3.0 | 5.5 | 5.5 | 3.0 | 5.5 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frbp, ped/bikes | 1.00 | 0.94 | 1.00 | 1.00 | 1.00 | 1.00 |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 0.85 | 1.00 | 0.85 | 1.00 | 1.00 |
| Flt Protected | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (prot) | 1770 | 1486 | 1863 | 1583 | 1770 | 1863 |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (perm) | 1770 | 1486 | 1863 | 1583 | 1770 | 1863 |
| Peak-hour factor, PHF | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adj. Flow (vph) | 306 | 24 | 1097 | 452 | 49 | 1248 |
| RTOR Reduction (vph) | 0 | 19 | 0 | 177 | 0 | 0 |
| Lane Group Flow (vph) | 306 | 5 | 1097 | 275 | 49 | 1248 |
| Confl. Peds. (#/hr) | | 23 | | | | |
| Turn Type | Prot | Perm | NA | Perm | Prot | NA |
| Protected Phases | 4 | | 6 | | 5 | 2 |
| Permitted Phases | | 4 | | 6 | | |
| Actuated Green, G (s) | 21.2 | 21.2 | 60.7 | 60.7 | 6.5 | 70.2 |
| Effective Green, g (s) | 21.2 | 21.2 | 60.7 | 60.7 | 6.5 | 70.2 |
| Actuated g/C Ratio | 0.21 | 0.21 | 0.61 | 0.61 | 0.07 | 0.70 |
| Clearance Time (s) | 3.0 | 3.0 | 5.5 | 5.5 | 3.0 | 5.5 |
| Vehicle Extension (s) | 2.0 | 2.0 | 2.4 | 2.4 | 2.5 | 2.4 |
| Lane Grp Cap (vph) | 375 | 315 | 1131 | 961 | 115 | 1309 |
| v/s Ratio Prot | c0.17 | | 0.59 | | 0.03 | c0.67 |
| v/s Ratio Perm | | 0.00 | | 0.17 | | |
| v/c Ratio | 0.82 | 0.02 | 0.97 | 0.29 | 0.43 | 0.95 |
| Uniform Delay, d1 | 37.5 | 31.1 | 18.7 | 9.3 | 44.9 | 13.4 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 12.2 | 0.0 | 19.6 | 0.1 | 1.8 | 15.0 |
| Delay (s) | 49.7 | 31.1 | 38.3 | 9.4 | 46.8 | 28.4 |
| Level of Service | D | C | D | A | D | C |
| Approach Delay (s) | 48.3 | | 29.9 | | | 29.1 |
| Approach LOS | D | | C | | | C |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 31.5 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.95 | | |
| Actuated Cycle Length (s) | 99.9 | Sum of lost time (s) | 11.5 |
| Intersection Capacity Utilization | 88.4% | ICU Level of Service | E |
| Analysis Period (min) | 15 | | |

c Critical Lane Group

HCM Unsignalized Intersection Capacity Analysis

15: Coronado St & Obispo Rd

9/23/2014



| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | ↶ | ↶ | | ↶ | |
| Volume (veh/h) | 133 | 349 | 213 | 0 | 0 | 110 |
| Sign Control | | Free | Free | | Stop | |
| Grade | | 0% | 0% | | 0% | |
| Peak Hour Factor | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 |
| Hourly flow rate (vph) | 149 | 392 | 239 | 0 | 0 | 124 |
| Pedestrians | | | 1 | | 5 | |
| Lane Width (ft) | | | 12.0 | | 12.0 | |
| Walking Speed (ft/s) | | | 4.0 | | 4.0 | |
| Percent Blockage | | | 0 | | 0 | |
| Right turn flare (veh) | | | | | | |
| Median type | | None | None | | | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | 151 | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 244 | | | | 936 | 244 |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 244 | | | | 936 | 244 |
| tC, single (s) | 4.1 | | | | 6.4 | 6.2 |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 2.2 | | | | 3.5 | 3.3 |
| p0 queue free % | 89 | | | | 100 | 84 |
| cM capacity (veh/h) | 1316 | | | | 259 | 791 |

| Direction, Lane # | EB 1 | WB 1 | SB 1 |
|------------------------|------|------|------|
| Volume Total | 542 | 239 | 124 |
| Volume Left | 149 | 0 | 0 |
| Volume Right | 0 | 0 | 124 |
| cSH | 1316 | 1700 | 791 |
| Volume to Capacity | 0.11 | 0.14 | 0.16 |
| Queue Length 95th (ft) | 10 | 0 | 14 |
| Control Delay (s) | 3.1 | 0.0 | 10.4 |
| Lane LOS | A | | B |
| Approach Delay (s) | 3.1 | 0.0 | 10.4 |
| Approach LOS | | | B |

| Intersection Summary | | | |
|-----------------------------------|-------|-----|------------------------|
| Average Delay | | 3.3 | |
| Intersection Capacity Utilization | 54.1% | | ICU Level of Service A |
| Analysis Period (min) | | 15 | |

HCM Unsignalized Intersection Capacity Analysis
 16: SR-1 & Magellan Ave

9/23/2014



| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | | | | | | |
| Volume (veh/h) | 14 | 15 | 22 | 1430 | 1422 | 33 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 15 | 16 | 23 | 1505 | 1497 | 35 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 3066 | 1514 | 1532 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 3066 | 1514 | 1532 | | | |
| tC, single (s) | 6.4 | 6.2 | 4.1 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | |
| p0 queue free % | 0 | 89 | 95 | | | |
| cM capacity (veh/h) | 13 | 147 | 435 | | | |

| Direction, Lane # | EB 1 | NB 1 | NB 2 | SB 1 |
|------------------------|-------|------|------|------|
| Volume Total | 31 | 23 | 1505 | 1532 |
| Volume Left | 15 | 23 | 0 | 0 |
| Volume Right | 16 | 0 | 0 | 35 |
| cSH | 24 | 435 | 1700 | 1700 |
| Volume to Capacity | 1.25 | 0.05 | 0.89 | 0.90 |
| Queue Length 95th (ft) | 95 | 4 | 0 | 0 |
| Control Delay (s) | 501.9 | 13.8 | 0.0 | 0.0 |
| Lane LOS | F | B | | |
| Approach Delay (s) | 501.9 | 0.2 | | 0.0 |
| Approach LOS | F | | | |

| Intersection Summary | | | |
|-----------------------------------|-------|-----|------------------------|
| Average Delay | | 5.1 | |
| Intersection Capacity Utilization | 86.8% | | ICU Level of Service E |
| Analysis Period (min) | 15 | | |

HCM Unsignalized Intersection Capacity Analysis

17: SR-1 & Medio Ave

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↕ | | | ↕ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 3 | 0 | 30 | 11 | 0 | 29 | 22 | 1405 | 35 | 21 | 1402 | 11 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Hourly flow rate (vph) | 3 | 0 | 31 | 11 | 0 | 30 | 23 | 1464 | 36 | 22 | 1460 | 11 |
| Pedestrians | | 1 | | | | | | | | | | 2 |
| Lane Width (ft) | | 12.0 | | | | | | | | | | 12.0 |
| Walking Speed (ft/s) | | 4.0 | | | | | | | | | | 4.0 |
| Percent Blockage | | 0 | | | | | | | | | | 0 |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | | None |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 3052 | 3057 | 1467 | 3063 | 3044 | 1484 | 1473 | | | 1500 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 3052 | 3057 | 1467 | 3063 | 3044 | 1484 | 1473 | | | 1500 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 45 | 100 | 80 | 0 | 100 | 80 | 95 | | | 95 | | |
| cM capacity (veh/h) | 6 | 11 | 157 | 6 | 11 | 153 | 457 | | | 447 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|-------|--------|------|------|------|------|
| Volume Total | 34 | 42 | 23 | 1500 | 22 | 1472 |
| Volume Left | 3 | 11 | 23 | 0 | 22 | 0 |
| Volume Right | 31 | 30 | 0 | 36 | 0 | 11 |
| cSH | 46 | 19 | 457 | 1700 | 447 | 1700 |
| Volume to Capacity | 0.75 | 2.25 | 0.05 | 0.88 | 0.05 | 0.87 |
| Queue Length 95th (ft) | 74 | 141 | 4 | 0 | 4 | 0 |
| Control Delay (s) | 198.9 | 1004.4 | 13.3 | 0.0 | 13.5 | 0.0 |
| Lane LOS | F | F | B | | B | |
| Approach Delay (s) | 198.9 | 1004.4 | 0.2 | | 0.2 | |
| Approach LOS | F | F | | | | |

Intersection Summary

| | |
|-----------------------------------|-------|
| Average Delay | 15.9 |
| Intersection Capacity Utilization | 88.6% |
| ICU Level of Service | E |
| Analysis Period (min) | 15 |

HCM Unsignalized Intersection Capacity Analysis

18: SR-1 & Miramar Dr

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | ↗ | ↘ | | ↗ | ↘ | |
| Volume (veh/h) | 2 | 0 | 2 | 0 | 0 | 6 | 0 | 1510 | 2 | 6 | 1429 | 4 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 2 | 0 | 2 | 0 | 0 | 7 | 0 | 1641 | 2 | 7 | 1553 | 4 |
| Pedestrians | | | | | 1 | | | | | | 1 | |
| Lane Width (ft) | | | | | 12.0 | | | | | | 12.0 | |
| Walking Speed (ft/s) | | | | | 4.0 | | | | | | 4.0 | |
| Percent Blockage | | | | | 0 | | | | | | 0 | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | None | | | None | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 3217 | 3213 | 1555 | 3212 | 3214 | 1644 | 1558 | | | 1644 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 3217 | 3213 | 1555 | 3212 | 3214 | 1644 | 1558 | | | 1644 | | |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 60 | 100 | 98 | 100 | 100 | 95 | 100 | | | 98 | | |
| cM capacity (veh/h) | 5 | 10 | 139 | 6 | 10 | 123 | 425 | | | 393 | | |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 |
|------------------------|-------|------|------|------|------|------|
| Volume Total | 4 | 7 | 0 | 1643 | 7 | 1558 |
| Volume Left | 2 | 0 | 0 | 0 | 7 | 0 |
| Volume Right | 2 | 7 | 0 | 2 | 0 | 4 |
| cSH | 10 | 123 | 1700 | 1700 | 393 | 1700 |
| Volume to Capacity | 0.42 | 0.05 | 0.00 | 0.97 | 0.02 | 0.92 |
| Queue Length 95th (ft) | 24 | 4 | 0 | 0 | 1 | 0 |
| Control Delay (s) | 506.0 | 36.0 | 0.0 | 0.0 | 14.3 | 0.0 |
| Lane LOS | F | E | | | B | |
| Approach Delay (s) | 506.0 | 36.0 | 0.0 | | 0.1 | |
| Approach LOS | F | E | | | | |

| Intersection Summary | | |
|-----------------------------------|-------|------------------------|
| Average Delay | | 0.8 |
| Intersection Capacity Utilization | 89.9% | ICU Level of Service E |
| Analysis Period (min) | | 15 |

SR-92 Existing Conditions Report

HCM Unsignalized Intersection Capacity Analysis

47: Muddy Rd/Ox Mt Landfill Rd & SR-92

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | | | | | | | | | | | |
| Volume (veh/h) | 10 | 1234 | 1 | 2 | 464 | 22 | 2 | 0 | 1 | 12 | 0 | 4 |
| Sign Control | | Free | | | Free | | | Stop | | | Stop | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 11 | 1299 | 1 | 2 | 488 | 23 | 2 | 0 | 1 | 13 | 0 | 4 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | None | | | None | | | | | | | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 512 | | | 1300 | | | 1817 | 1836 | 1299 | 1814 | 1814 | 488 |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 512 | | | 1300 | | | 1817 | 1836 | 1299 | 1814 | 1814 | 488 |
| tC, single (s) | 4.1 | | | 4.1 | | | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 2.2 | | | 2.2 | | | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free % | 99 | | | 100 | | | 96 | 100 | 99 | 79 | 100 | 99 |
| cM capacity (veh/h) | 1054 | | | 533 | | | 59 | 75 | 197 | 60 | 77 | 579 |

| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | SB 1 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 11 | 1300 | 491 | 23 | 3 | 17 |
| Volume Left | 11 | 0 | 2 | 0 | 2 | 13 |
| Volume Right | 0 | 1 | 0 | 23 | 1 | 4 |
| cSH | 1054 | 1700 | 533 | 1700 | 77 | 77 |
| Volume to Capacity | 0.01 | 0.76 | 0.00 | 0.01 | 0.04 | 0.22 |
| Queue Length 95th (ft) | 1 | 0 | 0 | 0 | 3 | 19 |
| Control Delay (s) | 8.5 | 0.0 | 0.1 | 0.0 | 53.7 | 64.7 |
| Lane LOS | A | | A | | F | F |
| Approach Delay (s) | 0.1 | | 0.1 | | 53.7 | 64.7 |
| Approach LOS | | | | | F | F |

Intersection Summary

| | |
|-----------------------------------|-------|
| Average Delay | 0.8 |
| Intersection Capacity Utilization | 81.7% |
| ICU Level of Service | D |
| Analysis Period (min) | 15 |

HCM Unsignalized Intersection Capacity Analysis

48: Skyline Blvd (West) & SR-92

9/18/2014



| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | ↑ | ↗ | ↖ | ↑ | ↘ | ↗ |
| Volume (veh/h) | 1184 | 27 | 58 | 443 | 17 | 40 |
| Sign Control | Free | | | Free | Stop | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 |
| Hourly flow rate (vph) | 1208 | 28 | 59 | 452 | 17 | 41 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | 8 |
| Median type | None | | None | | | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | | | 1208 | | 1779 | 1208 |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | | | 1208 | | 1779 | 1208 |
| tC, single (s) | | | 4.1 | | 6.4 | 6.2 |
| tC, 2 stage (s) | | | | | | |
| tF (s) | | | 2.2 | | 3.5 | 3.3 |
| p0 queue free % | | | 90 | | 79 | 82 |
| cM capacity (veh/h) | | | 577 | | 81 | 223 |

| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 |
|------------------------|------|------|------|------|------|
| Volume Total | 1208 | 28 | 59 | 452 | 58 |
| Volume Left | 0 | 0 | 59 | 0 | 17 |
| Volume Right | 0 | 28 | 0 | 0 | 41 |
| cSH | 1700 | 1700 | 577 | 1700 | 272 |
| Volume to Capacity | 0.71 | 0.02 | 0.10 | 0.27 | 0.21 |
| Queue Length 95th (ft) | 0 | 0 | 9 | 0 | 20 |
| Control Delay (s) | 0.0 | 0.0 | 11.9 | 0.0 | 35.5 |
| Lane LOS | | | B | E | |
| Approach Delay (s) | 0.0 | | 1.4 | | 35.5 |
| Approach LOS | | | | | E |

| Intersection Summary | | | | | |
|-----------------------------------|--|--|-------|----------------------|---|
| Average Delay | | | 1.5 | | |
| Intersection Capacity Utilization | | | 72.3% | ICU Level of Service | C |
| Analysis Period (min) | | | 15 | | |

HCM Signalized Intersection Capacity Analysis

49: SR-92 & SR-35 (East)

9/18/2014



| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
|------------------------|------|-------|------|------|-------|--------|
| Lane Configurations | | | | | | |
| Volume (vph) | 225 | 1067 | 436 | 16 | 45 | 114 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 3.5 | 4.5 | 5.0 | 5.0 | 3.5 | 3.5 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 1.00 | 1.00 | 0.85 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (prot) | 1770 | 1863 | 1863 | 1583 | 1770 | 1583 |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (perm) | 1770 | 1863 | 1863 | 1583 | 1770 | 1583 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 245 | 1160 | 474 | 17 | 49 | 124 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 10 | 0 | 76 |
| Lane Group Flow (vph) | 245 | 1160 | 474 | 7 | 49 | 48 |
| Turn Type | Prot | NA | NA | Perm | NA | custom |
| Protected Phases | 1 | 6 | 2 | | 8 | 8 |
| Permitted Phases | | | | 2 | | 1 |
| Actuated Green, G (s) | 15.5 | 41.3 | 21.8 | 21.8 | 5.9 | 21.4 |
| Effective Green, g (s) | 15.5 | 41.3 | 21.8 | 21.8 | 5.9 | 21.4 |
| Actuated g/C Ratio | 0.28 | 0.75 | 0.39 | 0.39 | 0.11 | 0.39 |
| Clearance Time (s) | 3.5 | 4.5 | 5.0 | 5.0 | 3.5 | 3.5 |
| Vehicle Extension (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lane Grp Cap (vph) | 497 | 1393 | 735 | 625 | 189 | 714 |
| v/s Ratio Prot | 0.14 | c0.62 | 0.25 | | c0.03 | 0.01 |
| v/s Ratio Perm | | | | 0.00 | | 0.02 |
| v/c Ratio | 0.49 | 0.83 | 0.64 | 0.01 | 0.26 | 0.07 |
| Uniform Delay, d1 | 16.6 | 4.6 | 13.6 | 10.1 | 22.6 | 10.6 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 0.3 | 4.2 | 1.5 | 0.0 | 0.3 | 0.0 |
| Delay (s) | 16.9 | 8.9 | 15.0 | 10.2 | 22.9 | 10.6 |
| Level of Service | B | A | B | B | C | B |
| Approach Delay (s) | | 10.3 | 14.9 | | 14.1 | |
| Approach LOS | | B | B | | B | |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 11.7 | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | 0.83 | | |
| Actuated Cycle Length (s) | 55.2 | Sum of lost time (s) | 12.0 |
| Intersection Capacity Utilization | 69.1% | ICU Level of Service | C |
| Analysis Period (min) | 15 | | |
| c Critical Lane Group | | | |

HCM Unsignalized Intersection Capacity Analysis

47: Muddy Rd/Ox Mt Landfill Rd & SR-92

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | | | | | | | | | | | |
| Volume (veh/h) | 4 | 837 | 4 | 1 | 1047 | 5 | 0 | 0 | 2 | 2 | 0 | 12 |
| Sign Control | | Free | | | Free | | | Stop | | | Stop | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 4 | 910 | 4 | 1 | 1138 | 5 | 0 | 0 | 2 | 2 | 0 | 13 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | None | | | None | | | | | | | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1143 | | | 914 | | | 2074 | 2066 | 912 | 2061 | 2063 | 1138 |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1143 | | | 914 | | | 2074 | 2066 | 912 | 2061 | 2063 | 1138 |
| tC, single (s) | 4.1 | | | 4.1 | | | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 2.2 | | | 2.2 | | | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free % | 99 | | | 100 | | | 100 | 100 | 99 | 95 | 100 | 95 |
| cM capacity (veh/h) | 611 | | | 746 | | | 37 | 54 | 332 | 40 | 54 | 245 |

| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | SB 1 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 4 | 914 | 1139 | 5 | 2 | 15 |
| Volume Left | 4 | 0 | 1 | 0 | 0 | 2 |
| Volume Right | 0 | 4 | 0 | 5 | 2 | 13 |
| cSH | 611 | 1700 | 746 | 1700 | 332 | 141 |
| Volume to Capacity | 0.01 | 0.54 | 0.00 | 0.00 | 0.01 | 0.11 |
| Queue Length 95th (ft) | 1 | 0 | 0 | 0 | 0 | 9 |
| Control Delay (s) | 10.9 | 0.0 | 0.1 | 0.0 | 15.9 | 33.5 |
| Lane LOS | B | | A | | C | D |
| Approach Delay (s) | 0.1 | | 0.1 | | 15.9 | 33.5 |
| Approach LOS | | | | | C | D |

Intersection Summary

| | |
|-----------------------------------|-------|
| Average Delay | 0.3 |
| Intersection Capacity Utilization | 65.9% |
| ICU Level of Service | C |
| Analysis Period (min) | 15 |

HCM Unsignalized Intersection Capacity Analysis

48: Skyline Blvd (West) & SR-92

9/18/2014



| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | ↑ | ↑ | ↑ | ↑ | ↑ | ↑ |
| Volume (veh/h) | 723 | 25 | 158 | 1186 | 53 | 185 |
| Sign Control | Free | | | Free | Stop | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 786 | 27 | 172 | 1289 | 58 | 201 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | 8 |
| Median type | None | | | None | | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | | | 786 | 2418 | | 786 |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | | | 786 | 2418 | | 786 |
| tC, single (s) | | | 4.1 | 6.4 | | 6.2 |
| tC, 2 stage (s) | | | | | | |
| tF (s) | | | 2.2 | 3.5 | | 3.3 |
| p0 queue free % | | | 79 | 0 | | 49 |
| cM capacity (veh/h) | | | 833 | 28 | | 392 |

| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 |
|------------------------|------|------|------|-------|-------|
| Volume Total | 786 | 27 | 172 | 1289 | 259 |
| Volume Left | 0 | 0 | 172 | 0 | 58 |
| Volume Right | 0 | 27 | 0 | 0 | 201 |
| cSH | 1700 | 1700 | 833 | 1700 | 118 |
| Volume to Capacity | 0.46 | 0.02 | 0.21 | 0.76 | 2.20 |
| Queue Length 95th (ft) | 0 | 0 | 19 | 0 | 551 |
| Control Delay (s) | 0.0 | 0.0 | 10.4 | 0.0 | 626.9 |
| Lane LOS | B | | | F | |
| Approach Delay (s) | 0.0 | | 1.2 | 626.9 | |
| Approach LOS | F | | | | |

| Intersection Summary | | | | | |
|-----------------------------------|--|--|-------|----------------------|---|
| Average Delay | | | 64.7 | | |
| Intersection Capacity Utilization | | | 72.4% | ICU Level of Service | C |
| Analysis Period (min) | | | 15 | | |

HCM Signalized Intersection Capacity Analysis

49: SR-92 & SR-35 (East)

9/18/2014



| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
|------------------------|------|-------|-------|------|------|--------|
| Lane Configurations | | | | | | |
| Volume (vph) | 99 | 833 | 1112 | 27 | 26 | 259 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 3.5 | 4.5 | 5.0 | 5.0 | 3.5 | 3.5 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frpb, ped/bikes | 1.00 | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 1.00 | 1.00 | 0.85 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (prot) | 1770 | 1863 | 1863 | 1551 | 1770 | 1583 |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (perm) | 1770 | 1863 | 1863 | 1551 | 1770 | 1583 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 108 | 905 | 1209 | 29 | 28 | 282 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 12 | 0 | 25 |
| Lane Group Flow (vph) | 108 | 905 | 1209 | 17 | 28 | 257 |
| Confl. Bikes (#/hr) | | | | 1 | | |
| Turn Type | Prot | NA | NA | Perm | NA | custom |
| Protected Phases | 1 | 6 | 2 | | 8 | 8 |
| Permitted Phases | | | | 2 | | 1 |
| Actuated Green, G (s) | 10.1 | 72.4 | 58.3 | 58.3 | 16.8 | 26.9 |
| Effective Green, g (s) | 10.1 | 72.4 | 58.3 | 58.3 | 16.8 | 26.9 |
| Actuated g/C Ratio | 0.10 | 0.74 | 0.60 | 0.60 | 0.17 | 0.28 |
| Clearance Time (s) | 3.5 | 4.5 | 5.0 | 5.0 | 3.5 | 3.5 |
| Vehicle Extension (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lane Grp Cap (vph) | 183 | 1387 | 1117 | 930 | 305 | 495 |
| v/s Ratio Prot | 0.06 | c0.49 | c0.65 | | 0.02 | c0.09 |
| v/s Ratio Perm | | | | 0.01 | | 0.07 |
| v/c Ratio | 0.59 | 0.65 | 1.08 | 0.02 | 0.09 | 0.52 |
| Uniform Delay, d1 | 41.6 | 6.2 | 19.5 | 7.9 | 33.8 | 29.7 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 3.4 | 0.8 | 52.1 | 0.0 | 0.0 | 0.5 |
| Delay (s) | 44.9 | 7.0 | 71.6 | 7.9 | 33.8 | 30.2 |
| Level of Service | D | A | E | A | C | C |
| Approach Delay (s) | | 11.0 | 70.1 | | 30.5 | |
| Approach LOS | | B | E | | C | |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 41.9 | HCM 2000 Level of Service | D |
| HCM 2000 Volume to Capacity ratio | 0.93 | | |
| Actuated Cycle Length (s) | 97.2 | Sum of lost time (s) | 12.0 |
| Intersection Capacity Utilization | 82.1% | ICU Level of Service | E |
| Analysis Period (min) | 15 | | |

c Critical Lane Group

HCM Unsignalized Intersection Capacity Analysis

47: Muddy Rd/Ox Mt Landfill Rd & SR-92

9/18/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | ↗ | ↘ | | | ↖ | ↗ | | ↕ | | | ↕ | |
| Volume (veh/h) | 1 | 770 | 4 | 3 | 1210 | 3 | 0 | 0 | 2 | 12 | 0 | 13 |
| Sign Control | | Free | | | Free | | | Stop | | | Stop | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 1 | 811 | 4 | 3 | 1274 | 3 | 0 | 0 | 2 | 13 | 0 | 14 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | None | | | None | | | | | | | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1277 | | | 815 | | | 2108 | 2098 | 813 | 2095 | 2097 | 1274 |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1277 | | | 815 | | | 2108 | 2098 | 813 | 2095 | 2097 | 1274 |
| tC, single (s) | 4.1 | | | 4.1 | | | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 2.2 | | | 2.2 | | | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free % | 100 | | | 100 | | | 100 | 100 | 99 | 67 | 100 | 93 |
| cM capacity (veh/h) | 544 | | | 813 | | | 35 | 52 | 379 | 38 | 52 | 204 |

| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | SB 1 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 1 | 815 | 1277 | 3 | 2 | 26 |
| Volume Left | 1 | 0 | 3 | 0 | 0 | 13 |
| Volume Right | 0 | 4 | 0 | 3 | 2 | 14 |
| cSH | 544 | 1700 | 813 | 1700 | 379 | 66 |
| Volume to Capacity | 0.00 | 0.48 | 0.00 | 0.00 | 0.01 | 0.40 |
| Queue Length 95th (ft) | 0 | 0 | 0 | 0 | 0 | 38 |
| Control Delay (s) | 11.6 | 0.0 | 0.2 | 0.0 | 14.6 | 92.6 |
| Lane LOS | B | | A | | B | F |
| Approach Delay (s) | 0.0 | | 0.2 | | 14.6 | 92.6 |
| Approach LOS | | | | | B | F |

Intersection Summary

| | |
|-----------------------------------|-------|
| Average Delay | 1.3 |
| Intersection Capacity Utilization | 80.9% |
| ICU Level of Service | D |
| Analysis Period (min) | 15 |

HCM Unsignalized Intersection Capacity Analysis

48: Skyline Blvd (West) & SR-92

9/18/2014



| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | ↑ | ↗ | ↖ | ↑ | ↘ | ↗ |
| Volume (veh/h) | 770 | 19 | 97 | 1191 | 24 | 97 |
| Sign Control | Free | | | Free | Stop | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Hourly flow rate (vph) | 856 | 21 | 108 | 1323 | 27 | 108 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | 8 |
| Median type | None | | | None | | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | | | 856 | | 2394 | 856 |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | | | 856 | | 2394 | 856 |
| tC, single (s) | | | 4.1 | | 6.4 | 6.2 |
| tC, 2 stage (s) | | | | | | |
| tF (s) | | | 2.2 | | 3.5 | 3.3 |
| p0 queue free % | | | 86 | | 17 | 70 |
| cM capacity (veh/h) | | | 784 | | 32 | 358 |

| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 |
|------------------------|------|------|------|------|------|
| Volume Total | 856 | 21 | 108 | 1323 | 134 |
| Volume Left | 0 | 0 | 108 | 0 | 27 |
| Volume Right | 0 | 21 | 0 | 0 | 108 |
| cSH | 1700 | 1700 | 784 | 1700 | 161 |
| Volume to Capacity | 0.50 | 0.01 | 0.14 | 0.78 | 0.83 |
| Queue Length 95th (ft) | 0 | 0 | 12 | 0 | 141 |
| Control Delay (s) | 0.0 | 0.0 | 10.3 | 0.0 | 72.9 |
| Lane LOS | B | | | F | |
| Approach Delay (s) | 0.0 | | 0.8 | | 72.9 |
| Approach LOS | | | | F | |

| Intersection Summary | | | | | |
|-----------------------------------|--|--|-------|----------------------|---|
| Average Delay | | | 4.5 | | |
| Intersection Capacity Utilization | | | 72.7% | ICU Level of Service | C |
| Analysis Period (min) | | | 15 | | |

HCM Signalized Intersection Capacity Analysis

49: SR-92 & SR-35 (East)

9/18/2014



| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
|------------------------|-------|------|-------|------|------|--------|
| Lane Configurations | | | | | | |
| Volume (vph) | 115 | 744 | 1046 | 24 | 30 | 214 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 3.5 | 4.5 | 5.0 | 5.0 | 3.5 | 3.5 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frpb, ped/bikes | 1.00 | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 1.00 | 1.00 | 0.85 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (prot) | 1770 | 1863 | 1863 | 1551 | 1770 | 1583 |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (perm) | 1770 | 1863 | 1863 | 1551 | 1770 | 1583 |
| Peak-hour factor, PHF | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Adj. Flow (vph) | 120 | 775 | 1090 | 25 | 31 | 223 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 9 | 0 | 37 |
| Lane Group Flow (vph) | 120 | 775 | 1090 | 16 | 31 | 186 |
| Confl. Bikes (#/hr) | | | | 1 | | |
| Turn Type | Prot | NA | NA | Perm | NA | custom |
| Protected Phases | 1 | 6 | 2 | | 8 | 8 |
| Permitted Phases | | | | 2 | | 1 |
| Actuated Green, G (s) | 10.6 | 72.9 | 58.3 | 58.3 | 12.9 | 23.5 |
| Effective Green, g (s) | 10.6 | 72.9 | 58.3 | 58.3 | 12.9 | 23.5 |
| Actuated g/C Ratio | 0.11 | 0.78 | 0.62 | 0.62 | 0.14 | 0.25 |
| Clearance Time (s) | 3.5 | 4.5 | 5.0 | 5.0 | 3.5 | 3.5 |
| Vehicle Extension (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lane Grp Cap (vph) | 200 | 1447 | 1157 | 964 | 243 | 455 |
| v/s Ratio Prot | c0.07 | 0.42 | c0.59 | | 0.02 | c0.06 |
| v/s Ratio Perm | | | | 0.01 | | 0.06 |
| v/c Ratio | 0.60 | 0.54 | 0.94 | 0.02 | 0.13 | 0.41 |
| Uniform Delay, d1 | 39.6 | 4.0 | 16.2 | 6.8 | 35.5 | 29.4 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 3.2 | 0.2 | 14.5 | 0.0 | 0.1 | 0.2 |
| Delay (s) | 42.8 | 4.2 | 30.7 | 6.8 | 35.6 | 29.6 |
| Level of Service | D | A | C | A | D | C |
| Approach Delay (s) | | 9.4 | 30.2 | | 30.3 | |
| Approach LOS | | A | C | | C | |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 22.0 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.82 | | |
| Actuated Cycle Length (s) | 93.8 | Sum of lost time (s) | 12.0 |
| Intersection Capacity Utilization | 78.1% | ICU Level of Service | D |
| Analysis Period (min) | 15 | | |

c Critical Lane Group

SR-92 Buildout Conditions Report

HCM Unsignalized Intersection Capacity Analysis

47: Muddy Rd/Ox Mt Landfill Rd & SR-92

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | | | | | | | | | | | |
| Volume (veh/h) | 2 | 1315 | 0 | 0 | 3 | 774 | 1 | 0 | 2 | 14 | 0 | 1 |
| Sign Control | | Free | | | Free | | | Stop | | | Stop | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 2 | 1384 | 0 | 0 | 3 | 815 | 1 | 0 | 2 | 15 | 0 | 1 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | None | | | None | | | | | | | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 818 | | | 1384 | | | 1393 | 2206 | 1384 | 1394 | 1392 | 3 |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 818 | | | 1384 | | | 1393 | 2206 | 1384 | 1394 | 1392 | 3 |
| tC, single (s) | 4.1 | | | 4.1 | | | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 2.2 | | | 2.2 | | | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free % | 100 | | | 100 | | | 99 | 100 | 99 | 87 | 100 | 100 |
| cM capacity (veh/h) | 810 | | | 495 | | | 119 | 44 | 176 | 117 | 142 | 1081 |

| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | SB 1 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 2 | 1384 | 3 | 815 | 3 | 16 |
| Volume Left | 2 | 0 | 0 | 0 | 1 | 15 |
| Volume Right | 0 | 0 | 0 | 815 | 2 | 1 |
| cSH | 810 | 1700 | 495 | 1700 | 152 | 125 |
| Volume to Capacity | 0.00 | 0.81 | 0.00 | 0.48 | 0.02 | 0.13 |
| Queue Length 95th (ft) | 0 | 0 | 0 | 0 | 2 | 11 |
| Control Delay (s) | 9.5 | 0.0 | 0.0 | 0.0 | 29.3 | 38.0 |
| Lane LOS | A | | | | D | E |
| Approach Delay (s) | 0.0 | | 0.0 | | 29.3 | 38.0 |
| Approach LOS | | | | | D | E |

Intersection Summary

| | |
|-----------------------------------|--------|
| Average Delay | 0.3 |
| Intersection Capacity Utilization | 130.5% |
| ICU Level of Service | H |
| Analysis Period (min) | 15 |

HCM Unsignalized Intersection Capacity Analysis

48: Skyline Blvd (West) & SR-92

9/23/2014



| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | ↑ | ↗ | ↖ | ↑ | ↘ | ↗ |
| Volume (veh/h) | 1205 | 0 | 120 | 340 | 116 | 304 |
| Sign Control | Free | | | Free | Stop | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 |
| Hourly flow rate (vph) | 1230 | 0 | 122 | 347 | 118 | 310 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | 8 |
| Median type | None | | None | | | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | | | 1230 | | 1821 | 1230 |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | | | 1230 | | 1821 | 1230 |
| tC, single (s) | | | 4.1 | | 6.4 | 6.2 |
| tC, 2 stage (s) | | | | | | |
| tF (s) | | | 2.2 | | 3.5 | 3.3 |
| p0 queue free % | | | 78 | | 0 | 0 |
| cM capacity (veh/h) | | | 567 | | 67 | 217 |

| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 |
|------------------------|------|------|------|------|-------|
| Volume Total | 1230 | 0 | 122 | 347 | 429 |
| Volume Left | 0 | 0 | 122 | 0 | 118 |
| Volume Right | 0 | 0 | 0 | 0 | 310 |
| cSH | 1700 | 1700 | 567 | 1700 | 182 |
| Volume to Capacity | 0.72 | 0.00 | 0.22 | 0.20 | 2.36 |
| Queue Length 95th (ft) | 0 | 0 | 20 | 0 | 885 |
| Control Delay (s) | 0.0 | 0.0 | 13.1 | 0.0 | 669.3 |
| Lane LOS | | | B | F | |
| Approach Delay (s) | 0.0 | | 3.4 | | 669.3 |
| Approach LOS | | | | | F |

| Intersection Summary | | | | | | |
|-----------------------------------|--|--|-------|----------------------|---|--|
| Average Delay | | | 135.6 | | | |
| Intersection Capacity Utilization | | | 88.9% | ICU Level of Service | E | |
| Analysis Period (min) | | | 15 | | | |

HCM Signalized Intersection Capacity Analysis

49: SR-92 & SR-35 (East)

9/23/2014



| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
|-----------------------------------|-------|------|------|-------|-------|--------|
| Lane Configurations | | | | | | |
| Volume (vph) | 448 | 1081 | 0 | 1028 | 41 | 111 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 3.5 | 4.5 | | 5.0 | 3.5 | 3.5 |
| Lane Util. Factor | 1.00 | 1.00 | | 1.00 | 1.00 | 1.00 |
| Fr _t | 1.00 | 1.00 | | 0.85 | 1.00 | 0.85 |
| Fl _t Protected | 0.95 | 1.00 | | 1.00 | 0.95 | 1.00 |
| Satd. Flow (prot) | 1770 | 1863 | | 1583 | 1770 | 1583 |
| Fl _t Permitted | 0.95 | 1.00 | | 1.00 | 0.95 | 1.00 |
| Satd. Flow (perm) | 1770 | 1863 | | 1583 | 1770 | 1583 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 487 | 1175 | 0 | 1117 | 45 | 121 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 239 | 0 | 78 |
| Lane Group Flow (vph) | 487 | 1175 | 0 | 878 | 45 | 43 |
| Turn Type | Prot | NA | | Perm | Prot | custom |
| Protected Phases | 1 | 6 | 2 | | 8 | 8 |
| Permitted Phases | | | | 2 | | 1 |
| Actuated Green, G (s) | 30.0 | 92.0 | | 58.0 | 7.9 | 37.9 |
| Effective Green, g (s) | 30.0 | 92.0 | | 58.0 | 7.9 | 37.9 |
| Actuated g/C Ratio | 0.28 | 0.85 | | 0.54 | 0.07 | 0.35 |
| Clearance Time (s) | 3.5 | 4.5 | | 5.0 | 3.5 | 3.5 |
| Vehicle Extension (s) | 2.0 | 2.0 | | 2.0 | 2.0 | 2.0 |
| Lane Grp Cap (vph) | 492 | 1588 | | 850 | 129 | 607 |
| v/s Ratio Prot | c0.28 | 0.63 | | | c0.03 | 0.01 |
| v/s Ratio Perm | | | | c0.55 | | 0.02 |
| v/c Ratio | 0.99 | 0.74 | | 1.03 | 0.35 | 0.07 |
| Uniform Delay, d ₁ | 38.8 | 3.2 | | 25.0 | 47.6 | 23.3 |
| Progression Factor | 1.00 | 1.00 | | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d ₂ | 37.4 | 1.6 | | 39.8 | 0.6 | 0.0 |
| Delay (s) | 76.2 | 4.8 | | 64.7 | 48.2 | 23.3 |
| Level of Service | E | A | | E | D | C |
| Approach Delay (s) | | 25.7 | 64.7 | | 30.0 | |
| Approach LOS | | C | E | | C | |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 40.7 | HCM 2000 Level of Service | D |
| HCM 2000 Volume to Capacity ratio | 0.96 | | |
| Actuated Cycle Length (s) | 107.9 | Sum of lost time (s) | 12.0 |
| Intersection Capacity Utilization | 96.0% | ICU Level of Service | F |
| Analysis Period (min) | 15 | | |
| c Critical Lane Group | | | |

HCM Unsignalized Intersection Capacity Analysis
 47: Muddy Rd/Ox Mt Landfill Rd & SR-92

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | | | | | | | | | | | |
| Volume (veh/h) | 4 | 926 | 4 | 1 | 1350 | 5 | 0 | 0 | 2 | 2 | 0 | 12 |
| Sign Control | | Free | | | Free | | | Stop | | | Stop | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 4 | 1007 | 4 | 1 | 1467 | 5 | 0 | 0 | 2 | 2 | 0 | 13 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | None | | | None | | | | | | | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1473 | | | 1011 | | | 2500 | 2492 | 1009 | 2487 | 2489 | 1467 |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1473 | | | 1011 | | | 2500 | 2492 | 1009 | 2487 | 2489 | 1467 |
| tC, single (s) | 4.1 | | | 4.1 | | | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 2.2 | | | 2.2 | | | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free % | 99 | | | 100 | | | 100 | 100 | 99 | 89 | 100 | 92 |
| cM capacity (veh/h) | 458 | | | 686 | | | 18 | 29 | 292 | 20 | 29 | 157 |

| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | SB 1 |
|------------------------|------|------|------|------|------|------|
| Volume Total | 4 | 1011 | 1468 | 5 | 2 | 15 |
| Volume Left | 4 | 0 | 1 | 0 | 0 | 2 |
| Volume Right | 0 | 4 | 0 | 5 | 2 | 13 |
| cSH | 458 | 1700 | 686 | 1700 | 292 | 78 |
| Volume to Capacity | 0.01 | 0.59 | 0.00 | 0.00 | 0.01 | 0.19 |
| Queue Length 95th (ft) | 1 | 0 | 0 | 0 | 1 | 17 |
| Control Delay (s) | 12.9 | 0.0 | 0.1 | 0.0 | 17.4 | 61.6 |
| Lane LOS | B | | A | | C | F |
| Approach Delay (s) | 0.1 | | 0.1 | | 17.4 | 61.6 |
| Approach LOS | | | | | C | F |

Intersection Summary

| | |
|-----------------------------------|-------|
| Average Delay | 0.5 |
| Intersection Capacity Utilization | 81.8% |
| ICU Level of Service | D |
| Analysis Period (min) | 15 |

HCM Unsignalized Intersection Capacity Analysis

48: Skyline Blvd (West) & SR-92

9/23/2014



| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | ↑ | ↑ | ↑ | ↑ | ↑ | ↑ |
| Volume (veh/h) | 765 | 70 | 482 | 1463 | 134 | 461 |
| Sign Control | Free | | | Free | Stop | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 832 | 76 | 524 | 1590 | 146 | 501 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | 8 |
| Median type | None | | None | | | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | | | 832 | | 3470 | 832 |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | | | 832 | | 3470 | 832 |
| tC, single (s) | | | 4.1 | | 6.4 | 6.2 |
| tC, 2 stage (s) | | | | | | |
| tF (s) | | | 2.2 | | 3.5 | 3.3 |
| p0 queue free % | | | 35 | | 0 | 0 |
| cM capacity (veh/h) | | | 801 | | 3 | 369 |

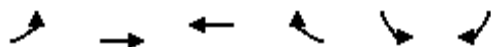
| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 |
|------------------------|------|------|------|------|-------|
| Volume Total | 832 | 76 | 524 | 1590 | 647 |
| Volume Left | 0 | 0 | 524 | 0 | 146 |
| Volume Right | 0 | 76 | 0 | 0 | 501 |
| cSH | 1700 | 1700 | 801 | 1700 | 11 |
| Volume to Capacity | 0.49 | 0.04 | 0.65 | 0.94 | 58.41 |
| Queue Length 95th (ft) | 0 | 0 | 124 | 0 | Err |
| Control Delay (s) | 0.0 | 0.0 | 17.6 | 0.0 | Err |
| Lane LOS | C | | | F | |
| Approach Delay (s) | 0.0 | | 4.4 | | Err |
| Approach LOS | | | | F | |

| Intersection Summary | | | | | |
|-----------------------------------|--|--|--------|----------------------|---|
| Average Delay | | | 1765.3 | | |
| Intersection Capacity Utilization | | | 91.1% | ICU Level of Service | F |
| Analysis Period (min) | | | 15 | | |

HCM Signalized Intersection Capacity Analysis

49: SR-92 & SR-35 (East)

9/23/2014



| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
|------------------------|-------|------|-------|------|------|--------|
| Lane Configurations | | | | | | |
| Volume (vph) | 312 | 865 | 1688 | 91 | 19 | 257 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 3.5 | 4.5 | 5.0 | 5.0 | 3.5 | 3.5 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frbp, ped/bikes | 1.00 | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 1.00 | 1.00 | 0.85 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (prot) | 1770 | 1863 | 1863 | 1551 | 1770 | 1583 |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (perm) | 1770 | 1863 | 1863 | 1551 | 1770 | 1583 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 339 | 940 | 1835 | 99 | 21 | 279 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 10 | 0 | 3 |
| Lane Group Flow (vph) | 339 | 940 | 1835 | 89 | 21 | 276 |
| Confl. Bikes (#/hr) | | | | 1 | | |
| Turn Type | Prot | NA | NA | Perm | Prot | custom |
| Protected Phases | 1 | 6 | 2 | | 8 | 8 |
| Permitted Phases | | | | 2 | | 1 |
| Actuated Green, G (s) | 24.8 | 87.2 | 58.4 | 58.4 | 17.8 | 42.6 |
| Effective Green, g (s) | 24.8 | 87.2 | 58.4 | 58.4 | 17.8 | 42.6 |
| Actuated g/C Ratio | 0.22 | 0.77 | 0.52 | 0.52 | 0.16 | 0.38 |
| Clearance Time (s) | 3.5 | 4.5 | 5.0 | 5.0 | 3.5 | 3.5 |
| Vehicle Extension (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lane Grp Cap (vph) | 388 | 1437 | 962 | 801 | 278 | 645 |
| v/s Ratio Prot | c0.19 | 0.50 | c0.99 | | 0.01 | c0.07 |
| v/s Ratio Perm | | | | 0.06 | | 0.11 |
| v/c Ratio | 0.87 | 0.65 | 1.91 | 0.11 | 0.08 | 0.43 |
| Uniform Delay, d1 | 42.6 | 5.9 | 27.3 | 14.0 | 40.6 | 26.1 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 18.5 | 0.8 | 412.3 | 0.0 | 0.0 | 0.2 |
| Delay (s) | 61.1 | 6.8 | 439.6 | 14.0 | 40.6 | 26.3 |
| Level of Service | E | A | F | B | D | C |
| Approach Delay (s) | | 21.2 | 417.8 | | 27.3 | |
| Approach LOS | | C | F | | C | |

Intersection Summary

| | | | |
|-----------------------------------|--------|---------------------------|------|
| HCM 2000 Control Delay | 240.0 | HCM 2000 Level of Service | F |
| HCM 2000 Volume to Capacity ratio | 1.40 | | |
| Actuated Cycle Length (s) | 113.0 | Sum of lost time (s) | 12.0 |
| Intersection Capacity Utilization | 122.8% | ICU Level of Service | H |
| Analysis Period (min) | 15 | | |

c Critical Lane Group

HCM Unsignalized Intersection Capacity Analysis

47: Muddy Rd/Ox Mt Landfill Rd & SR-92

9/23/2014



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | | | | | | | | | | | |
| Volume (veh/h) | 1 | 861 | 4 | 3 | 1511 | 3 | 0 | 0 | 2 | 11 | 0 | 13 |
| Sign Control | | Free | | | Free | | | Stop | | | Stop | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Hourly flow rate (vph) | 1 | 906 | 4 | 3 | 1591 | 3 | 0 | 0 | 2 | 12 | 0 | 14 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | None | | | None | | | | | | | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 1594 | | | 911 | | | 2521 | 2511 | 908 | 2507 | 2509 | 1591 |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 1594 | | | 911 | | | 2521 | 2511 | 908 | 2507 | 2509 | 1591 |
| tC, single (s) | 4.1 | | | 4.1 | | | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 2.2 | | | 2.2 | | | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free % | 100 | | | 100 | | | 100 | 100 | 99 | 39 | 100 | 90 |
| cM capacity (veh/h) | 411 | | | 748 | | | 17 | 28 | 333 | 19 | 28 | 132 |

| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | SB 1 |
|------------------------|------|------|------|------|------|-------|
| Volume Total | 1 | 911 | 1594 | 3 | 2 | 25 |
| Volume Left | 1 | 0 | 3 | 0 | 0 | 12 |
| Volume Right | 0 | 4 | 0 | 3 | 2 | 14 |
| cSH | 411 | 1700 | 748 | 1700 | 333 | 36 |
| Volume to Capacity | 0.00 | 0.54 | 0.00 | 0.00 | 0.01 | 0.71 |
| Queue Length 95th (ft) | 0 | 0 | 0 | 0 | 0 | 63 |
| Control Delay (s) | 13.8 | 0.0 | 0.7 | 0.0 | 15.9 | 232.9 |
| Lane LOS | B | | A | | C | F |
| Approach Delay (s) | 0.0 | | 0.7 | | 15.9 | 232.9 |
| Approach LOS | | | | | C | F |

Intersection Summary

| | |
|-----------------------------------|-------|
| Average Delay | 2.8 |
| Intersection Capacity Utilization | 96.6% |
| ICU Level of Service | F |
| Analysis Period (min) | 15 |

HCM Unsignalized Intersection Capacity Analysis

48: Skyline Blvd (West) & SR-92

9/23/2014



| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
|------------------------|------|------|------|------|------|------|
| Lane Configurations | ↑ | ↑ | ↑ | ↑ | ↑ | ↑ |
| Volume (veh/h) | 802 | 74 | 411 | 1476 | 97 | 383 |
| Sign Control | Free | | | Free | Stop | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Hourly flow rate (vph) | 891 | 82 | 457 | 1640 | 108 | 426 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | 8 |
| Median type | None | | | None | | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | | | 891 | 3444 | | 891 |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | | | 891 | 3444 | | 891 |
| tC, single (s) | | | 4.1 | 6.4 | | 6.2 |
| tC, 2 stage (s) | | | | | | |
| tF (s) | | | 2.2 | 3.5 | | 3.3 |
| p0 queue free % | | | 40 | 0 | | 0 |
| cM capacity (veh/h) | | | 761 | 3 | | 341 |

| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 |
|------------------------|------|------|------|------|-------|
| Volume Total | 891 | 82 | 457 | 1640 | 533 |
| Volume Left | 0 | 0 | 457 | 0 | 108 |
| Volume Right | 0 | 82 | 0 | 0 | 426 |
| cSH | 1700 | 1700 | 761 | 1700 | 15 |
| Volume to Capacity | 0.52 | 0.05 | 0.60 | 0.96 | 36.38 |
| Queue Length 95th (ft) | 0 | 0 | 102 | 0 | Err |
| Control Delay (s) | 0.0 | 0.0 | 16.6 | 0.0 | Err |
| Lane LOS | C | | | F | |
| Approach Delay (s) | 0.0 | | 3.6 | | Err |
| Approach LOS | | | | | F |

| Intersection Summary | | | | | |
|-----------------------------------|----|--|--------|----------------------|---|
| Average Delay | | | 1482.1 | | |
| Intersection Capacity Utilization | | | 89.7% | ICU Level of Service | E |
| Analysis Period (min) | 15 | | | | |

HCM Signalized Intersection Capacity Analysis

49: SR-92 & SR-35 (East)

9/23/2014



| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
|------------------------|-------|------|-------|------|------|--------|
| Lane Configurations | | | | | | |
| Volume (vph) | 338 | 772 | 1627 | 77 | 22 | 213 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 3.5 | 4.5 | 5.0 | 5.0 | 3.5 | 3.5 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frpb, ped/bikes | 1.00 | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 |
| Flpb, ped/bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 1.00 | 1.00 | 0.85 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (prot) | 1770 | 1863 | 1863 | 1551 | 1770 | 1583 |
| Flt Permitted | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (perm) | 1770 | 1863 | 1863 | 1551 | 1770 | 1583 |
| Peak-hour factor, PHF | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Adj. Flow (vph) | 352 | 804 | 1695 | 80 | 23 | 222 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 8 | 0 | 5 |
| Lane Group Flow (vph) | 352 | 804 | 1695 | 72 | 23 | 217 |
| Confl. Bikes (#/hr) | | | | 1 | | |
| Turn Type | Prot | NA | NA | Perm | Prot | custom |
| Protected Phases | 1 | 6 | 2 | | 8 | 8 |
| Permitted Phases | | | | 2 | | 1 |
| Actuated Green, G (s) | 25.2 | 87.6 | 58.4 | 58.4 | 14.9 | 40.1 |
| Effective Green, g (s) | 25.2 | 87.6 | 58.4 | 58.4 | 14.9 | 40.1 |
| Actuated g/C Ratio | 0.23 | 0.79 | 0.53 | 0.53 | 0.13 | 0.36 |
| Clearance Time (s) | 3.5 | 4.5 | 5.0 | 5.0 | 3.5 | 3.5 |
| Vehicle Extension (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lane Grp Cap (vph) | 403 | 1476 | 984 | 819 | 238 | 624 |
| v/s Ratio Prot | c0.20 | 0.43 | c0.91 | | 0.01 | c0.05 |
| v/s Ratio Perm | | | | 0.05 | | 0.09 |
| v/c Ratio | 0.87 | 0.54 | 1.72 | 0.09 | 0.10 | 0.35 |
| Uniform Delay, d1 | 41.1 | 4.2 | 26.1 | 12.9 | 41.9 | 25.7 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 18.0 | 0.2 | 329.5 | 0.0 | 0.1 | 0.1 |
| Delay (s) | 59.1 | 4.4 | 355.5 | 12.9 | 42.0 | 25.8 |
| Level of Service | E | A | F | B | D | C |
| Approach Delay (s) | | 21.0 | 340.1 | | 27.3 | |
| Approach LOS | | C | F | | C | |

Intersection Summary

| | | | |
|-----------------------------------|--------|---------------------------|------|
| HCM 2000 Control Delay | 199.8 | HCM 2000 Level of Service | F |
| HCM 2000 Volume to Capacity ratio | 1.30 | | |
| Actuated Cycle Length (s) | 110.5 | Sum of lost time (s) | 12.0 |
| Intersection Capacity Utilization | 121.0% | ICU Level of Service | H |
| Analysis Period (min) | 15 | | |

c Critical Lane Group

Mitigated Buildout Analysis

Arterial Level of Service: EB SR-92

| Cross Street | Node | Delay (s/veh) | Travel time (s) | Dist (mi) | Arterial Speed |
|---------------------|------|---------------|-----------------|-----------|----------------|
| | 11 | 0.6 | 8.9 | 0.1 | 28 |
| | 12 | 2.9 | 29.9 | 0.2 | 27 |
| | 13 | 0.9 | 8.1 | 0.1 | 26 |
| | 14 | 4.6 | 37.5 | 0.3 | 26 |
| | 15 | 2.4 | 18.5 | 0.1 | 26 |
| | 16 | 1.2 | 9.4 | 0.1 | 26 |
| | 17 | 1.1 | 8.6 | 0.1 | 26 |
| | 45 | 11.5 | 85.7 | 0.6 | 26 |
| | 19 | 7.0 | 51.1 | 0.4 | 26 |
| | 20 | 1.4 | 10.1 | 0.1 | 26 |
| | 21 | 6.3 | 45.7 | 0.3 | 26 |
| | 22 | 3.7 | 26.4 | 0.2 | 26 |
| | 23 | 2.1 | 15.1 | 0.1 | 26 |
| | 24 | 3.5 | 25.0 | 0.2 | 26 |
| | 25 | 3.7 | 26.1 | 0.2 | 26 |
| | 26 | 3.9 | 27.8 | 0.2 | 26 |
| | 27 | 3.7 | 26.5 | 0.2 | 26 |
| Skyline Blvd (West) | 48 | 4.6 | 15.4 | 0.1 | 30 |
| | 28 | 1.4 | 16.7 | 0.1 | 27 |
| | 29 | 1.2 | 11.7 | 0.1 | 26 |
| | 30 | 2.0 | 17.8 | 0.1 | 26 |
| | 31 | 1.3 | 10.8 | 0.1 | 26 |
| | 32 | 1.0 | 7.7 | 0.1 | 27 |
| | 33 | 3.2 | 25.1 | 0.2 | 26 |
| | 34 | 2.0 | 14.7 | 0.1 | 25 |
| | 35 | 2.6 | 19.5 | 0.1 | 26 |
| | 36 | 2.4 | 18.0 | 0.1 | 26 |
| | 37 | 4.4 | 31.9 | 0.2 | 26 |
| | 38 | 3.9 | 28.0 | 0.2 | 26 |
| | 39 | 3.4 | 24.5 | 0.2 | 26 |
| | 40 | 2.2 | 15.9 | 0.1 | 26 |
| | 46 | 4.9 | 33.9 | 0.2 | 26 |
| SR-35 (East) | 49 | 6.6 | 16.4 | 0.1 | 24 |
| Total | | 107.8 | 768.4 | 5.6 | 26 |

Arterial Level of Service: WB SR-92

| Cross Street | Node | Delay (s/veh) | Travel time (s) | Dist (mi) | Arterial Speed |
|---------------------|------|---------------|-----------------|-----------|----------------|
| | 46 | 3.4 | 16.9 | 0.1 | 24 |
| | 40 | 3.4 | 32.3 | 0.2 | 27 |
| | 39 | 1.9 | 15.7 | 0.1 | 26 |
| | 38 | 2.9 | 23.8 | 0.2 | 27 |
| | 37 | 3.4 | 27.5 | 0.2 | 26 |
| | 36 | 3.9 | 31.4 | 0.2 | 26 |
| | 35 | 2.3 | 18.1 | 0.1 | 26 |
| | 34 | 2.5 | 19.6 | 0.1 | 26 |
| | 33 | 1.8 | 14.2 | 0.1 | 26 |
| | 32 | 3.2 | 25.1 | 0.2 | 26 |
| | 31 | 1.0 | 8.0 | 0.1 | 26 |
| | 30 | 1.4 | 10.7 | 0.1 | 26 |
| | 29 | 2.3 | 17.9 | 0.1 | 26 |
| | 28 | 1.5 | 11.8 | 0.1 | 26 |
| Skyline Blvd (West) | 48 | 2.7 | 10.9 | 0.1 | 42 |
| | 27 | 0.3 | 15.6 | 0.1 | 30 |
| | 26 | 0.9 | 24.0 | 0.2 | 29 |
| | 25 | 1.5 | 25.2 | 0.2 | 28 |
| | 24 | 1.7 | 24.4 | 0.2 | 28 |
| | 23 | 1.8 | 23.1 | 0.2 | 28 |
| | 22 | 1.2 | 14.2 | 0.1 | 28 |
| | 21 | 2.2 | 25.0 | 0.2 | 27 |
| | 20 | 4.1 | 43.7 | 0.3 | 27 |
| | 19 | 1.0 | 9.7 | 0.1 | 27 |
| | 45 | 5.1 | 49.9 | 0.4 | 27 |
| | 17 | 8.9 | 83.1 | 0.6 | 27 |
| | 16 | 1.0 | 8.7 | 0.1 | 26 |
| | 15 | 1.0 | 9.0 | 0.1 | 27 |
| | 14 | 2.1 | 18.3 | 0.1 | 26 |
| | 13 | 4.2 | 37.1 | 0.3 | 27 |
| | 12 | 0.9 | 8.1 | 0.1 | 26 |
| | 11 | 3.7 | 30.6 | 0.2 | 26 |
| Ox Mt Landfill Rd | 47 | 2.4 | 7.1 | 0.1 | 47 |
| Total | | 81.8 | 740.8 | 5.6 | 27 |

Arterial Level of Service: EB SR-92

| Cross Street | Node | Delay (s/veh) | Travel time (s) | Dist (mi) | Arterial Speed |
|---------------------|------|---------------|-----------------|-----------|----------------|
| | 11 | 0.4 | 8.7 | 0.1 | 29 |
| | 12 | 2.2 | 29.2 | 0.2 | 28 |
| | 13 | 0.7 | 7.9 | 0.1 | 27 |
| | 14 | 3.8 | 36.7 | 0.3 | 27 |
| | 15 | 2.0 | 18.2 | 0.1 | 27 |
| | 16 | 1.1 | 9.2 | 0.1 | 26 |
| | 17 | 1.0 | 8.5 | 0.1 | 27 |
| | 45 | 10.3 | 84.6 | 0.6 | 27 |
| | 19 | 6.4 | 50.5 | 0.4 | 27 |
| | 20 | 1.3 | 10.0 | 0.1 | 26 |
| | 21 | 5.9 | 45.3 | 0.3 | 26 |
| | 22 | 3.4 | 26.2 | 0.2 | 26 |
| | 23 | 2.0 | 15.0 | 0.1 | 26 |
| | 24 | 3.3 | 24.8 | 0.2 | 26 |
| | 25 | 3.4 | 25.9 | 0.2 | 26 |
| | 26 | 3.7 | 27.6 | 0.2 | 26 |
| | 27 | 3.5 | 26.2 | 0.2 | 26 |
| Skyline Blvd (West) | 48 | 4.3 | 15.1 | 0.1 | 31 |
| | 28 | 1.2 | 16.4 | 0.1 | 28 |
| | 29 | 1.0 | 11.4 | 0.1 | 27 |
| | 30 | 1.7 | 17.4 | 0.1 | 27 |
| | 31 | 1.2 | 10.6 | 0.1 | 26 |
| | 32 | 0.9 | 7.6 | 0.1 | 27 |
| | 33 | 2.9 | 24.7 | 0.2 | 27 |
| | 34 | 1.8 | 14.6 | 0.1 | 26 |
| | 35 | 2.4 | 19.2 | 0.1 | 27 |
| | 36 | 2.3 | 17.8 | 0.1 | 26 |
| | 37 | 4.1 | 31.6 | 0.2 | 26 |
| | 38 | 3.7 | 27.8 | 0.2 | 26 |
| | 39 | 3.2 | 24.4 | 0.2 | 26 |
| | 40 | 2.1 | 15.7 | 0.1 | 26 |
| | 46 | 4.5 | 33.6 | 0.2 | 26 |
| SR-35 (East) | 49 | 7.3 | 16.9 | 0.1 | 24 |
| Total | | 99.0 | 759.5 | 5.6 | 26 |

Arterial Level of Service: WB SR-92

| Cross Street | Node | Delay (s/veh) | Travel time (s) | Dist (mi) | Arterial Speed |
|---------------------|------|---------------|-----------------|-----------|----------------|
| | 46 | 3.4 | 16.9 | 0.1 | 24 |
| | 40 | 3.3 | 32.4 | 0.2 | 27 |
| | 39 | 1.8 | 15.7 | 0.1 | 26 |
| | 38 | 2.9 | 23.8 | 0.2 | 26 |
| | 37 | 3.5 | 27.6 | 0.2 | 26 |
| | 36 | 4.0 | 31.6 | 0.2 | 26 |
| | 35 | 2.4 | 18.2 | 0.1 | 26 |
| | 34 | 2.6 | 19.7 | 0.1 | 26 |
| | 33 | 1.9 | 14.2 | 0.1 | 26 |
| | 32 | 3.3 | 25.2 | 0.2 | 26 |
| | 31 | 1.1 | 8.0 | 0.1 | 26 |
| | 30 | 1.4 | 10.7 | 0.1 | 26 |
| | 29 | 2.4 | 18.0 | 0.1 | 26 |
| | 28 | 1.6 | 11.9 | 0.1 | 26 |
| Skyline Blvd (West) | 48 | 5.9 | 17.2 | 0.1 | 26 |
| | 27 | 1.0 | 16.3 | 0.1 | 29 |
| | 26 | 2.1 | 25.2 | 0.2 | 27 |
| | 25 | 2.7 | 26.4 | 0.2 | 27 |
| | 24 | 2.9 | 25.5 | 0.2 | 27 |
| | 23 | 2.9 | 24.1 | 0.2 | 27 |
| | 22 | 1.8 | 14.8 | 0.1 | 26 |
| | 21 | 3.2 | 25.9 | 0.2 | 26 |
| | 20 | 5.7 | 45.1 | 0.3 | 26 |
| | 19 | 1.3 | 10.1 | 0.1 | 26 |
| | 45 | 6.7 | 51.3 | 0.4 | 26 |
| | 17 | 11.3 | 85.2 | 0.6 | 26 |
| | 16 | 1.2 | 8.9 | 0.1 | 26 |
| | 15 | 1.2 | 9.2 | 0.1 | 26 |
| | 14 | 2.5 | 18.7 | 0.1 | 26 |
| | 13 | 5.2 | 38.3 | 0.3 | 26 |
| | 12 | 1.1 | 8.3 | 0.1 | 26 |
| | 11 | 4.2 | 31.0 | 0.2 | 26 |
| Ox Mt Landfill Rd | 47 | 2.2 | 8.4 | 0.1 | 30 |
| Total | | 101.0 | 764.0 | 5.6 | 26 |

Arterial Level of Service: EB SR-92

| Cross Street | Node | Delay (s/veh) | Travel time (s) | Dist (mi) | Arterial Speed |
|---------------------|------|---------------|-----------------|-----------|----------------|
| | 11 | 0.4 | 8.7 | 0.1 | 29 |
| | 12 | 2.1 | 29.1 | 0.2 | 28 |
| | 13 | 0.7 | 7.9 | 0.1 | 27 |
| | 14 | 3.7 | 36.6 | 0.3 | 27 |
| | 15 | 2.0 | 18.2 | 0.1 | 27 |
| | 16 | 1.0 | 9.2 | 0.1 | 26 |
| | 17 | 1.0 | 8.5 | 0.1 | 27 |
| | 45 | 10.1 | 84.6 | 0.6 | 27 |
| | 19 | 6.3 | 50.5 | 0.4 | 27 |
| | 20 | 1.3 | 10.0 | 0.1 | 26 |
| | 21 | 5.8 | 45.2 | 0.3 | 26 |
| | 22 | 3.4 | 26.2 | 0.2 | 26 |
| | 23 | 2.0 | 15.0 | 0.1 | 26 |
| | 24 | 3.3 | 24.9 | 0.2 | 26 |
| | 25 | 3.4 | 26.0 | 0.2 | 26 |
| | 26 | 3.7 | 27.6 | 0.2 | 26 |
| | 27 | 3.5 | 26.2 | 0.2 | 26 |
| Skyline Blvd (West) | 48 | 4.7 | 16.0 | 0.1 | 29 |
| | 28 | 1.3 | 16.5 | 0.1 | 28 |
| | 29 | 1.0 | 11.5 | 0.1 | 27 |
| | 30 | 1.9 | 17.6 | 0.1 | 27 |
| | 31 | 1.2 | 10.7 | 0.1 | 26 |
| | 32 | 0.9 | 7.7 | 0.1 | 27 |
| | 33 | 3.1 | 24.9 | 0.2 | 26 |
| | 34 | 1.9 | 14.7 | 0.1 | 26 |
| | 35 | 2.5 | 19.4 | 0.1 | 26 |
| | 36 | 2.3 | 17.9 | 0.1 | 26 |
| | 37 | 4.2 | 31.7 | 0.2 | 26 |
| | 38 | 3.7 | 27.9 | 0.2 | 26 |
| | 39 | 3.3 | 24.4 | 0.2 | 26 |
| | 40 | 2.1 | 15.8 | 0.1 | 26 |
| | 46 | 4.6 | 33.6 | 0.2 | 26 |
| SR-35 (East) | 49 | 5.9 | 15.4 | 0.1 | 26 |
| Total | | 98.0 | 759.9 | 5.6 | 26 |

Arterial Level of Service: WB SR-92

| Cross Street | Node | Delay (s/veh) | Travel time (s) | Dist (mi) | Arterial Speed |
|---------------------|------|---------------|-----------------|-----------|----------------|
| | 46 | 3.4 | 16.9 | 0.1 | 24 |
| | 40 | 3.3 | 32.4 | 0.2 | 27 |
| | 39 | 1.8 | 15.7 | 0.1 | 26 |
| | 38 | 2.9 | 23.8 | 0.2 | 26 |
| | 37 | 3.4 | 27.6 | 0.2 | 26 |
| | 36 | 4.0 | 31.5 | 0.2 | 26 |
| | 35 | 2.3 | 18.1 | 0.1 | 26 |
| | 34 | 2.5 | 19.7 | 0.1 | 26 |
| | 33 | 1.9 | 14.2 | 0.1 | 26 |
| | 32 | 3.3 | 25.2 | 0.2 | 26 |
| | 31 | 1.1 | 8.0 | 0.1 | 26 |
| | 30 | 1.4 | 10.7 | 0.1 | 26 |
| | 29 | 2.4 | 18.0 | 0.1 | 26 |
| | 28 | 1.6 | 11.9 | 0.1 | 26 |
| Skyline Blvd (West) | 48 | 5.7 | 16.9 | 0.1 | 27 |
| | 27 | 1.1 | 16.4 | 0.1 | 28 |
| | 26 | 2.3 | 25.2 | 0.2 | 27 |
| | 25 | 2.8 | 26.4 | 0.2 | 27 |
| | 24 | 3.0 | 25.6 | 0.2 | 26 |
| | 23 | 2.9 | 24.2 | 0.2 | 26 |
| | 22 | 1.9 | 14.9 | 0.1 | 26 |
| | 21 | 3.3 | 26.1 | 0.2 | 26 |
| | 20 | 5.8 | 45.1 | 0.3 | 26 |
| | 19 | 1.3 | 10.1 | 0.1 | 26 |
| | 45 | 6.8 | 51.3 | 0.4 | 26 |
| | 17 | 11.5 | 85.9 | 0.6 | 26 |
| | 16 | 1.2 | 8.9 | 0.1 | 26 |
| | 15 | 1.3 | 9.2 | 0.1 | 26 |
| | 14 | 2.6 | 18.7 | 0.1 | 26 |
| | 13 | 5.2 | 38.2 | 0.3 | 26 |
| | 12 | 1.1 | 8.3 | 0.1 | 26 |
| | 11 | 4.3 | 31.1 | 0.2 | 26 |
| Ox Mt Landfill Rd | 47 | 2.8 | 9.3 | 0.1 | 27 |
| Total | | 102.3 | 765.6 | 5.6 | 26 |

Arterial Level of Service: EB SR-92

| Cross Street | Node | Delay (s/veh) | Travel time (s) | Dist (mi) | Arterial Speed |
|---------------------|------|---------------|-----------------|-----------|----------------|
| | 11 | 0.6 | 8.9 | 0.1 | 29 |
| | 12 | 2.8 | 29.8 | 0.2 | 27 |
| | 13 | 0.9 | 8.1 | 0.1 | 26 |
| | 14 | 4.5 | 37.5 | 0.3 | 27 |
| | 15 | 2.3 | 18.5 | 0.1 | 26 |
| | 16 | 1.2 | 9.4 | 0.1 | 26 |
| | 17 | 1.1 | 8.6 | 0.1 | 26 |
| | 45 | 11.4 | 85.5 | 0.6 | 26 |
| | 19 | 6.9 | 51.2 | 0.4 | 26 |
| | 20 | 1.4 | 10.2 | 0.1 | 26 |
| | 21 | 6.3 | 45.6 | 0.3 | 26 |
| | 22 | 3.7 | 26.4 | 0.2 | 26 |
| | 23 | 2.1 | 15.1 | 0.1 | 26 |
| | 24 | 3.5 | 25.1 | 0.2 | 26 |
| | 25 | 3.6 | 26.1 | 0.2 | 26 |
| | 26 | 3.9 | 27.8 | 0.2 | 26 |
| | 27 | 4.5 | 27.2 | 0.2 | 25 |
| Skyline Blvd (West) | 48 | 16.9 | 27.0 | 0.1 | 17 |
| | 28 | 6.9 | 26.4 | 0.1 | 17 |
| | 29 | 1.2 | 11.7 | 0.1 | 26 |
| | 30 | 1.9 | 17.7 | 0.1 | 27 |
| | 31 | 1.3 | 10.8 | 0.1 | 26 |
| | 32 | 1.0 | 7.7 | 0.1 | 27 |
| | 33 | 3.3 | 25.1 | 0.2 | 26 |
| | 34 | 2.0 | 14.7 | 0.1 | 25 |
| | 35 | 2.6 | 19.5 | 0.1 | 26 |
| | 36 | 2.4 | 18.0 | 0.1 | 26 |
| | 37 | 4.4 | 31.9 | 0.2 | 26 |
| | 38 | 3.9 | 28.1 | 0.2 | 26 |
| | 39 | 3.4 | 24.5 | 0.2 | 26 |
| | 40 | 2.2 | 15.9 | 0.1 | 26 |
| | 46 | 4.8 | 33.8 | 0.2 | 26 |
| SR-35 (East) | 49 | 8.4 | 17.7 | 0.1 | 23 |
| Total | | 127.2 | 791.9 | 5.6 | 25 |

Arterial Level of Service: WB SR-92

| Cross Street | Node | Delay (s/veh) | Travel time (s) | Dist (mi) | Arterial Speed |
|---------------------|------|---------------|-----------------|-----------|----------------|
| | 46 | 2.4 | 19.9 | 0.1 | 20 |
| | 40 | 2.5 | 31.5 | 0.2 | 28 |
| | 39 | 1.6 | 15.4 | 0.1 | 27 |
| | 38 | 2.6 | 23.5 | 0.2 | 27 |
| | 37 | 3.2 | 27.4 | 0.2 | 26 |
| | 36 | 3.8 | 31.4 | 0.2 | 26 |
| | 35 | 2.3 | 18.1 | 0.1 | 26 |
| | 34 | 2.5 | 19.6 | 0.1 | 26 |
| | 33 | 1.8 | 14.2 | 0.1 | 26 |
| | 32 | 3.2 | 25.1 | 0.2 | 26 |
| | 31 | 1.0 | 8.0 | 0.1 | 26 |
| | 30 | 1.4 | 10.7 | 0.1 | 26 |
| | 29 | 2.3 | 18.0 | 0.1 | 26 |
| | 28 | 1.5 | 11.8 | 0.1 | 26 |
| Skyline Blvd (West) | 48 | 3.9 | 11.6 | 0.1 | 39 |
| | 27 | 0.4 | 19.7 | 0.1 | 24 |
| | 26 | 0.9 | 24.0 | 0.2 | 29 |
| | 25 | 1.4 | 25.0 | 0.2 | 28 |
| | 24 | 1.7 | 24.3 | 0.2 | 28 |
| | 23 | 1.9 | 23.2 | 0.2 | 28 |
| | 22 | 1.2 | 14.3 | 0.1 | 27 |
| | 21 | 2.3 | 25.1 | 0.2 | 27 |
| | 20 | 4.3 | 43.7 | 0.3 | 27 |
| | 19 | 1.0 | 9.8 | 0.1 | 27 |
| | 45 | 5.4 | 50.2 | 0.4 | 27 |
| | 17 | 9.5 | 83.9 | 0.6 | 27 |
| | 16 | 1.0 | 8.7 | 0.1 | 26 |
| | 15 | 1.0 | 9.0 | 0.1 | 27 |
| | 14 | 2.2 | 18.4 | 0.1 | 26 |
| | 13 | 4.5 | 37.7 | 0.3 | 26 |
| | 12 | 1.0 | 8.2 | 0.1 | 26 |
| | 11 | 4.1 | 30.9 | 0.2 | 26 |
| Ox Mt Landfill Rd | 47 | 3.8 | 10.7 | 0.1 | 29 |
| Total | | 83.7 | 753.0 | 5.6 | 27 |

Arterial Level of Service: EB SR-92

| Cross Street | Node | Delay (s/veh) | Travel time (s) | Dist (mi) | Arterial Speed |
|---------------------|------|---------------|-----------------|-----------|----------------|
| | 11 | 0.4 | 8.7 | 0.1 | 29 |
| | 12 | 2.2 | 29.3 | 0.2 | 28 |
| | 13 | 0.7 | 8.0 | 0.1 | 27 |
| | 14 | 3.8 | 36.8 | 0.3 | 27 |
| | 15 | 2.0 | 18.3 | 0.1 | 26 |
| | 16 | 1.1 | 9.3 | 0.1 | 26 |
| | 17 | 1.0 | 8.5 | 0.1 | 27 |
| | 45 | 10.3 | 84.8 | 0.6 | 27 |
| | 19 | 6.4 | 50.6 | 0.4 | 26 |
| | 20 | 1.3 | 10.1 | 0.1 | 26 |
| | 21 | 5.8 | 45.3 | 0.3 | 26 |
| | 22 | 3.4 | 26.3 | 0.2 | 26 |
| | 23 | 2.0 | 15.0 | 0.1 | 26 |
| | 24 | 3.3 | 24.9 | 0.2 | 26 |
| | 25 | 3.4 | 25.9 | 0.2 | 26 |
| | 26 | 3.7 | 27.7 | 0.2 | 26 |
| | 27 | 3.6 | 26.4 | 0.2 | 26 |
| Skyline Blvd (West) | 48 | 15.4 | 25.6 | 0.1 | 18 |
| | 28 | 4.2 | 23.6 | 0.1 | 19 |
| | 29 | 0.9 | 11.4 | 0.1 | 27 |
| | 30 | 1.7 | 17.5 | 0.1 | 27 |
| | 31 | 1.2 | 10.7 | 0.1 | 26 |
| | 32 | 0.9 | 7.6 | 0.1 | 27 |
| | 33 | 3.0 | 24.9 | 0.2 | 26 |
| | 34 | 1.8 | 14.6 | 0.1 | 26 |
| | 35 | 2.5 | 19.3 | 0.1 | 27 |
| | 36 | 2.3 | 17.9 | 0.1 | 26 |
| | 37 | 4.2 | 31.7 | 0.2 | 26 |
| | 38 | 3.7 | 27.9 | 0.2 | 26 |
| | 39 | 3.3 | 24.4 | 0.2 | 26 |
| | 40 | 2.1 | 15.7 | 0.1 | 26 |
| | 46 | 4.6 | 33.6 | 0.2 | 26 |
| SR-35 (East) | 49 | 7.4 | 16.6 | 0.1 | 24 |
| Total | | 113.5 | 779.0 | 5.6 | 26 |

Arterial Level of Service: WB SR-92

| Cross Street | Node | Delay (s/veh) | Travel time (s) | Dist (mi) | Arterial Speed |
|---------------------|------|---------------|-----------------|-----------|----------------|
| | 46 | 5.8 | 23.3 | 0.1 | 17 |
| | 40 | 3.4 | 32.5 | 0.2 | 27 |
| | 39 | 1.9 | 15.8 | 0.1 | 26 |
| | 38 | 3.1 | 24.0 | 0.2 | 26 |
| | 37 | 3.7 | 27.9 | 0.2 | 26 |
| | 36 | 4.3 | 31.9 | 0.2 | 26 |
| | 35 | 2.5 | 18.4 | 0.1 | 25 |
| | 34 | 2.7 | 19.9 | 0.1 | 26 |
| | 33 | 2.0 | 14.4 | 0.1 | 26 |
| | 32 | 3.5 | 25.5 | 0.2 | 26 |
| | 31 | 1.1 | 8.1 | 0.1 | 25 |
| | 30 | 1.5 | 10.8 | 0.1 | 26 |
| | 29 | 2.7 | 18.3 | 0.1 | 26 |
| | 28 | 2.5 | 12.7 | 0.1 | 24 |
| Skyline Blvd (West) | 48 | 12.4 | 22.9 | 0.1 | 20 |
| | 27 | 1.9 | 21.3 | 0.1 | 22 |
| | 26 | 2.3 | 25.4 | 0.2 | 27 |
| | 25 | 3.0 | 26.6 | 0.2 | 27 |
| | 24 | 3.1 | 25.8 | 0.2 | 26 |
| | 23 | 3.1 | 24.4 | 0.2 | 26 |
| | 22 | 1.9 | 15.0 | 0.1 | 26 |
| | 21 | 3.4 | 26.3 | 0.2 | 26 |
| | 20 | 6.0 | 45.5 | 0.3 | 26 |
| | 19 | 1.4 | 10.1 | 0.1 | 26 |
| | 45 | 6.9 | 51.6 | 0.4 | 26 |
| | 17 | 11.8 | 86.1 | 0.6 | 26 |
| | 16 | 1.2 | 8.9 | 0.1 | 25 |
| | 15 | 1.3 | 9.2 | 0.1 | 26 |
| | 14 | 2.6 | 18.8 | 0.1 | 26 |
| | 13 | 5.3 | 38.4 | 0.3 | 26 |
| | 12 | 1.2 | 8.4 | 0.1 | 25 |
| | 11 | 4.3 | 31.1 | 0.2 | 26 |
| Ox Mt Landfill Rd | 47 | 2.2 | 8.4 | 0.1 | 30 |
| Total | | 116.2 | 787.7 | 5.6 | 26 |

Arterial Level of Service: EB SR-92

| Cross Street | Node | Delay (s/veh) | Travel time (s) | Dist (mi) | Arterial Speed |
|---------------------|------|---------------|-----------------|-----------|----------------|
| | 11 | 0.4 | 8.7 | 0.1 | 29 |
| | 12 | 2.1 | 29.1 | 0.2 | 28 |
| | 13 | 0.7 | 7.9 | 0.1 | 27 |
| | 14 | 3.6 | 36.6 | 0.3 | 27 |
| | 15 | 2.0 | 18.1 | 0.1 | 27 |
| | 16 | 1.0 | 9.2 | 0.1 | 26 |
| | 17 | 1.0 | 8.5 | 0.1 | 27 |
| | 45 | 10.0 | 84.3 | 0.6 | 27 |
| | 19 | 6.3 | 50.4 | 0.4 | 27 |
| | 20 | 1.3 | 10.0 | 0.1 | 26 |
| | 21 | 5.8 | 45.2 | 0.3 | 26 |
| | 22 | 3.4 | 26.1 | 0.2 | 26 |
| | 23 | 1.9 | 15.0 | 0.1 | 26 |
| | 24 | 3.2 | 24.8 | 0.2 | 26 |
| | 25 | 3.4 | 25.9 | 0.2 | 26 |
| | 26 | 3.7 | 27.7 | 0.2 | 26 |
| | 27 | 3.6 | 26.2 | 0.2 | 26 |
| Skyline Blvd (West) | 48 | 16.5 | 27.1 | 0.1 | 17 |
| | 28 | 3.7 | 23.2 | 0.1 | 20 |
| | 29 | 0.9 | 11.4 | 0.1 | 27 |
| | 30 | 1.7 | 17.4 | 0.1 | 27 |
| | 31 | 1.2 | 10.6 | 0.1 | 26 |
| | 32 | 0.9 | 7.6 | 0.1 | 27 |
| | 33 | 3.0 | 24.8 | 0.2 | 26 |
| | 34 | 1.8 | 14.6 | 0.1 | 26 |
| | 35 | 2.5 | 19.4 | 0.1 | 26 |
| | 36 | 2.3 | 17.9 | 0.1 | 26 |
| | 37 | 4.2 | 31.7 | 0.2 | 26 |
| | 38 | 3.8 | 27.9 | 0.2 | 26 |
| | 39 | 3.3 | 24.4 | 0.2 | 26 |
| | 40 | 2.1 | 15.8 | 0.1 | 26 |
| | 46 | 4.6 | 33.7 | 0.2 | 26 |
| SR-35 (East) | 49 | 7.1 | 16.1 | 0.1 | 25 |
| Total | | 112.9 | 777.7 | 5.6 | 26 |

Arterial Level of Service: WB SR-92

| Cross Street | Node | Delay (s/veh) | Travel time (s) | Dist (mi) | Arterial Speed |
|---------------------|------|---------------|-----------------|-----------|----------------|
| | 46 | 4.6 | 22.1 | 0.1 | 18 |
| | 40 | 3.3 | 32.4 | 0.2 | 27 |
| | 39 | 1.9 | 15.8 | 0.1 | 26 |
| | 38 | 3.1 | 24.1 | 0.2 | 26 |
| | 37 | 3.7 | 27.8 | 0.2 | 26 |
| | 36 | 4.3 | 31.9 | 0.2 | 26 |
| | 35 | 2.5 | 18.3 | 0.1 | 25 |
| | 34 | 2.7 | 19.9 | 0.1 | 26 |
| | 33 | 2.0 | 14.4 | 0.1 | 26 |
| | 32 | 3.5 | 25.4 | 0.2 | 26 |
| | 31 | 1.1 | 8.1 | 0.1 | 25 |
| | 30 | 1.5 | 10.8 | 0.1 | 26 |
| | 29 | 2.6 | 18.2 | 0.1 | 26 |
| | 28 | 2.3 | 12.6 | 0.1 | 25 |
| Skyline Blvd (West) | 48 | 12.1 | 22.8 | 0.1 | 20 |
| | 27 | 1.8 | 21.1 | 0.1 | 22 |
| | 26 | 2.3 | 25.4 | 0.2 | 27 |
| | 25 | 3.0 | 26.7 | 0.2 | 27 |
| | 24 | 3.2 | 25.8 | 0.2 | 26 |
| | 23 | 3.1 | 24.4 | 0.2 | 26 |
| | 22 | 1.9 | 15.0 | 0.1 | 26 |
| | 21 | 3.4 | 26.3 | 0.2 | 26 |
| | 20 | 6.1 | 45.5 | 0.3 | 26 |
| | 19 | 1.4 | 10.2 | 0.1 | 26 |
| | 45 | 7.0 | 51.5 | 0.4 | 26 |
| | 17 | 11.8 | 86.0 | 0.6 | 26 |
| | 16 | 1.2 | 8.9 | 0.1 | 25 |
| | 15 | 1.3 | 9.2 | 0.1 | 26 |
| | 14 | 2.6 | 18.8 | 0.1 | 26 |
| | 13 | 5.3 | 38.5 | 0.3 | 26 |
| | 12 | 1.2 | 8.4 | 0.1 | 25 |
| | 11 | 4.4 | 31.2 | 0.2 | 26 |
| Ox Mt Landfill Rd | 47 | 2.7 | 9.2 | 0.1 | 27 |
| Total | | 115.2 | 786.7 | 5.6 | 26 |

| Intersection | | | | | | |
|--------------------------|------|------|------|------|------|------|
| Int Delay, s/veh | 0 | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ↘↗ | | ↑ | | | ↑ |
| Traffic Vol, veh/h | 0 | 0 | 1331 | 0 | 0 | 456 |
| Future Vol, veh/h | 0 | 0 | 1331 | 0 | 0 | 456 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, # | 0 | - | 0 | - | - | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 0 | 1447 | 0 | 0 | 496 |

| Major/Minor | Minor1 | Major1 | Major2 | | | |
|----------------------|--------|--------|--------|---|---|---|
| Conflicting Flow All | 1943 | 1447 | 0 | - | - | - |
| Stage 1 | 1447 | - | - | - | - | - |
| Stage 2 | 496 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | - | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | - | - |
| Pot Cap-1 Maneuver | 71 | 161 | - | 0 | 0 | - |
| Stage 1 | 216 | - | - | 0 | 0 | - |
| Stage 2 | 612 | - | - | 0 | 0 | - |
| Platoon blocked, % | | | - | | | - |
| Mov Cap-1 Maneuver | 71 | 161 | - | - | - | - |
| Mov Cap-2 Maneuver | 71 | - | - | - | - | - |
| Stage 1 | 216 | - | - | - | - | - |
| Stage 2 | 612 | - | - | - | - | - |

| Approach | WB | NB | SB |
|----------------------|----|----|----|
| HCM Control Delay, s | 0 | 0 | 0 |
| HCM LOS | A | | |

| Minor Lane/Major Mvmt | NBTWBLn1 | SBT |
|-----------------------|----------|-----|
| Capacity (veh/h) | - | - |
| HCM Lane V/C Ratio | - | - |
| HCM Control Delay (s) | - | 0 |
| HCM Lane LOS | - | A |
| HCM 95th %tile Q(veh) | - | - |

| Intersection | | | | | | |
|--------------------------|------|------|------|------|------|------|
| Int Delay, s/veh | 0 | | | | | |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | | ↑ | ↑ | | ↓ | |
| Traffic Vol, veh/h | 0 | 1509 | 1103 | 0 | 0 | 0 |
| Future Vol, veh/h | 0 | 1509 | 1103 | 0 | 0 | 0 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, # | - | 0 | 0 | - | 0 | - |
| Grade, % | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 1640 | 1199 | 0 | 0 | 0 |

| Major/Minor | Major1 | Major2 | Minor2 | | |
|----------------------|--------|--------|--------|---|-------------|
| Conflicting Flow All | - | 0 | - | 0 | 2839 1199 |
| Stage 1 | - | - | - | - | 1199 - |
| Stage 2 | - | - | - | - | 1640 - |
| Critical Hdwy | - | - | - | - | 6.42 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 - |
| Follow-up Hdwy | - | - | - | - | 3.518 3.318 |
| Pot Cap-1 Maneuver | 0 | - | - | 0 | 19 226 |
| Stage 1 | 0 | - | - | 0 | 286 - |
| Stage 2 | 0 | - | - | 0 | 174 - |
| Platoon blocked, % | - | - | - | - | - |
| Mov Cap-1 Maneuver | - | - | - | - | 19 226 |
| Mov Cap-2 Maneuver | - | - | - | - | 19 - |
| Stage 1 | - | - | - | - | 286 - |
| Stage 2 | - | - | - | - | 174 - |

| Approach | EB | WB | SB |
|----------------------|----|----|----|
| HCM Control Delay, s | 0 | 0 | 0 |
| HCM LOS | | | A |

| Minor Lane/Major Mvmt | EBT | WBT | SBLn1 |
|-----------------------|-----|-----|-------|
| Capacity (veh/h) | - | - | - |
| HCM Lane V/C Ratio | - | - | - |
| HCM Control Delay (s) | - | - | 0 |
| HCM Lane LOS | - | - | A |
| HCM 95th %tile Q(veh) | - | - | - |

| Intersection | | | | | | |
|--------------------------|------|------|------|------|------|------|
| Int Delay, s/veh | 0 | | | | | |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | | ↑ | ↑ | | ↓ | |
| Traffic Vol, veh/h | 0 | 1331 | 456 | 0 | 0 | 0 |
| Future Vol, veh/h | 0 | 1331 | 456 | 0 | 0 | 0 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, # | - | 0 | 0 | - | 0 | - |
| Grade, % | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 1447 | 496 | 0 | 0 | 0 |

| Major/Minor | Major1 | Major2 | Minor2 | | |
|----------------------|--------|--------|--------|---|-------------|
| Conflicting Flow All | - | 0 | - | 0 | 1943 496 |
| Stage 1 | - | - | - | - | 496 - |
| Stage 2 | - | - | - | - | 1447 - |
| Critical Hdwy | - | - | - | - | 6.42 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 - |
| Follow-up Hdwy | - | - | - | - | 3.518 3.318 |
| Pot Cap-1 Maneuver | 0 | - | - | 0 | 71 574 |
| Stage 1 | 0 | - | - | 0 | 612 - |
| Stage 2 | 0 | - | - | 0 | 216 - |
| Platoon blocked, % | - | - | - | - | - |
| Mov Cap-1 Maneuver | - | - | - | - | 71 574 |
| Mov Cap-2 Maneuver | - | - | - | - | 71 - |
| Stage 1 | - | - | - | - | 612 - |
| Stage 2 | - | - | - | - | 216 - |

| Approach | EB | WB | SB |
|----------------------|----|----|----|
| HCM Control Delay, s | 0 | 0 | 0 |
| HCM LOS | | | A |

| Minor Lane/Major Mvmt | EBT | WBT | SBLn1 |
|-----------------------|-----|-----|-------|
| Capacity (veh/h) | - | - | - |
| HCM Lane V/C Ratio | - | - | - |
| HCM Control Delay (s) | - | - | 0 |
| HCM Lane LOS | - | - | A |
| HCM 95th %tile Q(veh) | - | - | - |

| Intersection | | | | | | | | | | | | |
|--------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Int Delay, s/veh | 0.3 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ↖ | ↗ | | | ↖ | ↗ | | ↕ | | | ↕ | |
| Traffic Vol, veh/h | 2 | 1315 | 0 | 0 | 3 | 774 | 1 | 0 | 2 | 14 | 0 | 1 |
| Future Vol, veh/h | 2 | 1315 | 0 | 0 | 3 | 774 | 1 | 0 | 2 | 14 | 0 | 1 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | 150 | - | - | - | - | 55 | - | - | - | - | - | - |
| Veh in Median Storage, # | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 2 | 1384 | 0 | 0 | 3 | 815 | 1 | 0 | 2 | 15 | 0 | 1 |

| Major/Minor | Major1 | | | Major2 | | | Minor1 | | | Minor2 | | |
|----------------------|--------|---|---|--------|---|---|--------|-------|-------|--------|-------|-------|
| Conflicting Flow All | 818 | 0 | 0 | 1384 | 0 | 0 | 1799 | 2206 | 1384 | 1392 | 1391 | 3 |
| Stage 1 | - | - | - | - | - | - | 1388 | 1388 | - | 3 | 3 | - |
| Stage 2 | - | - | - | - | - | - | 411 | 818 | - | 1389 | 1388 | - |
| Critical Hdwy | 4.12 | - | - | 4.12 | - | - | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - |
| Follow-up Hdwy | 2.218 | - | - | 2.218 | - | - | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 |
| Pot Cap-1 Maneuver | 810 | - | - | 495 | - | - | 62 | 44 | 176 | 119 | 142 | 1081 |
| Stage 1 | - | - | - | - | - | - | 177 | 210 | - | 1020 | 893 | - |
| Stage 2 | - | - | - | - | - | - | 618 | 390 | - | 176 | 210 | - |
| Platoon blocked, % | | - | - | | - | - | | | | | | |
| Mov Cap-1 Maneuver | 810 | - | - | 495 | - | - | 62 | 44 | 176 | 117 | 142 | 1081 |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | 62 | 44 | - | 117 | 142 | - |
| Stage 1 | - | - | - | - | - | - | 177 | 210 | - | 1018 | 893 | - |
| Stage 2 | - | - | - | - | - | - | 617 | 390 | - | 173 | 210 | - |

| Approach | EB | | | WB | | | NB | | | SB | | |
|----------------------|----|--|--|----|--|--|----|--|--|------|--|--|
| HCM Control Delay, s | 0 | | | 0 | | | 39 | | | 38.2 | | |
| HCM LOS | | | | | | | E | | | E | | |

| Minor Lane/Major Mvmt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 |
|-----------------------|-------|-------|-----|-----|-----|-----|-----|-------|
| Capacity (veh/h) | 109 | 810 | - | - | 495 | - | - | 124 |
| HCM Lane V/C Ratio | 0.029 | 0.003 | - | - | - | - | - | 0.127 |
| HCM Control Delay (s) | 39 | 9.5 | - | - | 0 | - | - | 38.2 |
| HCM Lane LOS | E | A | - | - | A | - | - | E |
| HCM 95th %tile Q(veh) | 0.1 | 0 | - | - | 0 | - | - | 0.4 |

HCM 6th Roundabout
48: Skyline Blvd (West) & SR-92

01/15/2020

| Intersection | | | | | | | |
|-----------------------------|-------|-------|--------|-------|-------|-------|--------|
| Intersection Delay, s/veh | 13.2 | | | | | | |
| Intersection LOS | B | | | | | | |
| Approach | EB | | | WB | | NB | |
| Entry Lanes | 2 | | | 2 | | 1 | |
| Conflicting Circle Lanes | 2 | | | 2 | | 2 | |
| Adj Approach Flow, veh/h | 1230 | | | 469 | | 428 | |
| Demand Flow Rate, veh/h | 1255 | | | 478 | | 436 | |
| Vehicles Circulating, veh/h | 124 | | | 120 | | 1255 | |
| Vehicles Exiting, veh/h | 474 | | | 1255 | | 124 | |
| Ped Vol Crossing Leg, #/h | 0 | | | 0 | | 0 | |
| Ped Cap Adj | 1.000 | | | 1.000 | | 1.000 | |
| Approach Delay, s/veh | 8.5 | | | 4.6 | | 36.1 | |
| Approach LOS | A | | | A | | E | |
| Lane | Left | Right | Bypass | Left | Right | Left | Bypass |
| Designated Moves | LT | TR | R | LT | TR | L | R |
| Assumed Moves | LT | TR | R | LT | TR | L | R |
| RT Channelized | | | Free | | | | Yield |
| Lane Util | 0.470 | 0.530 | | 0.471 | 0.529 | 1.000 | |
| Follow-Up Headway, s | 2.667 | 2.535 | | 2.667 | 2.535 | 2.535 | |
| Critical Headway, s | 4.645 | 4.328 | 0 | 4.645 | 4.328 | 4.328 | 316 |
| Entry Flow, veh/h | 590 | 665 | 1938 | 225 | 253 | 120 | 384 |
| Cap Entry Lane, veh/h | 1204 | 1278 | 0.980 | 1209 | 1282 | 489 | 0.980 |
| Entry HV Adj Factor | 0.980 | 0.981 | 0 | 0.980 | 0.983 | 0.983 | 310 |
| Flow Entry, veh/h | 578 | 652 | 1900 | 220 | 249 | 118 | 376 |
| Cap Entry, veh/h | 1180 | 1253 | 0.000 | 1184 | 1260 | 480 | 0.824 |
| V/C Ratio | 0.490 | 0.520 | 0.0 | 0.186 | 0.197 | 0.246 | 45.6 |
| Control Delay, s/veh | 8.4 | 8.5 | A | 4.7 | 4.5 | 11.1 | E |
| LOS | A | A | 0 | A | A | B | 7 |
| 95th %tile Queue, veh | 3 | 3 | | 1 | 1 | 1 | |

| Intersection | | | | | | | | |
|--------------------------------|-------|-------|-------|-------|--------|-------|-------|--|
| Intersection Delay, s/veh 11.6 | | | | | | | | |
| Intersection LOS B | | | | | | | | |
| Approach | EB | | WB | | | SB | | |
| Entry Lanes | 2 | | 2 | | | 2 | | |
| Conflicting Circle Lanes | 2 | | 2 | | | 2 | | |
| Adj Approach Flow, veh/h | 1662 | | 1117 | | | 166 | | |
| Demand Flow Rate, veh/h | 1695 | | 1140 | | | 169 | | |
| Vehicles Circulating, veh/h | 46 | | 497 | | | 1100 | | |
| Vehicles Exiting, veh/h | 1223 | | 1244 | | | 497 | | |
| Ped Vol Crossing Leg, #/h | 0 | | 0 | | | 0 | | |
| Ped Cap Adj | 1.000 | | 1.000 | | | 1.000 | | |
| Approach Delay, s/veh | 10.7 | | 13.3 | | | 9.3 | | |
| Approach LOS | B | | B | | | A | | |
| Lane | Left | Right | Left | Right | Bypass | Left | Right | |
| Designated Moves | LT | TR | LT | TR | R | L | TR | |
| Assumed Moves | LT | TR | LT | TR | R | L | TR | |
| RT Channelized | Yield | | | | | | | |
| Lane Util | 0.470 | 0.530 | 0.470 | 0.530 | | 0.272 | 0.728 | |
| Follow-Up Headway, s | 2.667 | 2.535 | 2.667 | 2.535 | | 2.667 | 2.535 | |
| Critical Headway, s | 4.645 | 4.328 | 4.645 | 4.328 | 40 | 4.645 | 4.328 | |
| Entry Flow, veh/h | 797 | 898 | 517 | 583 | 831 | 46 | 123 | |
| Cap Entry Lane, veh/h | 1294 | 1366 | 855 | 931 | 0.980 | 491 | 557 | |
| Entry HV Adj Factor | 0.980 | 0.981 | 0.980 | 0.980 | 39 | 0.978 | 0.984 | |
| Flow Entry, veh/h | 781 | 881 | 507 | 572 | 815 | 45 | 121 | |
| Cap Entry, veh/h | 1268 | 1339 | 838 | 912 | 0.048 | 480 | 548 | |
| V/C Ratio | 0.616 | 0.658 | 0.605 | 0.626 | 4.9 | 0.094 | 0.221 | |
| Control Delay, s/veh | 10.4 | 11.0 | 13.7 | 13.5 | A | 8.7 | 9.5 | |
| LOS | B | B | B | B | 0 | A | A | |
| 95th %tile Queue, veh | 4 | 5 | 4 | 5 | | 0 | 1 | |

| Intersection | | | | | | |
|--------------------------|------|------|------|------|------|------|
| Int Delay, s/veh | 0 | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ↑↓ | | ↑ | | | ↑ |
| Traffic Vol, veh/h | 0 | 0 | 930 | 0 | 0 | 1597 |
| Future Vol, veh/h | 0 | 0 | 930 | 0 | 0 | 1597 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, # | 0 | - | 0 | - | - | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 0 | 1011 | 0 | 0 | 1736 |

| Major/Minor | Minor1 | Major1 | Major2 | | | |
|----------------------|--------|--------|--------|---|---|---|
| Conflicting Flow All | 2747 | 1011 | 0 | - | - | - |
| Stage 1 | 1011 | - | - | - | - | - |
| Stage 2 | 1736 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | - | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | - | - |
| Pot Cap-1 Maneuver | 22 | 291 | - | 0 | 0 | - |
| Stage 1 | 352 | - | - | 0 | 0 | - |
| Stage 2 | 156 | - | - | 0 | 0 | - |
| Platoon blocked, % | | | - | | | - |
| Mov Cap-1 Maneuver | 22 | 291 | - | - | - | - |
| Mov Cap-2 Maneuver | 22 | - | - | - | - | - |
| Stage 1 | 352 | - | - | - | - | - |
| Stage 2 | 156 | - | - | - | - | - |

| Approach | WB | NB | SB |
|----------------------|----|----|----|
| HCM Control Delay, s | 0 | 0 | 0 |
| HCM LOS | A | | |

| Minor Lane/Major Mvmt | NBTWBLn1 | SBT |
|-----------------------|----------|-----|
| Capacity (veh/h) | - | - |
| HCM Lane V/C Ratio | - | - |
| HCM Control Delay (s) | - | 0 |
| HCM Lane LOS | - | A |
| HCM 95th %tile Q(veh) | - | - |

| Intersection | | | | | | |
|--------------------------|------|------|------|------|------|------|
| Int Delay, s/veh | 0 | | | | | |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | | ↑ | ↑ | | ↓ | |
| Traffic Vol, veh/h | 0 | 1226 | 1945 | 0 | 0 | 0 |
| Future Vol, veh/h | 0 | 1226 | 1945 | 0 | 0 | 0 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, # | - | 0 | 0 | - | 0 | - |
| Grade, % | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 1333 | 2114 | 0 | 0 | 0 |

| Major/Minor | Major1 | Major2 | Minor2 | | |
|----------------------|--------|--------|--------|---|-------------|
| Conflicting Flow All | - | 0 | - | 0 | 3447 2114 |
| Stage 1 | - | - | - | - | 2114 - |
| Stage 2 | - | - | - | - | 1333 - |
| Critical Hdwy | - | - | - | - | 6.42 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 - |
| Follow-up Hdwy | - | - | - | - | 3.518 3.318 |
| Pot Cap-1 Maneuver | 0 | - | - | 0 | 8 64 |
| Stage 1 | 0 | - | - | 0 | 100 - |
| Stage 2 | 0 | - | - | 0 | 246 - |
| Platoon blocked, % | - | - | - | - | - |
| Mov Cap-1 Maneuver | - | - | - | - | 8 64 |
| Mov Cap-2 Maneuver | - | - | - | - | 8 - |
| Stage 1 | - | - | - | - | 100 - |
| Stage 2 | - | - | - | - | 246 - |

| Approach | EB | WB | SB |
|----------------------|----|----|----|
| HCM Control Delay, s | 0 | 0 | 0 |
| HCM LOS | | | A |

| Minor Lane/Major Mvmt | EBT | WBT | SBLn1 |
|-----------------------|-----|-----|-------|
| Capacity (veh/h) | - | - | - |
| HCM Lane V/C Ratio | - | - | - |
| HCM Control Delay (s) | - | - | 0 |
| HCM Lane LOS | - | - | A |
| HCM 95th %tile Q(veh) | - | - | - |

| Intersection | | | | | | |
|--------------------------|------|------|------|------|------|------|
| Int Delay, s/veh | 0 | | | | | |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | | ↑ | ↑ | | ↓ | |
| Traffic Vol, veh/h | 0 | 930 | 1597 | 0 | 0 | 0 |
| Future Vol, veh/h | 0 | 930 | 1597 | 0 | 0 | 0 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, # | - | 0 | 0 | - | 0 | - |
| Grade, % | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 1011 | 1736 | 0 | 0 | 0 |

| Major/Minor | Major1 | Major2 | Minor2 | | |
|----------------------|--------|--------|--------|---|-------------|
| Conflicting Flow All | - | 0 | - | 0 | 2747 1736 |
| Stage 1 | - | - | - | - | 1736 - |
| Stage 2 | - | - | - | - | 1011 - |
| Critical Hdwy | - | - | - | - | 6.42 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 - |
| Follow-up Hdwy | - | - | - | - | 3.518 3.318 |
| Pot Cap-1 Maneuver | 0 | - | - | 0 | 22 108 |
| Stage 1 | 0 | - | - | 0 | 156 - |
| Stage 2 | 0 | - | - | 0 | 352 - |
| Platoon blocked, % | - | - | - | - | - |
| Mov Cap-1 Maneuver | - | - | - | - | 22 108 |
| Mov Cap-2 Maneuver | - | - | - | - | 22 - |
| Stage 1 | - | - | - | - | 156 - |
| Stage 2 | - | - | - | - | 352 - |

| Approach | EB | WB | SB |
|----------------------|----|----|----|
| HCM Control Delay, s | 0 | 0 | 0 |
| HCM LOS | | | A |

| Minor Lane/Major Mvmt | EBT | WBT | SBLn1 |
|-----------------------|-----|-----|-------|
| Capacity (veh/h) | - | - | - |
| HCM Lane V/C Ratio | - | - | - |
| HCM Control Delay (s) | - | - | 0 |
| HCM Lane LOS | - | - | A |
| HCM 95th %tile Q(veh) | - | - | - |

| Intersection | | | | | | | | | | | | |
|--------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Int Delay, s/veh | 0.4 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | | | | | | | | | | | |
| Traffic Vol, veh/h | 4 | 926 | 4 | 1 | 1350 | 5 | 0 | 0 | 2 | 2 | 0 | 12 |
| Future Vol, veh/h | 4 | 926 | 4 | 1 | 1350 | 5 | 0 | 0 | 2 | 2 | 0 | 12 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | 150 | - | - | - | - | 55 | - | - | - | - | - | - |
| Veh in Median Storage, # | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 4 | 1007 | 4 | 1 | 1467 | 5 | 0 | 0 | 2 | 2 | 0 | 13 |

| Major/Minor | Major1 | | | Major2 | | | Minor1 | | | Minor2 | | |
|----------------------|--------|---|---|--------|---|---|--------|-------|-------|--------|-------|-------|
| Conflicting Flow All | 1472 | 0 | 0 | 1011 | 0 | 0 | 2495 | 2491 | 1009 | 2487 | 2488 | 1467 |
| Stage 1 | - | - | - | - | - | - | 1017 | 1017 | - | 1469 | 1469 | - |
| Stage 2 | - | - | - | - | - | - | 1478 | 1474 | - | 1018 | 1019 | - |
| Critical Hdwy | 4.12 | - | - | 4.12 | - | - | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - |
| Follow-up Hdwy | 2.218 | - | - | 2.218 | - | - | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 |
| Pot Cap-1 Maneuver | 458 | - | - | 686 | - | - | 20 | 29 | 292 | 20 | 29 | 157 |
| Stage 1 | - | - | - | - | - | - | 287 | 315 | - | 159 | 192 | - |
| Stage 2 | - | - | - | - | - | - | 157 | 191 | - | 286 | 314 | - |
| Platoon blocked, % | - | - | - | - | - | - | - | - | - | - | - | - |
| Mov Cap-1 Maneuver | 458 | - | - | 686 | - | - | 18 | 29 | 292 | 20 | 29 | 157 |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | 18 | 29 | - | 20 | 29 | - |
| Stage 1 | - | - | - | - | - | - | 284 | 312 | - | 158 | 190 | - |
| Stage 2 | - | - | - | - | - | - | 143 | 189 | - | 281 | 311 | - |

| Approach | EB | WB | NB | SB |
|----------------------|-----|----|------|------|
| HCM Control Delay, s | 0.1 | 0 | 17.4 | 61.1 |
| HCM LOS | | | C | F |

| Minor Lane/Major Mvmt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 |
|-----------------------|-------|-------|-----|-----|-------|-----|-----|-------|
| Capacity (veh/h) | 292 | 458 | - | - | 686 | - | - | 79 |
| HCM Lane V/C Ratio | 0.007 | 0.009 | - | - | 0.002 | - | - | 0.193 |
| HCM Control Delay (s) | 17.4 | 12.9 | - | - | 10.3 | 0 | - | 61.1 |
| HCM Lane LOS | C | B | - | - | B | A | - | F |
| HCM 95th %tile Q(veh) | 0 | 0 | - | - | 0 | - | - | 0.7 |

HCM 6th Roundabout
48: Skyline Blvd (West) & SR-92

01/15/2020

| Intersection | | | | | | | |
|-----------------------------|-------|-------|--------|-------|-------|-------|--------|
| Intersection Delay, s/veh | 23.0 | | | | | | |
| Intersection LOS | C | | | | | | |
| Approach | EB | | | WB | | NB | |
| Entry Lanes | 2 | | | 2 | | 1 | |
| Conflicting Circle Lanes | 2 | | | 2 | | 2 | |
| Adj Approach Flow, veh/h | 908 | | | 2114 | | 647 | |
| Demand Flow Rate, veh/h | 927 | | | 2156 | | 660 | |
| Vehicles Circulating, veh/h | 534 | | | 149 | | 849 | |
| Vehicles Exiting, veh/h | 1771 | | | 849 | | 534 | |
| Ped Vol Crossing Leg, #/h | 0 | | | 0 | | 0 | |
| Ped Cap Adj | 1.000 | | | 1.000 | | 1.000 | |
| Approach Delay, s/veh | 9.8 | | | 25.5 | | 33.2 | |
| Approach LOS | A | | | D | | D | |
| Lane | Left | Right | Bypass | Left | Right | Left | Bypass |
| Designated Moves | LT | TR | R | LT | TR | L | R |
| Assumed Moves | LT | TR | R | LT | TR | L | R |
| RT Channelized | | | Free | | | | Yield |
| Lane Util | 0.470 | 0.530 | | 0.470 | 0.530 | 1.000 | |
| Follow-Up Headway, s | 2.667 | 2.535 | | 2.667 | 2.535 | 2.535 | |
| Critical Headway, s | 4.645 | 4.328 | 78 | 4.645 | 4.328 | 4.328 | 511 |
| Entry Flow, veh/h | 399 | 450 | 1938 | 1013 | 1143 | 149 | 580 |
| Cap Entry Lane, veh/h | 826 | 902 | 0.980 | 1177 | 1251 | 690 | 0.980 |
| Entry HV Adj Factor | 0.980 | 0.980 | 76 | 0.981 | 0.980 | 0.980 | 501 |
| Flow Entry, veh/h | 391 | 441 | 1900 | 994 | 1121 | 146 | 569 |
| Cap Entry, veh/h | 810 | 884 | 0.040 | 1154 | 1227 | 676 | 0.880 |
| V/C Ratio | 0.483 | 0.499 | 0.0 | 0.861 | 0.914 | 0.216 | 40.6 |
| Control Delay, s/veh | 10.9 | 10.6 | A | 22.9 | 27.9 | 7.9 | E |
| LOS | B | B | 0 | C | D | A | 10 |
| 95th %tile Queue, veh | 3 | 3 | | 12 | 15 | 1 | |

| Intersection | | | | | | | | |
|--------------------------------|-------|-------|-------|-------|--------|-------|-------|--|
| Intersection Delay, s/veh 27.2 | | | | | | | | |
| Intersection LOS D | | | | | | | | |
| Approach | EB | | WB | | | SB | | |
| Entry Lanes | 2 | | 2 | | | 2 | | |
| Conflicting Circle Lanes | 2 | | 2 | | | 2 | | |
| Adj Approach Flow, veh/h | 1279 | | 1934 | | | 300 | | |
| Demand Flow Rate, veh/h | 1305 | | 1973 | | | 306 | | |
| Vehicles Circulating, veh/h | 21 | | 346 | | | 1872 | | |
| Vehicles Exiting, veh/h | 2157 | | 980 | | | 346 | | |
| Ped Vol Crossing Leg, #/h | 0 | | 0 | | | 0 | | |
| Ped Cap Adj | 1.000 | | 1.000 | | | 1.000 | | |
| Approach Delay, s/veh | 7.6 | | 31.2 | | | 84.5 | | |
| Approach LOS | A | | D | | | F | | |
| Lane | Left | Right | Left | Right | Bypass | Left | Right | |
| Designated Moves | LT | TR | LT | TR | R | L | TR | |
| Assumed Moves | LT | TR | LT | TR | R | L | TR | |
| RT Channelized | Yield | | | | | | | |
| Lane Util | 0.470 | 0.530 | 0.470 | 0.530 | | 0.069 | 0.931 | |
| Follow-Up Headway, s | 2.667 | 2.535 | 2.667 | 2.535 | | 2.667 | 2.535 | |
| Critical Headway, s | 4.645 | 4.328 | 4.645 | 4.328 | 101 | 4.645 | 4.328 | |
| Entry Flow, veh/h | 613 | 692 | 880 | 992 | 970 | 21 | 285 | |
| Cap Entry Lane, veh/h | 1324 | 1395 | 982 | 1058 | 0.980 | 241 | 289 | |
| Entry HV Adj Factor | 0.981 | 0.980 | 0.980 | 0.981 | 99 | 1.000 | 0.979 | |
| Flow Entry, veh/h | 601 | 678 | 863 | 973 | 951 | 21 | 279 | |
| Cap Entry, veh/h | 1299 | 1367 | 962 | 1038 | 0.104 | 241 | 283 | |
| V/C Ratio | 0.463 | 0.496 | 0.896 | 0.937 | 4.7 | 0.087 | 0.986 | |
| Control Delay, s/veh | 7.5 | 7.7 | 30.2 | 34.8 | A | 16.8 | 89.6 | |
| LOS | A | A | D | D | 0 | C | F | |
| 95th %tile Queue, veh | 3 | 3 | 13 | 15 | | 0 | 10 | |

| Intersection | | | | | | |
|--------------------------|------|------|------|------|------|------|
| Int Delay, s/veh | 0 | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ↑↑ | | ↑ | | | ↑ |
| Traffic Vol, veh/h | 0 | 0 | 874 | 0 | 0 | 1573 |
| Future Vol, veh/h | 0 | 0 | 874 | 0 | 0 | 1573 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, # | 0 | - | 0 | - | - | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 0 | 950 | 0 | 0 | 1710 |

| Major/Minor | Minor1 | Major1 | Major2 | | | |
|----------------------|--------|--------|--------|---|---|---|
| Conflicting Flow All | 2660 | 950 | 0 | - | - | - |
| Stage 1 | 950 | - | - | - | - | - |
| Stage 2 | 1710 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | - | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | - | - |
| Pot Cap-1 Maneuver | 25 | 315 | - | 0 | 0 | - |
| Stage 1 | 376 | - | - | 0 | 0 | - |
| Stage 2 | 160 | - | - | 0 | 0 | - |
| Platoon blocked, % | | | - | | | - |
| Mov Cap-1 Maneuver | 25 | 315 | - | - | - | - |
| Mov Cap-2 Maneuver | 25 | - | - | - | - | - |
| Stage 1 | 376 | - | - | - | - | - |
| Stage 2 | 160 | - | - | - | - | - |

| Approach | WB | NB | SB |
|----------------------|----|----|----|
| HCM Control Delay, s | 0 | 0 | 0 |
| HCM LOS | A | | |

| Minor Lane/Major Mvmt | NBTWBLn1 | SBT |
|-----------------------|----------|-----|
| Capacity (veh/h) | - | - |
| HCM Lane V/C Ratio | - | - |
| HCM Control Delay (s) | - | 0 |
| HCM Lane LOS | - | A |
| HCM 95th %tile Q(veh) | - | - |

| Intersection | | | | | | |
|--------------------------|------|------|------|------|------|------|
| Int Delay, s/veh | 0 | | | | | |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | | ↑ | ↑ | | ↓ | |
| Traffic Vol, veh/h | 0 | 1185 | 1840 | 0 | 0 | 0 |
| Future Vol, veh/h | 0 | 1185 | 1840 | 0 | 0 | 0 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, # | - | 0 | 0 | - | 0 | - |
| Grade, % | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 1288 | 2000 | 0 | 0 | 0 |

| Major/Minor | Major1 | Major2 | Minor2 | | |
|----------------------|--------|--------|--------|---|-------------|
| Conflicting Flow All | - | 0 | - | 0 | 3288 2000 |
| Stage 1 | - | - | - | - | 2000 - |
| Stage 2 | - | - | - | - | 1288 - |
| Critical Hdwy | - | - | - | - | 6.42 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 - |
| Follow-up Hdwy | - | - | - | - | 3.518 3.318 |
| Pot Cap-1 Maneuver | 0 | - | - | 0 | 10 75 |
| Stage 1 | 0 | - | - | 0 | 115 - |
| Stage 2 | 0 | - | - | 0 | 259 - |
| Platoon blocked, % | - | - | - | - | - |
| Mov Cap-1 Maneuver | - | - | - | - | 10 75 |
| Mov Cap-2 Maneuver | - | - | - | - | 10 - |
| Stage 1 | - | - | - | - | 115 - |
| Stage 2 | - | - | - | - | 259 - |

| Approach | EB | WB | SB |
|----------------------|----|----|----|
| HCM Control Delay, s | 0 | 0 | 0 |
| HCM LOS | | | A |

| Minor Lane/Major Mvmt | EBT | WBT | SBLn1 |
|-----------------------|-----|-----|-------|
| Capacity (veh/h) | - | - | - |
| HCM Lane V/C Ratio | - | - | - |
| HCM Control Delay (s) | - | - | 0 |
| HCM Lane LOS | - | - | A |
| HCM 95th %tile Q(veh) | - | - | - |

| Intersection | | | | | | |
|--------------------------|------|------|------|------|------|------|
| Int Delay, s/veh | 0 | | | | | |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | | ↑ | ↑ | | ↓ | |
| Traffic Vol, veh/h | 0 | 874 | 1573 | 0 | 0 | 0 |
| Future Vol, veh/h | 0 | 874 | 1573 | 0 | 0 | 0 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, # | - | 0 | 0 | - | 0 | - |
| Grade, % | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 950 | 1710 | 0 | 0 | 0 |

| Major/Minor | Major1 | Major2 | Minor2 | | |
|----------------------|--------|--------|--------|---|-------------|
| Conflicting Flow All | - | 0 | - | 0 | 2660 1710 |
| Stage 1 | - | - | - | - | 1710 - |
| Stage 2 | - | - | - | - | 950 - |
| Critical Hdwy | - | - | - | - | 6.42 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 - |
| Follow-up Hdwy | - | - | - | - | 3.518 3.318 |
| Pot Cap-1 Maneuver | 0 | - | - | 0 | 25 112 |
| Stage 1 | 0 | - | - | 0 | 160 - |
| Stage 2 | 0 | - | - | 0 | 376 - |
| Platoon blocked, % | - | - | - | - | - |
| Mov Cap-1 Maneuver | - | - | - | - | 25 112 |
| Mov Cap-2 Maneuver | - | - | - | - | 25 - |
| Stage 1 | - | - | - | - | 160 - |
| Stage 2 | - | - | - | - | 376 - |

| Approach | EB | WB | SB |
|----------------------|----|----|----|
| HCM Control Delay, s | 0 | 0 | 0 |
| HCM LOS | | | A |

| Minor Lane/Major Mvmt | EBT | WBT | SBLn1 |
|-----------------------|-----|-----|-------|
| Capacity (veh/h) | - | - | - |
| HCM Lane V/C Ratio | - | - | - |
| HCM Control Delay (s) | - | - | 0 |
| HCM Lane LOS | - | - | A |
| HCM 95th %tile Q(veh) | - | - | - |

| Intersection | | | | | | | | | | | | |
|--------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Int Delay, s/veh | 2.5 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ↖ | ↗ | | | ↖ | ↗ | | ↕ | | | ↕ | |
| Traffic Vol, veh/h | 1 | 861 | 4 | 3 | 1511 | 3 | 0 | 0 | 2 | 11 | 0 | 13 |
| Future Vol, veh/h | 1 | 861 | 4 | 3 | 1511 | 3 | 0 | 0 | 2 | 11 | 0 | 13 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | 150 | - | - | - | - | 55 | - | - | - | - | - | - |
| Veh in Median Storage, # | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 906 | 4 | 3 | 1591 | 3 | 0 | 0 | 2 | 12 | 0 | 14 |

| Major/Minor | Major1 | | | Major2 | | | Minor1 | | | Minor2 | | |
|----------------------|--------|---|---|--------|---|---|--------|-------|-------|--------|-------|-------|
| Conflicting Flow All | 1594 | 0 | 0 | 910 | 0 | 0 | 2516 | 2510 | 908 | 2508 | 2509 | 1591 |
| Stage 1 | - | - | - | - | - | - | 910 | 910 | - | 1597 | 1597 | - |
| Stage 2 | - | - | - | - | - | - | 1606 | 1600 | - | 911 | 912 | - |
| Critical Hdwy | 4.12 | - | - | 4.12 | - | - | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - |
| Follow-up Hdwy | 2.218 | - | - | 2.218 | - | - | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 |
| Pot Cap-1 Maneuver | 411 | - | - | 748 | - | - | 19 | 28 | 334 | 19 | 28 | 132 |
| Stage 1 | - | - | - | - | - | - | 329 | 353 | - | 134 | 166 | - |
| Stage 2 | - | - | - | - | - | - | 132 | 165 | - | 328 | 353 | - |
| Platoon blocked, % | | - | - | | - | - | | | | | | |
| Mov Cap-1 Maneuver | 411 | - | - | 748 | - | - | 17 | 27 | 334 | 18 | 27 | 132 |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | 17 | 27 | - | 18 | 27 | - |
| Stage 1 | - | - | - | - | - | - | 328 | 352 | - | 134 | 160 | - |
| Stage 2 | - | - | - | - | - | - | 114 | 159 | - | 325 | 352 | - |

| Approach | EB | | | WB | | | NB | | | SB | | |
|----------------------|----|--|--|----|--|--|------|--|--|-------|--|--|
| HCM Control Delay, s | 0 | | | 0 | | | 15.8 | | | 249.9 | | |
| HCM LOS | | | | | | | C | | | F | | |

| Minor Lane/Major Mvmt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 | |
|-----------------------|-------|-------|-----|-----|-------|-----|-----|-------|-----|
| Capacity (veh/h) | 334 | 411 | - | - | 748 | - | - | 34 | |
| HCM Lane V/C Ratio | 0.006 | 0.003 | - | - | 0.004 | - | - | 0.743 | |
| HCM Control Delay (s) | 15.8 | 13.8 | - | - | 9.8 | 0 | - | 249.9 | |
| HCM Lane LOS | | C | B | - | - | A | A | - | F |
| HCM 95th %tile Q(veh) | | 0 | 0 | - | - | 0 | - | - | 2.6 |

HCM 6th Roundabout
48: Skyline Blvd (West) & SR-92

01/15/2020

| Intersection | | | | | | | |
|-----------------------------|-------|-------|--------|-------|-------|-------|--------|
| Intersection Delay, s/veh | 18.9 | | | | | | |
| Intersection LOS | C | | | | | | |
| Approach | EB | | WB | | | NB | |
| Entry Lanes | 2 | | 2 | | | 1 | |
| Conflicting Circle Lanes | 2 | | 2 | | | 2 | |
| Adj Approach Flow, veh/h | 973 | | 2097 | | | 534 | |
| Demand Flow Rate, veh/h | 993 | | 2139 | | | 545 | |
| Vehicles Circulating, veh/h | 466 | | 110 | | | 909 | |
| Vehicles Exiting, veh/h | 1783 | | 909 | | | 466 | |
| Ped Vol Crossing Leg, #/h | 0 | | 0 | | | 0 | |
| Ped Cap Adj | 1.000 | | 1.000 | | | 1.000 | |
| Approach Delay, s/veh | 9.5 | | 21.3 | | | 27.0 | |
| Approach LOS | A | | C | | | D | |
| Lane | Left | Right | Bypass | Left | Right | Left | Bypass |
| Designated Moves | LT | TR | R | LT | TR | L | R |
| Assumed Moves | LT | TR | R | LT | TR | L | R |
| RT Channelized | | | Free | | | | Yield |
| Lane Util | 0.470 | 0.530 | | 0.470 | 0.530 | 1.000 | |
| Follow-Up Headway, s | 2.667 | 2.535 | | 2.667 | 2.535 | 2.535 | |
| Critical Headway, s | 4.645 | 4.328 | 84 | 4.645 | 4.328 | 4.328 | 435 |
| Entry Flow, veh/h | 427 | 482 | 1938 | 1005 | 1134 | 110 | 546 |
| Cap Entry Lane, veh/h | 879 | 956 | 0.980 | 1220 | 1293 | 656 | 0.980 |
| Entry HV Adj Factor | 0.981 | 0.980 | 82 | 0.981 | 0.980 | 0.982 | 426 |
| Flow Entry, veh/h | 419 | 472 | 1900 | 986 | 1112 | 108 | 535 |
| Cap Entry, veh/h | 862 | 936 | 0.043 | 1196 | 1268 | 644 | 0.796 |
| V/C Ratio | 0.486 | 0.504 | 0.0 | 0.824 | 0.877 | 0.168 | 32.0 |
| Control Delay, s/veh | 10.5 | 10.2 | A | 19.3 | 23.0 | 7.6 | D |
| LOS | B | B | 0 | C | C | A | 8 |
| 95th %tile Queue, veh | 3 | 3 | | 10 | 13 | 1 | |

HCM 6th Roundabout
49: SR-92 & SR-35 (East)

01/15/2020

| Intersection | | | | | | | | |
|--------------------------------|-------|-------|-------|-------|--------|-------|-------|--|
| Intersection Delay, s/veh 18.8 | | | | | | | | |
| Intersection LOS C | | | | | | | | |
| Approach | EB | | WB | | | SB | | |
| Entry Lanes | 2 | | 2 | | | 2 | | |
| Conflicting Circle Lanes | 2 | | 2 | | | 2 | | |
| Adj Approach Flow, veh/h | 1156 | | 1775 | | | 245 | | |
| Demand Flow Rate, veh/h | 1179 | | 1811 | | | 249 | | |
| Vehicles Circulating, veh/h | 23 | | 359 | | | 1729 | | |
| Vehicles Exiting, veh/h | 1955 | | 843 | | | 359 | | |
| Ped Vol Crossing Leg, #/h | 0 | | 0 | | | 0 | | |
| Ped Cap Adj | 1.000 | | 1.000 | | | 1.000 | | |
| Approach Delay, s/veh | 6.9 | | 24.3 | | | 34.4 | | |
| Approach LOS | A | | C | | | D | | |
| Lane | Left | Right | Left | Right | Bypass | Left | Right | |
| Designated Moves | LT | TR | LT | TR | R | L | TR | |
| Assumed Moves | LT | TR | LT | TR | R | L | TR | |
| RT Channelized | Yield | | | | | | | |
| Lane Util | 0.470 | 0.530 | 0.470 | 0.530 | | 0.092 | 0.908 | |
| Follow-Up Headway, s | 2.667 | 2.535 | 2.667 | 2.535 | | 2.667 | 2.535 | |
| Critical Headway, s | 4.645 | 4.328 | 4.645 | 4.328 | 82 | 4.645 | 4.328 | |
| Entry Flow, veh/h | 554 | 625 | 813 | 916 | 957 | 23 | 226 | |
| Cap Entry Lane, veh/h | 1322 | 1393 | 970 | 1047 | 0.980 | 275 | 327 | |
| Entry HV Adj Factor | 0.981 | 0.980 | 0.980 | 0.981 | 80 | 1.000 | 0.982 | |
| Flow Entry, veh/h | 543 | 613 | 797 | 898 | 938 | 23 | 222 | |
| Cap Entry, veh/h | 1296 | 1365 | 951 | 1026 | 0.085 | 275 | 321 | |
| V/C Ratio | 0.419 | 0.449 | 0.838 | 0.875 | 4.6 | 0.084 | 0.692 | |
| Control Delay, s/veh | 6.9 | 7.0 | 24.0 | 26.4 | A | 14.7 | 36.5 | |
| LOS | A | A | C | D | 0 | B | E | |
| 95th %tile Queue, veh | 2 | 2 | 10 | 12 | | 0 | 5 | |

Signal Warrant Analysis

STUDY AND ANALYSIS INFORMATION

Municipality: Midcoast
 County: San Mateo

Analysis Date: 7/8/2019
 Conducted By: Aditi Meshram
 Agency/Company Name: DKS Associates

Analysis Information

Data Collection Date: 7/11/2019 - 7/11/2019
 Day of the Week: Thursday

Is the intersection in a built-up area of an isolated community of <10,000 population? Yes

Major Street Information

Major Street Name and Route Number: Highway 1
 Major Street Approach #1 Direction: N-Bound
 Major Street Approach #2 Direction: S-Bound

Number of Lanes for Moving Traffic on Each Major Street Approach: 1 LANE(S)
 Speed Limit or 85th Percentile Speed on the Major Street: 50 MPH

Minor Street Information

Minor Street Name and Route Number: California Ave
 Minor Street Approach #1 Direction: E-Bound
 Minor Street Approach #2 Direction: W-Bound

Number of Lanes for Moving Traffic on Each Minor Street Approach: 1 LANE(S)

TRAFFIC SIGNAL WARRANT ANALYSIS FINDINGS

| | Applicable? | Warrant Met? |
|---|-------------|--------------|
| Warrant 1, Eight-Hour Vehicular Volume | Yes | No |
| Warrant 2, Four-Hour Vehicular Volume | Yes | No |
| Warrant 3, Peak Hour | Yes | No |
| Warrant 4, Pedestrian Volume | No | N/A |
| Warrant 5, School Crossing | No | N/A |
| Warrant 6, Coordinated Signal System | No | N/A |
| Warrant 7, Crash Experience | No | N/A |
| Warrant 8, Roadway Network | No | N/A |
| Warrant 9, Intersection Near a Grade Crossing | No | N/A |
| Bicycle Signal Warrant (CA) | No | N/A |

STUDY AND ANALYSIS INFORMATION

Municipality: Midcoast
 County: San Mateo

Analysis Date: 7/8/2019
 Conducted By: ADM
 Agency/Company Name: DKS Associates

Analysis Information

Data Collection Date: 7/11/2019 - 7/11/2019
 Day of the Week: Thursday

Is the intersection in a built-up area of an isolated community of <10,000 population? Yes

Major Street Information

Major Street Name and Route Number: Highway 1
 Major Street Approach #1 Direction: N-Bound
 Major Street Approach #2 Direction: S-Bound

Number of Lanes for Moving Traffic on Each Major Street Approach: 1 LANE(S)
 Speed Limit or 85th Percentile Speed on the Major Street: 50 MPH

Minor Street Information

Minor Street Name and Route Number: California Ave
 Minor Street Approach #1 Direction: E-Bound
 Minor Street Approach #2 Direction: W-Bound

Number of Lanes for Moving Traffic on Each Minor Street Approach: 1 LANE(S)

TRAFFIC SIGNAL WARRANT ANALYSIS FINDINGS

| | Applicable? | Warrant Met? |
|---|-------------|--------------|
| Warrant 1, Eight-Hour Vehicular Volume | Yes | Yes |
| Warrant 2, Four-Hour Vehicular Volume | Yes | Yes |
| Warrant 3, Peak Hour | Yes | No |
| Warrant 4, Pedestrian Volume | No | N/A |
| Warrant 5, School Crossing | No | N/A |
| Warrant 6, Coordinated Signal System | No | N/A |
| Warrant 7, Crash Experience | No | N/A |
| Warrant 8, Roadway Network | No | N/A |
| Warrant 9, Intersection Near a Grade Crossing | No | N/A |
| Warrant PA-1, ADT Volume Warrant | No | N/A |
| Warrant PA-2, Midblock and Trail Crossings | No | N/A |

STUDY AND ANALYSIS INFORMATION

Municipality: Midcoast
 County: San Mateo

Analysis Date: 7/8/2019
 Conducted By: ADM
 Agency/Company Name: DKS Associates

Analysis Information

Data Collection Date: 7/11/2019 - 7/11/2019
 Day of the Week: Thursday

Is the intersection in a built-up area of an isolated community of <10,000 population? No

Major Street Information

Major Street Name and Route Number: Highway 1
 Major Street Approach #1 Direction: N-Bound
 Major Street Approach #2 Direction: S-Bound

Number of Lanes for Moving Traffic on Each Major Street Approach: 1 LANE(S)
 Speed Limit or 85th Percentile Speed on the Major Street: 50 MPH

Minor Street Information

Minor Street Name and Route Number: Cypress Ave
 Minor Street Approach #1 Direction: E-Bound
 Minor Street Approach #2 Direction: W-Bound

Number of Lanes for Moving Traffic on Each Minor Street Approach: 1 LANE(S)

TRAFFIC SIGNAL WARRANT ANALYSIS FINDINGS

| | Applicable? | Warrant Met? |
|---|-------------|--------------|
| Warrant 1, Eight-Hour Vehicular Volume | Yes | Yes |
| Warrant 2, Four-Hour Vehicular Volume | Yes | Yes |
| Warrant 3, Peak Hour | Yes | No |
| Warrant 4, Pedestrian Volume | No | N/A |
| Warrant 5, School Crossing | No | N/A |
| Warrant 6, Coordinated Signal System | No | N/A |
| Warrant 7, Crash Experience | No | N/A |
| Warrant 8, Roadway Network | No | N/A |
| Warrant 9, Intersection Near a Grade Crossing | No | N/A |
| Warrant PA-1, ADT Volume Warrant | No | N/A |
| Warrant PA-2, Midblock and Trail Crossings | No | N/A |

STUDY AND ANALYSIS INFORMATION

Municipality: Midcoast
 County: San Mateo

Analysis Date: 7/8/2019
 Conducted By: ADM
 Agency/Company Name: DKS Associates

Analysis Information

Data Collection Date: 7/11/2019 - 7/11/2019
 Day of the Week: Thursday

Is the intersection in a built-up area of an isolated community of <10,000 population? No

Major Street Information

Major Street Name and Route Number: Highway 1
 Major Street Approach #1 Direction: N-Bound
 Major Street Approach #2 Direction: S-Bound

Number of Lanes for Moving Traffic on Each Major Street Approach: 1 LANE(S)
 Speed Limit or 85th Percentile Speed on the Major Street: 50 MPH

Minor Street Information

Minor Street Name and Route Number: Cypress Ave
 Minor Street Approach #1 Direction: E-Bound
 Minor Street Approach #2 Direction: W-Bound

Number of Lanes for Moving Traffic on Each Minor Street Approach: 1 LANE(S)

TRAFFIC SIGNAL WARRANT ANALYSIS FINDINGS

| | Applicable? | Warrant Met? |
|---|-------------|--------------|
| Warrant 1, Eight-Hour Vehicular Volume | Yes | No |
| Warrant 2, Four-Hour Vehicular Volume | Yes | Yes |
| Warrant 3, Peak Hour | Yes | No |
| Warrant 4, Pedestrian Volume | No | N/A |
| Warrant 5, School Crossing | No | N/A |
| Warrant 6, Coordinated Signal System | No | N/A |
| Warrant 7, Crash Experience | No | N/A |
| Warrant 8, Roadway Network | No | N/A |
| Warrant 9, Intersection Near a Grade Crossing | No | N/A |
| Warrant PA-1, ADT Volume Warrant | No | N/A |
| Warrant PA-2, Midblock and Trail Crossings | No | N/A |

STUDY AND ANALYSIS INFORMATION

Municipality: Midcoast
 County: San Mateo

Analysis Date: 7/8/2019
 Conducted By: ADM
 Agency/Company Name: DKS Associates

Analysis Information

Data Collection Date: 2/5/2014 - 2/6/2014
 Day of the Week: Wednesday

Is the intersection in a built-up area of an isolated community of <10,000 population? No

Major Street Information

Major Street Name and Route Number: Highway 1
 Major Street Approach #1 Direction: N-Bound
 Major Street Approach #2 Direction: S-Bound

Number of Lanes for Moving Traffic on Each Major Street Approach: 1 LANE(S)
 Speed Limit or 85th Percentile Speed on the Major Street: 50 MPH

Minor Street Information

Minor Street Name and Route Number: 16th St
 Minor Street Approach #1 Direction: E-Bound
 Minor Street Approach #2 Direction: W-Bound

Number of Lanes for Moving Traffic on Each Minor Street Approach: 1 LANE(S)

TRAFFIC SIGNAL WARRANT ANALYSIS FINDINGS

| | Applicable? | Warrant Met? |
|---|-------------|--------------|
| Warrant 1, Eight-Hour Vehicular Volume | Yes | No |
| Warrant 2, Four-Hour Vehicular Volume | Yes | No |
| Warrant 3, Peak Hour | Yes | No |
| Warrant 4, Pedestrian Volume | No | N/A |
| Warrant 5, School Crossing | No | N/A |
| Warrant 6, Coordinated Signal System | No | N/A |
| Warrant 7, Crash Experience | No | N/A |
| Warrant 8, Roadway Network | No | N/A |
| Warrant 9, Intersection Near a Grade Crossing | No | N/A |
| Warrant PA-1, ADT Volume Warrant | No | N/A |
| Warrant PA-2, Midblock and Trail Crossings | No | N/A |

STUDY AND ANALYSIS INFORMATION

Municipality: Midcoast
 County: San Mateo

Analysis Date: 7/8/2019
 Conducted By: ADM
 Agency/Company Name: DKS Associates

Analysis Information

Data Collection Date: 2/5/2014 - 2/6/2014
 Day of the Week: Wednesday

Is the intersection in a built-up area of an isolated community of <10,000 population? No

Major Street Information

Major Street Name and Route Number: Highway 1
 Major Street Approach #1 Direction: N-Bound
 Major Street Approach #2 Direction: S-Bound

Number of Lanes for Moving Traffic on Each Major Street Approach: 1 LANE(S)
 Speed Limit or 85th Percentile Speed on the Major Street: 50 MPH

Minor Street Information

Minor Street Name and Route Number: 16th St
 Minor Street Approach #1 Direction: E-Bound
 Minor Street Approach #2 Direction: W-Bound

Number of Lanes for Moving Traffic on Each Minor Street Approach: 1 LANE(S)

TRAFFIC SIGNAL WARRANT ANALYSIS FINDINGS

| | Applicable? | Warrant Met? |
|---|-------------|--------------|
| Warrant 1, Eight-Hour Vehicular Volume | Yes | No |
| Warrant 2, Four-Hour Vehicular Volume | Yes | No |
| Warrant 3, Peak Hour | Yes | No |
| Warrant 4, Pedestrian Volume | No | N/A |
| Warrant 5, School Crossing | No | N/A |
| Warrant 6, Coordinated Signal System | No | N/A |
| Warrant 7, Crash Experience | No | N/A |
| Warrant 8, Roadway Network | No | N/A |
| Warrant 9, Intersection Near a Grade Crossing | No | N/A |
| Warrant PA-1, ADT Volume Warrant | No | N/A |
| Warrant PA-2, Midblock and Trail Crossings | No | N/A |

Appendix D

List of Project Reports

Appendix D: List of Project Reports

The following reports were developed as part of the analysis and outreach process of the project. The documents are available for viewing on the Connect the Coastsides website:

<https://planning.smcgov.org/connect-coastside>

1. Buildout Analysis and Traffic Projects, November 2014
2. Evaluation of Transportation Alternatives to Address Buildout Deficiencies, April 2015
3. Development Forecast for the San Mateo County Comprehensive Transportation Management Plan, November 2015
4. Evaluation of Recommended Alternative to Address Potential Future Transportation Deficiencies, March 2016

The website also contains copies of presentations given to the Technical Advisory Committee, Midcoast Council, and Half Moon Bay City Council

Appendix E

List of Referenced Reports

Appendix E: List of Referenced Reports

The following studies and reports were referenced as part of the Midcoast framework and background information and informed the analysis and conclusions of the projects. The documents are available for viewing on the Connect the Coastside website:

<https://planning.smcgov.org/connect-coastside>

1. Highway 1 Safety and Mobility Improvement Study Phase 1, April 2010
2. Highway 1 Safety and Mobility Improvement Study Phase 2, November 2011
3. San Mateo County Congestion Management Program, November 2011
4. County of San Mateo Local Coastal Program Policies, June 2013
5. San Mateo County Traffic Impact Study Requirements, September 2013
6. Plan Princeton Exiting Conditions Report, May 2014
7. Coastside Access Study, April 2015
8. Highway 1 Congestion & Safety Improvement Project, August 2015
9. SamTrans Coastside Plan, August 2018

Additionally, the following policy, engineering, and design references were used in the development of standards and improvements.

1. NACTO Urban Bikeway Design Guide, 2011
2. California Manual of Uniform Control Devices, 2012
3. Zoning Regulations, Section 6254.4 (11), Planning and Building Department, County of San Mateo